



AGENDA

*Village of Hoffman Estates
Second Meeting of the Month
Village Board of Trustees*

*1900 Hassell Road
Hoffman Estates, IL 60169
847/882-9100*

Board Room

December 14, 2009

Immediately Following Public Works & Utilities Committee

1. **CALL TO ORDER/ROLL CALL**
2. **PLEDGE OF ALLEGIANCE TO THE FLAG**
3. **RECOGNITION OF AUDIENCE**
4. **CONSENT AGENDA/OMNIBUS VOTE (Roll Call Vote)**
(All items under the Consent Agenda are considered to be routine in nature and will be enacted by one motion. There will be no separate discussion of these items unless a Trustee so requests. In that event, the discussion will be the first item of business after approval of the Consent Agenda).
 - A. Approval of Agenda
 - B. Approval of Minutes – December 7, 2009 (*deferral requested*)
 - C. Approval of the schedule of bills for December 14, 2009: \$635,482.36.
5. **REPORTS**
 - A. **President's Report**
 - 1) Proclamation(s)
 - Dennis Jones Day (35 Years Service)
 - Marion Graczyk Day (30 Years Service)
 - Ted Bos Day (25 Years Service)
 - Richard Russo Day (25 Years Service)
 - Shelley Walenga Day (20 Years Service)
 - David Eggers Day (15 Years Service)
 - Michael Barber Day (10 Years Service)
 - Kathryn Cawley Day (10 Years Service)
 - Scott Reichel Day (10 Years Service)
 - Matthew Teipel Day (10 Years Service)
 - B. **Trustee Comments**
 - C. **Village Manager's Report**
 - D. **Village Clerk's Report**
6. **PLAN COMMISSION/ZONING BOARD OF APPEALS REPORTS**
 - A. **PLAN COMMISSION (Chairman Stanton)**
 - 1) Request by W2001 VHE Realty, LLC, and Rubina Realty Corporation for preliminary and final plat of resubdivision of Lot 2 and 3 of Poplar Creek Crossing subdivision, with 3 conditions (see packets).

Voting: 4 Ayes, 2 Nays, 5 Absent
Motion carried.

6. **PLAN COMMISSION/ZONING BOARD OF APPEALS REPORTS-Continued**

- 2) Request by W2001 VHE Realty, LLC for site plan amendment to the Poplar Creek Crossing Shopping Center, with 3 conditions (see packets).

Voting: 4 Ayes, 2 Nays, 5 Absent

Motion carried.

- 3) Request by W2001 VHE Realty, LLC for site plan amendment to the Citibank development at the Poplar Creek Crossing Shopping Center, with 3 conditions (see packets).

Voting: 4 Ayes, 2 Nays, 5 Absent

Motion carried.

- 4) Request by W2001 VHE Realty, LLC for site plan amendment to the Fifth Third Bank development at the Poplar Creek Crossing Shopping Center, with 3 conditions (see packets).

Voting: 4 Ayes, 2 Nays, 5 Absent

Motion carried.

- 5) Request by W2001 VHE Realty, LLC for site plan amendment to the Buffalo Wild Wings restaurant at the Poplar Creek Crossing Shopping Center, with 3 conditions (see packets).

Voting: 4 Ayes, 2 Nays, 5 Absent

Motion carried.

B. ZONING BOARD OF APPEALS (Chairman Weaver)

- 1) Request by W2001 VHE Realty, LLC, for a four foot, eleven inch (4'11") setback variation from Section 9-8-3-B-5-d of the Zoning Code to permit a fifteen foot, one inch (15'1") side yard setback along the south property line instead of the minimum required twenty foot (20') setback, with 1 condition (see packets).

Voting: 7 Ayes

Motion carried.

- 2) Request by the Village of Hoffman Estates (lessor) and The Everest Group/Sprint, Clearwire (lessee), for a special use under Sections 9-5-9-D and 9-3-9-A to permit the installation of two (2) microwave dishes and associated equipment to be no greater than 133 feet high on a Village water tank at 4690 Olmstead Drive, with 4 conditions (see packets).

Voting: 7 Ayes

Motion carried.

(Immediate authorization to apply for permits is requested)

- 3) Request by the Village of Hoffman Estates (lessor) and The Everest Group/Sprint, Clearwire (lessee), for a special use under Sections 9-3-9-A and 9-8-2-C-6 to permit the installation of two (2) microwave dishes and associated equipment to be no greater than 106 feet high on a Village water tank at 95 Aster Lane, with 4 conditions (see packets).

Voting: 5 Ayes, 2 Nays

Motion carried.

(Immediate authorization to apply for permits is requested)

6. **PLAN COMMISSION/ZONING BOARD OF APPEALS REPORTS-Continued**
- 4) Request by the Village of Hoffman Estates (lessor) and The Everest Group/Sprint, Clearwire (lessee), for a special use under Sections 9-3-9-A and 9-5-7-C-4 to permit the installation of two (2) microwave dishes and associated equipment to be no greater than 134 feet high on a Village water tank at 3990 Huntington Boulevard, with 4 conditions (see packets).

Voting: 7 Ayes

Motion carried.

(Immediate authorization to apply for permits is requested)

7. **ADDITIONAL BUSINESS** *(All other new business; those items not recommended by a majority of the Committee)*
- A. Request Board approval to cede 2009 Bond Cap to the Assist First Time Homebuyer Down Payment Assistance Program.
- B. Request Board approval of the 2010 property and casualty and workers' compensation excess insurance program renewal.
- C. Request Board approval of agreements for Pace Bus Route 554 service with:
- 1) Pace Suburban Bus for a local share of Route 554 in an amount not to exceed \$23,104.27 in 2010; and
- 2) Elgin, Streamwood, Hanover Park, and Schaumburg for local agency participation in Route 554 in 2010.
- D. Request Board approval of a time extension to the temporary moratorium on the enforcement of regulations in Section 9-3-8-K-5 of the Zoning Code to allow certain temporary business signs to be displayed during depressed economic conditions.
- E. Request Board approval of the adoption of the Community Development Block Grant (CDBG) Consolidated Annual Performance and Evaluation Report (CAPER) for Program Year 3.
- F. Request Board authorization for an EDA contract for Prairie Stone Entertainment District regulatory/wayfinding signs in an amount not to exceed \$8,952.75.
- G. Request Board approval of a Resolution creating the Tartan Day Commission of the Village of Hoffman Estates (increase in membership).
- H. Request Board approval of a Resolution terminating Agreement with the Northeastern Illinois Regional Crime Laboratory.
- I. Request Board approval to extend the Fire Department Entry Level Eligibility Roster for a period of one (1) year.
- J. Request Board approval to extend the automatic ticket writer contract with Advanced Public Safety, Deerfield Beach, FL and place the project on hold.

7. **ADDITIONAL BUSINESS - Continued**

K. Request Board approval of Agreement between the Village of Hoffman Estates and James H. Norris for the period January 1, 2010 through December 31, 2010.

8. **ADJOURNMENT – *Executive Session -- Litigation (5 ILCS 120/2-(c)-(11))***

The Village of Hoffman Estates complies with the Americans with Disabilities Act (ADA). For accessibility assistance, call the ADA Coordinator at 847/882-9100.

VILLAGE OF HOFFMAN ESTATES
ZONING BOARD OF APPEALS

FINDING OF FACT

DATE OF PUBLIC HEARING: December 8, 2009

DATE OF PRESENTATION TO VILLAGE BOARD: December 14, 2009

PETITION: Hearing held at the request of W2001 VHE Realty, LLC to consider variations from the Zoning Code to permit a setback variation for the existing Citibank facility on the property known as 2550 N. Sutton Road in the Poplar Creek Crossing Shopping Cent.

DISTRICT IN WHICH PROPERTY IS LOCATED: C-MU, Commercial Mixed-Use

ZONING CODE SECTION(S) FOR VARIATION(S): 9-8-3-B-5-d

FINDING-OF-FACT: The Zoning Board of Appeals (ZBA) found that the Standards for a Variation (9-1-15-C) were met.

MOTION: Request to grant W2001 VHE Realty, LLC at 2550 N. Sutton Road, *a 4'11" setback variation from Section 9-8-3-B-5-d of the Zoning Code to permit a 15'1" side yard setback along the south property line instead of the minimum required 20' setback.* The following condition shall apply:

This variation approval shall be subject to all conditions of the preliminary and final Plat of Resubdivision and site plan amendment approval for the Citibank portion of the Poplar Creek Crossing Shopping Center property.

RECOMMENDATION: The Zoning Board of Appeals (ZBA) recommends approval of this request.

The petitioner is requesting a side yard setback variation for the existing Citibank building at 2550 N. Sutton Road. The existing drive-thru canopy is 15'1" from the south property line. A 20' setback is required. The canopy overhang encroaches into the 20' setback. The site plan for the Citibank building was approved by the Village Board in 2006. The setback encroachment was missed during the site plan review process. The setback encroachment was discovered by staff while reviewing the petitioner's request for a resubdivision of the Poplar Creek Crossing Shopping Center property. The need for a setback variation is not caused by the petitioner's request for a resubdivision of the property.

The petitioner stated that they agreed to the recommended condition with the understanding that the setback variation is necessary and would like it to be approved whether the resubdivision of the property is approved or not. The petitioner does not want the variation approval to be contingent upon the approval of the resubdivision request. The recommended condition of approval states that the variation approval be subject to all conditions of approval of the

preliminary and final Plat of Resubdivision and site plan amendment approval for the Citibank portion of the property.

AUDIENCE COMMENTS

None.

VOTE:

7 Ayes

0 Nays

0 Absent

MOTION PASSED

ZONING BOARD OF APPEALS

Chairman William Weaver

Vice-Chairman Ronald Jehlik

Denise Wilson

Michael Ciffone

Masoom Ali

Donna Boomgarden

Michael Gaeta

*** IMMEDIATE AUTHORIZATION TO APPLY FOR PERMITS IS NOT REQUESTED**

*

**THIS VARIATION WILL EXPIRE UNLESS ACTED UPON WITHIN
ONE (1) YEAR OF VILLAGE BOARD APPROVAL**

FINDING OF FACT WRITTEN BY DEVELOPMENT SERVICES STAFF

VILLAGE OF HOFFMAN ESTATES

Memo

TO: William Weaver, Zoning Board of Appeals Chairman
FROM: Brian S. Portz, Associate Planner *bsp*
RE: 2550 N. SUTTON ROAD - SETBACK VARIATION FOR CITIBANK BUILDING ON THE POPLAR CREEK CROSSING SHOPPING CENTER PROPERTY
DATE: December 2, 2009

HEARING DATE: December 8, 2009

1. REQUEST SUMMARY

Request by W2001 VHE Realty, LLC to consider approval of setback variations for the Citibank building on the Poplar Creek Crossing Shopping Center property.

The Plan Commission considered a preliminary and final plat of resubdivision and site plan amendment for the shopping center property on December 2, 2009. The Plan Commission recommended approval of the preliminary and final Plat of Resubdivision and approval of a site plan amendment for the shopping center property, the Fifth Third bank lot, the Buffalo Wild Wings lot and the Citibank lot. The Plan Commission recommendation will be considered by the Village Board concurrently with the Zoning Board of Appeals recommendation.

2. BACKGROUND

The Poplar Creek Crossing Shopping Center received final Village Board site plan approval in April 2005. The approval included a Target store and several mid-sized retail tenants, several small tenant retail buildings, and conceptual building pad sites for a freestanding restaurant and two banks. Subsequently, final site plan approval was granted by the Village Board for a Claim Jumper restaurant, Citibank, Fifth Third Bank, and a Buffalo Wild Wings restaurant.

The shopping center is currently subdivided into three lots that contain the Target store and its parking lot, the rest of the retail buildings and parking lots, and a vacant 1.7 acre lot (south of the shopping center) that was retained by the previous property owner for future development. The entire shopping center is designed with common access, parking, detention, etc. There are access drives to the site from Sutton Road, Higgins Road, and Old Sutton Road.

As part of the development, two "future development sites" were proposed on the overall site plan. One site is located on the north side of the property and the other is located on the south side of the property. The site on the north side of the property was developed as a Fifth Third Bank and the site on the south side of the property was developed as a Citibank. These two properties are part of the shopping center property, but somewhat act as stand alone properties with their own parking areas.

3. PROPOSAL

The petitioner is proposing to resubdivide the existing 30.67 acre site into five separate lots to permit the sale of the Fifth Third Bank, Citibank, and Buffalo Wild Wings as separate lots. To accomplish this, new property lines will be drawn around the Fifth Third Bank, Citibank and Buffalo Wild Wings properties. Through the review of the resubdivision request, it was discovered that the existing Citibank drive-thru canopy encroaches into the required setback area along the south property line. The canopy is located 15'1" from the south property line instead of the required 20'.

4. SITE CONDITIONS

- a) The subject site is zoned C-MU, Commercial Mixed Use District.
- b) The surrounding properties to the south and west are zoned EDA, Economic Development Area District. To the north is a vacant property also zoned C-MU, Commercial Mixed Use District. East of the property, across Route 59, is vacant property that is zoned O-3, Office and Research Tollway District and B-2, Business District.

5. APPLICABLE REQUIREMENTS

Setback Variation

Section 9-8-3-B-5-d (p. CD9:126.4) states that the minimum side yard setback shall be 20 feet. This section includes an exception requiring no side yard setback for common or abutting walls. There are no common or abutting walls in this situation.

6. ADJACENT OWNER COMMENTS

Standard notification letters have been mailed and as of this writing, no comments have been received.

7. VARIATION HISTORY

Subject Site

No relevant variations

Similar In the Village

Ordinance No. 4096-2009 was granted to National City Bank on Algonquin Road to permit a 2.95 foot variation of the 30 foot setback requirement along Algonquin Road to construct an accessory bank teller canopy.

8. ENGINEERING/TRANSPORTATION COMMENTS

No comments

9. DEVELOPMENT SERVICES COMMENTS

Planning staff analyzed the plat of resubdivision request to determine if the new property line locations will cause any building encroachments into the front, side or rear yard setbacks. This analysis determined that the location of the proposed property lines on the Fifth Third Bank and Buffalo Wild Wings lots will not require setback variations. All setback requirements will be met on those two properties. On the Citibank property, the new property line locations on the east, west and north sides will not require any setback variations, but it was noted that a setback variation would be necessary on the south side because the existing drive-thru canopy was already encroaching into the 20' side yard setback area.

The need for a variation along the south property line of the Citibank lot is not due the petitioner's request to resubdivide the property. The encroachment already exists because the south property line existed in its current location when the building and canopy were originally constructed. This was an omission that occurred at the time of original development and now a variation is being requested to correct this omission.

10. MOTION

Should the Zoning Board of Appeals find that the Standards for a Variation are met, the following motion is provided:

A 4'11" setback variation from Section 9-8-3-B-5-d of the Zoning Code to permit a 15'1" side yard setback along the south property line instead of the minimum required 20' setback, subject to the following condition.

This variation approval shall be subject to all conditions of the preliminary and final Plat of Resubdivision and site plan amendment approval for the Citibank portion of the Poplar Creek Crossing Shopping Center property.

cc: M. Koplín, A. Janura, D. O'Malley, D. Plass, Petitioner

**VILLAGE OF HOFFMAN ESTATES
ZONING BOARD OF APPEALS
REQUEST FOR NON-RESIDENTIAL VARIATION HEARING**

FOR VILLAGE USE ONLY

Hearing Fee \$ 400.00 Date Paid 10/27/09 Received By P. Moore
 Hearing Date: 12/8/09 Time: 7:30pm Legal Published 11/23/09
 Receipt Number 30324 Check No. 10517 Zoning District C-MU

Front Yard Setback, Side Yard Setback, Rear Yard Setback, Height, or Other Variation	Present Condition	Required	Proposed
<i>side yard</i>	<i>15'1"</i>	<i>20'</i>	<i>15'1"</i>

PLEASE PRINT OR TYPE

1. Name of Property Owner(s)* W2001 VHE Realty, LLC

E-Mail Address peggy.kral@archon.com Fax #: (312) 629-7850

Owner's Address 71 S Wacker, Ste 2110 Phone (312) 629-7825

City Chicago State IL Zip 60606

Subject Property's Address (if different): Citibank, 2550 New Sutton Road, Hoffman Estates, Illinois (Poplar Creek Shopping Center)

2. Person applying if other than owner:*

Name _____ Company Archon Group

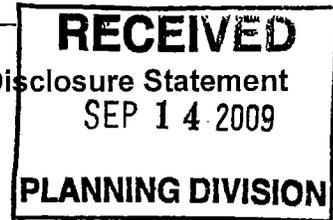
E-Mail Address _____ Fax #: _____

Address _____ Phone _____

City _____ State _____ Zip _____

3. Property Index Number (PIN) 01-33-301-003-0000

* If "owner" is an entity other than an individual(s), then an Economic Disclosure Statement must also be filed.



4. Is the applicant the original owner? Yes Did the condition that instituted this request for a variation exist at the time the applicant purchased this property? If yes, please describe.

No

5. Describe the improvement that needs a variation and the reasons a variation would be appropriate using the Standards for Variation (attached) as a guide. (e.g. An addition requires a 5-foot front yard setback variation. A hardship was caused by a street-widening project that reduced the front yard by 10 feet.)

The improvement that needs a variation is the canopy over the Citibank drive-through facility. The canopy encroaches 4.84 feet into the south yard. The Village Zoning Code allows, for a roof, a 2-foot encroachment into a side yard. See attached response to Question 5 for more detail.

6. Describe existing construction materials and proposed materials indicating if proposed construction will match or blend with the existing structure.

Construction materials are existing and include brick, spandrel glass, cast stone, steel and composite metal panels. No new construction is proposed.

7. Estimated Total Project Cost N/A

8. Will the proposed construction require removal/relocation of trees, driveway apron, utilities, other? If yes, please describe.

N/A

9. An Immediate Authorization to Apply for Permits allows the Code Enforcement Division to begin the building permit review process prior to the adoption of the Ordinance by the Village Board. The Immediate Authorization makes it possible for you to expedite the plan review process by approximately two weeks. Contact the Code Enforcement Division at 847/781-2631 to discuss the building permit application and review process.

Please check one of the following

Yes, I request Immediate Authorization to Apply for Permits upon approval of my application by the Village Board, allowing me to begin the building permit review process prior to adoption of the Ordinance approving my variation.

Or

No, I do not request Immediate Authorization to Apply for Permits.

10. I, the undersigned, certify the information and submissions provided accurately represent the current conditions and proposed improvement(s) needing a variation.

Owner's Signature Steve Smith

Name (Please Print) Stephen M Lipscomb

Applicant's Signature Peggy Kral

Name (Please Print) PEGGY KRAL

**VILLAGE OF HOFFMAN ESTATES
ZONING BOARD OF APPEALS
REQUEST FOR NON-RESIDENTIAL VARIATION HEARING**

RESPONSE TO QUESTION 5

W2001 VHE Realty, LLC (the "*Applicant*") currently owns the property commonly known as Poplar Creek Crossing Shopping Center (the "*Property*"). On April 4, 2005, the Village approved a plat of subdivision for the shopping center. Pursuant to the plat, the Target store and its adjacent parking are located in Lot 1 of the subdivision, while the entire remainder of the shopping center is located in Lot 2 and Lot 3. Now, the Applicant desires to resubdivide the Property to create four new lots within what is currently Lot 2 (the "*Resubdivision*").

This Resubdivision will accommodate the sale of the land under the Citibank, Fifth Third Bank, and Buffalo Wild Wings buildings. As Village staff interprets the Village's Zoning Code, these new lots can be created only if (i) the newly created lots (and improvements thereon) will comply with the Village's zoning requirements applicable to the C-MU District, or (ii) the Village approves zoning variations.

The four proposed lots (and the improvements thereon) comply with all applicable use, density, height, setback, and other zoning requirements, except for one side yard requirement for one lot: the south side yard for Citibank.

Citibank operates a walk-in and drive through banking facility ("*Citibank*") on proposed Lot 3 of the Resubdivision ("*Proposed Lot 3*"). Citibank's operations have included three drive through Automated Teller Machines (the "*ATMs*"), which are covered by a canopy (the "*Canopy*"), on the south side of Proposed Lot 3. The Canopy has existed as it currently stands since its construction in early 2007.

The Canopy is interpreted by staff to be part of the principal Citibank building as opposed to being an accessory building. As such, it is subject to the 20-foot side yard requirement for principal buildings within the C-MU district (Zoning Code § 9-8-3-B-5-d), rather than the 3-foot side yard requirement applicable to accessory buildings. (Zoning Code § 9-3-6-D). While the footprint of the Canopy on the ground conforms with all of the Village side yard requirements, the overhang on the Canopy encroaches into the side yard by between 4.69 and 4.84 feet, as shown on the plat of survey for Proposed Lot 3. Thus, the actual side yard provided is 15.16 feet. The first 2 feet of the eave of the Canopy are a permitted encroachment within the side yard. (See Zoning Code § 9-2-1). Therefore, the encroachment for which a variation is necessary is 2.84 feet.

The Applicant respectfully requests that the Zoning Board of Appeals recommend, and the Village Board of Trustees grant, a variation of the 20-foot side yard setback regulation in the Zoning Code for the Canopy (the "*Requested Variation*") because of the practical difficulties with, and particular hardships caused by, conforming to the regulations. For the reasons stated below: 1) the Property cannot yield a reasonable return if permitted to be used only under the conditions allowed by the applicable zoning regulations; 2) the Applicant's hardship is due to unique circumstances; and 3) the Requested Variation, if granted, will not alter the essential character of the locality.

1. Reasonable Return

The Property cannot yield a reasonable return if the Canopy is subjected to the 20-foot side yard regulation. A drive-through facility of three or more lanes is a virtual necessity for a suburban branch bank. The facility is subordinate to and serves the branch bank building and contributes to the convenience or necessity of the bank's patrons. As such, one might argue that the Canopy over the drive-through facility is more appropriately characterized as an "accessory building" rather than part of the principal building. (See Zoning Code § 9-2-1 (definition of "accessory building")). Whether or not it strictly satisfies the definition of "accessory building," the Canopy serves an accessory function and, if one were "starting from scratch," it would be more reasonable to apply the 3-foot side yard applicable to accessory buildings than it would the 20-foot general C-MU side yard requirement. (See Zoning Code § 9-3-6-D (3-foot side yard for accessory buildings)).

Nevertheless, even if the Canopy is part of the principal building and the 20-foot requirement applies, a variation is appropriate in this case. A drive-through facility "eats up" a lot of ground without resulting in much bulk. To illustrate, a drive-through facility typically projects at least 40 feet from the side of a bank building. However, except for its Canopy and supports, the facility at issue here is open air and does not block a significant amount of air and sunlight.

Illinois' Zoning Enabling Act begins with the words: "To the end that *adequate light, pure air*, and safety from fire and other dangers may be secured" 65 ILCS 5/11-13-1 (emphasis added). While these are important goals, they are not advanced when the 20-foot side yard requirement is applied to the Canopy. The Canopy, which encroaches less than 5 feet into the side yard, certainly blocks less air and sunlight than would a walled building addition that extends out to satisfies the side yard requirement. Further, the entire banking facility is a relatively low impact land use. The building itself is less than 4,700 square feet and, as mentioned above, the drive-through facility is open air, except for the Canopy and its supports.

As a result, the 20-foot side yard, when applied to this situation, is unreasonable and denies the Applicant a reasonable use of, and return, on its property. The minor variation requested is appropriate.

2. Unique Circumstances

The Applicant's hardship is due to unique circumstances and the conditions upon which the Requested Variation is based would not be applicable, generally, to other property within the same zoning classification. This Requested Variation is very specifically applicable only to an open canopy covering drive-through ATM facilities. Furthermore, the Canopy's current encroachment within the setback is minor, and the footprint of the Canopy falls within the Zoning Regulations.

3. Essential Character

The Requested Variation, if granted, will not alter the essential character of the locality. Because of the commercial nature of the location and the Canopy's minimal encroachment into the side yard, the Requested Variation will not be detrimental to the public welfare or injurious to the other property or improvements in the neighborhood in which the Property is located. Furthermore, well established landscaping along the south lot line of Proposed Lot 3 mitigates any impact that exists for neighbors to the south.

The Requested Variation will not impair an adequate supply of light and air to the adjacent property, or substantially increase the congestion in the public streets, increase the danger of fires, endanger the public safety, or substantially diminish or impair property values in the neighborhood. Because the Canopy is an open structure, light or air are not substantially obstructed.

As an aside, Citibank has been operating with the existing Canopy since March 5, 2007 with no complaints from surrounding neighbors. Unlike most variations granted by municipalities, any impacts resulting from the granting of the Requested Variation have already been determined, and no uncertainty regarding outcomes or unintended consequences remains.

As a result of the foregoing, the Applicant respectfully requests the Zoning Board of Appeals recommend approval of the Requested Variation

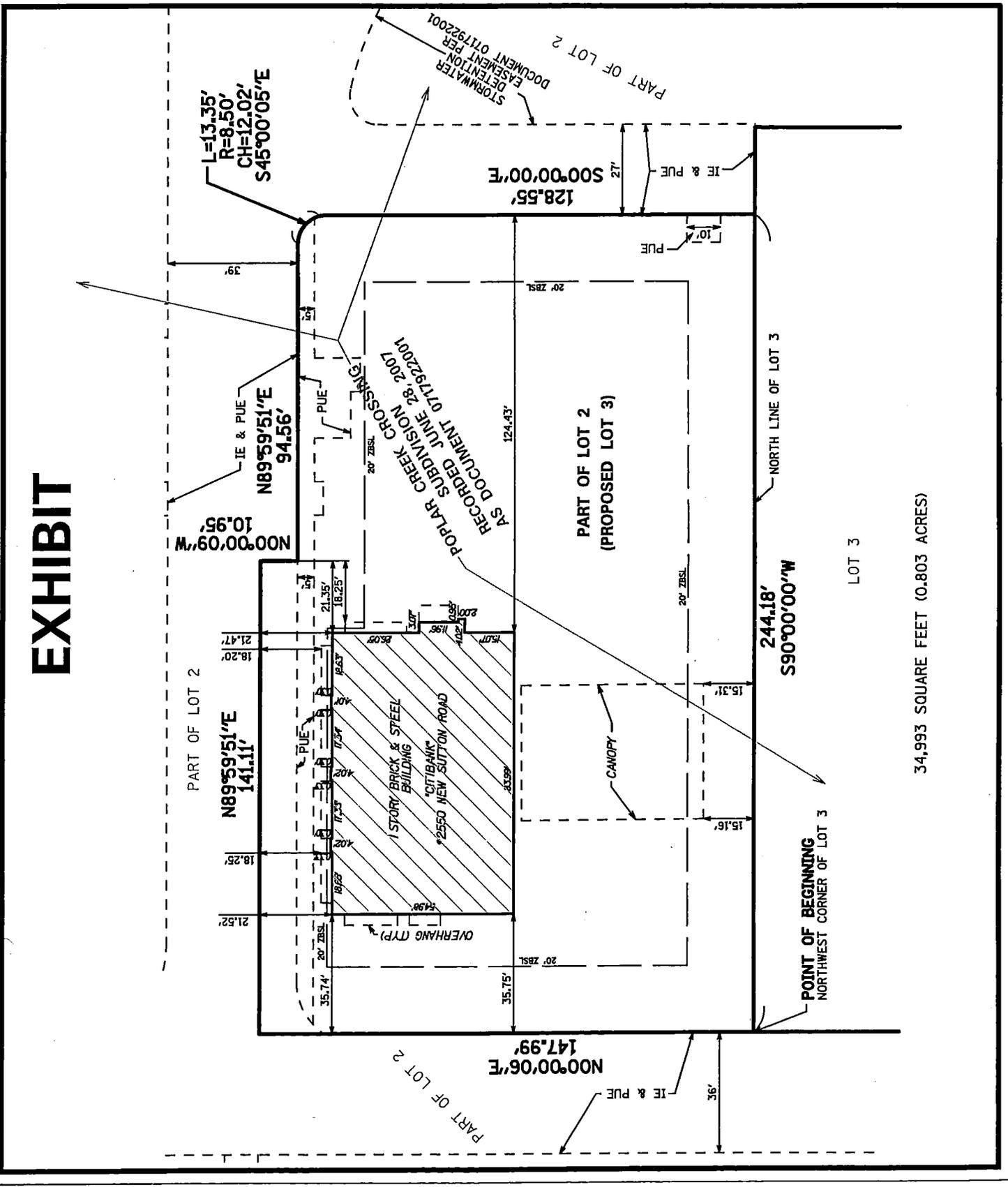
EXHIBIT

DATE: 08/12/09
 JOB NO: 6052
 FILENAME: 6052EXB-03

CONSULTING ENGINEERS
SITE DEVELOPMENT ENGINEERS
LAND SURVEYORS
 9575 W. Higgins Road, Suite 700,
 Rosemont, Illinois 60018
 Phone: (847) 696-4060 Fax: (847) 696-4065



SCALE: 1" = 40'



34,993 SQUARE FEET (0.803 ACRES)



HOFFMAN ESTATES

GROWING TO GREATNESS

November 20, 2009

To All Interested Parties:

Please be advised that the Village of Hoffman Estates will conduct public reviews at the request of W2001 VHE Realty, LLC for to consider approvals for a plat of resubdivision, site plan amendment, and variations on the property commonly known as the Poplar Creek Crossing Shopping Center and generally located at the southwest corner of Higgins Road and Sutton Road.

Two separate public reviews will be held for this property.

- The Plan Commission will conduct a public meeting to consider a plat of resubdivision of Lot 2 and 3 of the Poplar Creek Crossing subdivision and also a site plan amendment for the Poplar Creek Crossing Shopping Center, Citibank (2550 N. Sutton Road), Fifth Third Bank (4653 W. Higgins Road), and Buffalo Wild Wings (2540 N. Sutton Road). **The meeting will be conducted at 8:00 p.m. on Wednesday, December 2, 2009 in the Helen Wozniak Council Chambers of the Village Hall, 1900 Hassell Road.**
- The Zoning Board of Appeals will conduct a public hearing to consider variations from the Zoning Code to permit a setback variation for the existing Citibank facility on the property known as 2550 N. Sutton Road. **The hearing will be conducted at 7:30 p.m. on Tuesday, December 8, 2009 in the Helen Wozniak Council Chambers of the Village Hall, 1900 Hassell Road.**

A map designating the subject site is included on the back of this letter. We welcome your attendance and/or testimony in the subject reviews. Appropriate time will be available for questions/comments from interested parties.

Plans for this proposal are available for your review at the Village Hall, Monday through Friday from 8:30 a.m. to 5:00 p.m. Should you wish to review the plans or would like additional information prior to the public meetings, please contact me at (847)781-2670. Should it become necessary to continue either of these meetings to an additional date, it will be announced at the conclusion of testimony on December 2nd and/or December 8th. No further notification of these reviews will be sent.

Sincerely,

Brian Portz, Associate Planner
Department of Development Services

BP/pm

1900 Hassell Road
Hoffman Estates, Illinois 60169
www.hoffmanestates.org

Phone: 847-882-9100
Fax: 847-843-4822

William D. McLeod
MAYOR

Raymond M. Kincaid
TRUSTEE

Gary J. Pilafas
TRUSTEE

Karen V. Mills
TRUSTEE

Jacquelyn Green
TRUSTEE

Bev Romanoff
VILLAGE CLERK

Cary J. Collins
TRUSTEE

Anna Newell
TRUSTEE

James H. Norris
VILLAGE MANAGER



4800

4787 4785 4783

HIGGINS RD

29 W 240

OLD SUTTON RD

Poplar Creek Crossing S.C.

SUTTON RD

SE BLV

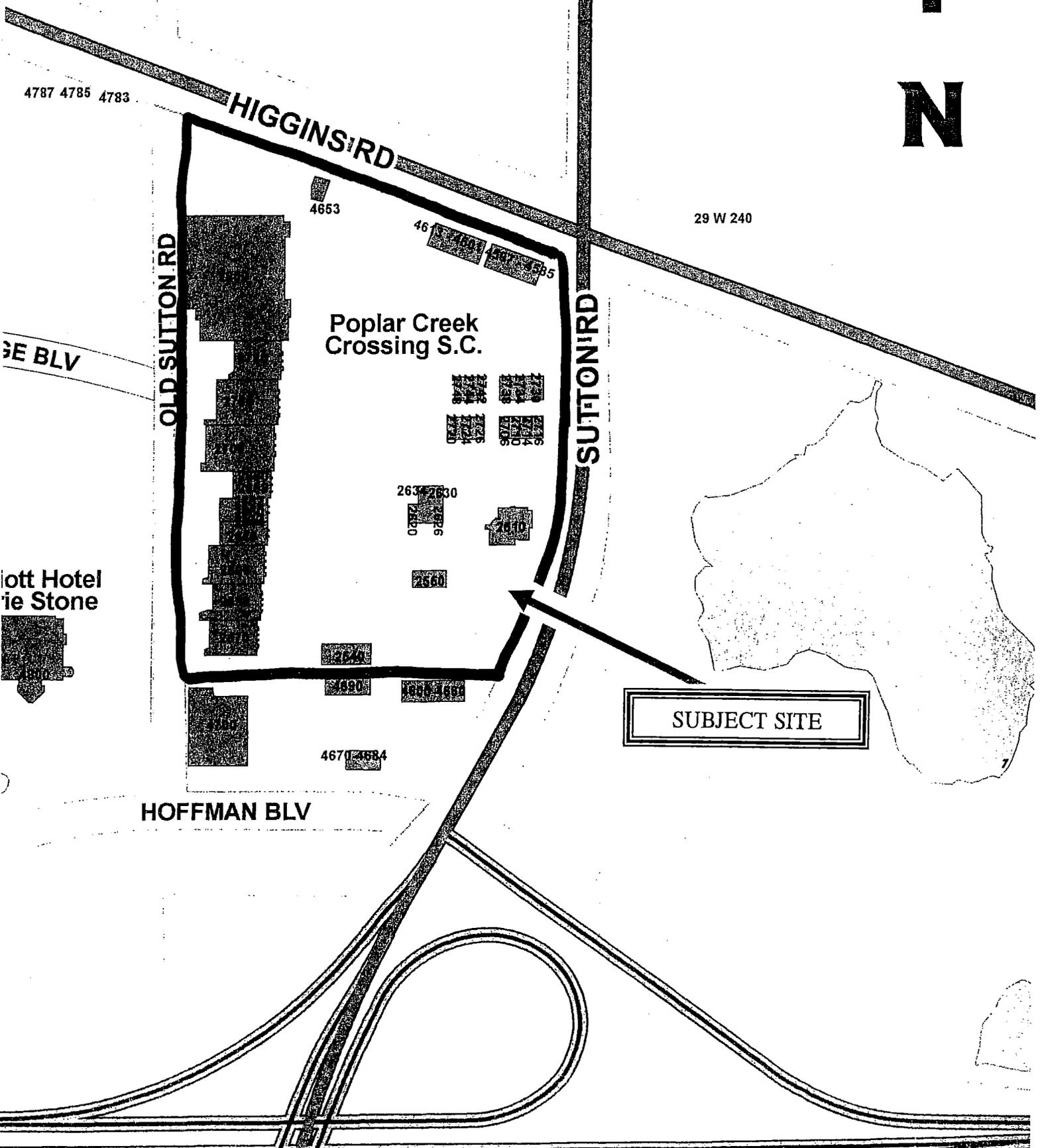
Watt Hotel
Pie Stone

2637 2630

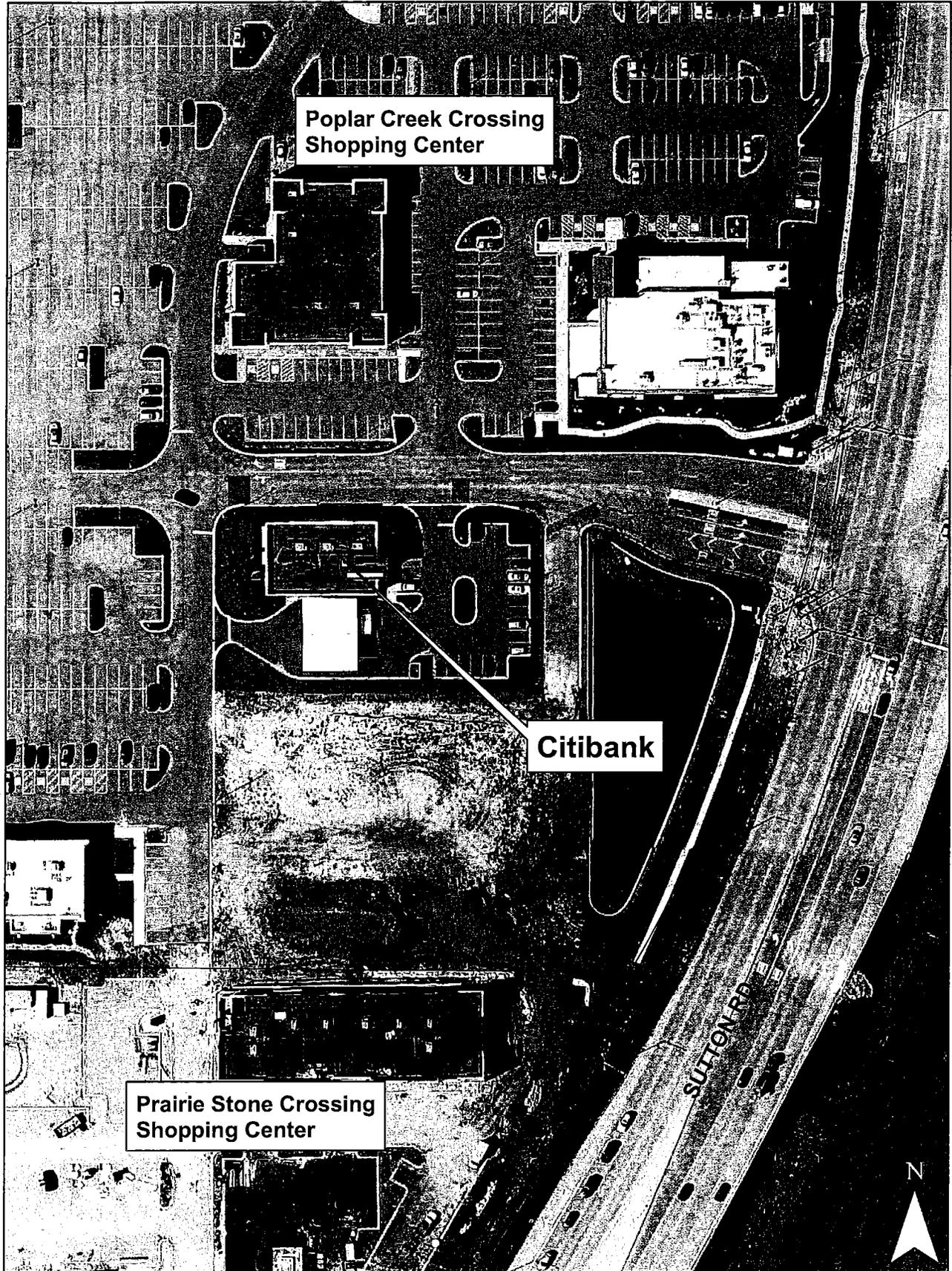
SUBJECT SITE

HOFFMAN BLV

4670 4684



Citibank - 2550 N. Sutton Road



1 inch = 100 feet

Department of Development Services
Planning Division
December 2009

VILLAGE OF HOFFMAN ESTATES

Memo

TO: President & Board of Trustees
FROM: Gary Stanton, Plan Commission Chairman
RE: **REQUEST BY W2001 VHE REALTY, LLC, AND RUBINA REALTY CORPORATION FOR:**

A. PRELIMINARY AND FINAL PLAT OF RESUBDIVISION OF LOT 2 AND 3 OF POPLAR CREEK CROSSING SUBDIVISION

REQUEST BY W2001 VHE REALTY, LLC, FOR:

B. SITE PLAN AMENDMENT TO THE POPLAR CREEK CROSSING SHOPPING CENTER; AND

C. SITE PLAN AMENDMENT TO THE CITIBANK DEVELOPMENT AT THE POPLAR CREEK CROSSING SHOPPING CENTER; AND

D. SITE PLAN AMENDMENT TO THE FIFTH THIRD BANK DEVELOPMENT AT THE POPLAR CREEK CROSSING SHOPPING CENTER; AND

E. SITE PLAN AMENDMENT TO THE BUFFALO WILD WINGS RESTAURANT AT THE POPLAR CREEK CROSSING SHOPPING CENTER

FINDING OF FACT

DATE: December 11, 2009

Plan Commission Hearing Date: December 2, 2009
P.C. #2009020, #2009023, #2009024, and #2009025

REQUEST

Request by W2001 VHE Realty, LLC and Rubina Realty Corporation for:

A. Preliminary and final plat of resubdivision of Lot 2 and 3 of Poplar Creek Crossing Subdivision.

Request by W2001 VHE Realty, LLC for:

B. Site plan amendment to the Poplar Creek Crossing Shopping Center.

- C. Site plan amendment to the Citibank development at the Poplar Creek Crossing Shopping Center.
- D. Site plan amendment to the Fifth Third Bank development at the Poplar Creek Crossing Shopping Center.
- E. Site plan amendment to the Buffalo Wild Wings restaurant at the Poplar Creek Crossing Shopping Center.

BACKGROUND

The Poplar Creek Crossing Shopping Center received final Village Board site plan approval in April 2005. The approval included a Target store and several mid-sized retail tenants, several small tenant retail buildings, and conceptual building pad sites for a freestanding restaurant and two banks. Subsequently, final site plan approval was granted by the Village Board for a Claim Jumper restaurant, Citibank, Fifth Third Bank, and a Buffalo Wild Wings restaurant.

The shopping center is currently subdivided into three lots that contain the Target store and its parking lot, the rest of the retail buildings and parking lots, and a vacant 1.7 acre lot (south of the shopping center) that was retained by the previous property owner for future development. The entire shopping center is designed with common access, parking, detention, etc. There are access drives located on Sutton Road, Higgins Road, and Old Sutton Road.

As part of the development, two "future development sites" were proposed on the overall site plan. One site is located on the north side of the property and the other is located on the south side of the property. The site on the north side of the property was developed as a Fifth Third Bank and the site on the south side of the property was developed as a Citibank. These two properties are part of the shopping center property, but somewhat act as "standalone" properties with their own parking areas. The Buffalo Wild Wings, on the other hand, is located on the south side of the main shopping center parking lot and shares parking with other businesses in this area of the property.

PETITIONER PROPOSAL

Plat of Resubdivision

The petitioner is proposing to resubdivide the existing 30.67 acre site into five separate lots to permit the sale of the Fifth Third Bank, Citibank, and Buffalo Wild Wings as separate lots. The Target store lot with its associated parking will not be changed with this proposed resubdivision. The proposed lots will be resubdivided as follows:

Lot Number	Acres	Land Use
Lot 1	26.7	All in-line retail buildings, along with the smaller retail buildings in front along Route 59
Lot 2	1.2	Fifth Third Bank and its parking lot
Lot 3	0.803	Citibank and its parking lot
Lot 4	0.47	Buffalo Wild Wings
Lot 5	1.38	Vacant land

When the plat of resubdivision was surveyed and drawn up by the engineering consultant, errors in the original plat of subdivision were discovered that consist of gaps and/or overlaps of the property lines. The errors consist of improper placement of property lines along the north, east, and south sides of the property. For example, the east property line was incorrectly drawn 4-5 feet to the east into the right of way of Route 59 on the original subdivision plat. The same type of error was noted along the north and south property lines. The plat of resubdivision will correct these errors. Because the property lines will have to move in toward the shopping center, the public utility easements and building setback lines will also have to be moved and rededicated. The Target property lines are not affected by these errors.

Due to the building setback lines moving closer to the buildings along the north and east sides of the property, the existing buildings were checked to ensure that they did not now encroach into those new setback locations. This setback analysis noted that none of the buildings will encroach into the new setback location, therefore, setback variations are not required.

Ongoing Maintenance of Shared Portions of the Overall Site

On this site, there are shared areas (driveways, parking, landscaping, lighting, etc.) that will be used by all the property owners on the lot. A private Operation and Easement Agreement that outlines the responsibilities for maintenance of the common areas exists between Target Corporation and the owners of the Poplar Creek Crossing Shopping Center. The new owner(s) of the Fifth Third Bank property, Citibank property, and Buffalo Wild Wings property will become a party to this agreement. The Village is not a party to this agreement.

The Village's main concern with the resubdivision of this property is to be able to enforce property maintenance violations upon any owner within the larger property. It is critical that the Village have a mechanism in place to quickly have violations corrected. Past experience has shown that without these conditions in place, the Village has to invest significant time trying to negotiate between property owners to have a simple problem fixed. For example, if there is a pothole on the internal driveway that connects to Route 59, the Village needs to have the authority to cite the owner of the Citibank property for the violation even though the violation is not actually on their property. In this example, the owner of the Citibank property relies on the driveway from Route 59 to provide access to their site. If there is a property maintenance violation on the remainder of the property, then the site plan for Citibank will not be in compliance with its site plan approval because that approval is reliant upon the larger property being in compliance with Village code. A condition of approval is proposed to address this issue. The Village Board recently approved a similar condition for the Saddle Room property resubdivision.

Poplar Creek Crossing Site Plan Amendment

The requested plat of resubdivision will require a site plan amendment to the Poplar Creek Crossing site. This site plan amendment will not involve construction of any buildings on the site. It is merely an amendment to the approved site plan to permit the resubdivision of the property and to allow for exceptions to the C-MU Design Guidelines and Standards that will be required as a result of this resubdivision. The property is zoned C-MU and is governed by the C-MU Design Guidelines and Standards. The waivers are necessary because of the location of the newly created property lines. The exceptions requested generally pertain to perimeter landscaping requirements in the Design Guidelines.

Because many of the new property lines around the Citibank lot, the Fifth Third Bank lot, and the Buffalo Wild Wings lot will traverse through drive aisles in the parking lot, the C-MU Design Guidelines and Standards pertaining to landscaping buffers will not be met, specifically, the C-MU Design Guidelines and Standards Section 2.10.1 that requires a 7 foot perimeter landscape buffer between abutting outlots within the same C-MU development.

The adjustment of the east property line to correct the original platting error will require an exception from the C-MU Design Guidelines and Standards pertaining perimeter landscaping buffers required adjacent to public road rights of way. The property line adjustment will result in a 15 foot landscape buffer instead of the required 20 feet. This 15 foot buffer is located from the Caribou Coffee drive-thru lane to the east property line. A 2 foot exception was approved with the Caribou drive-thru lane approval to permit an 18 foot landscape buffer at this location. There are other portions of the parking lot along the east property line that will encroach into the 20 foot landscape buffer with the adjusted property lines. These landscape buffer range in width from 16 feet to 19 feet. There are no parking lot encroachments into the landscape buffer on the north side of the property.

Citibank Site Plan Amendment

The Citibank portion of the property will be subdivided into its own lot with this proposed resubdivision. The parking lot for the Citibank building will be contained within the new lot.

Because this property will now have property lines on all four sides (instead of just one property line on the south side), the lot is considered to be a stand alone lot according to the C-MU Design Guidelines and Standards; therefore, the proposed lot was analyzed to see if it met the C-MU Design Guidelines and Standards. The following information was found in this analysis.

The proposed north property line for the Citibank property will partly traverse through the drive aisle on the north side of the building. Because of this, the C-MU Design Guidelines and Standards landscaping buffer requirement will not be met. A 7 foot landscape buffer is required along all property lines. This cannot be met on the north side of the building, therefore, an exception from the Design Guidelines is required for the north property line. A condition of approval has been recommended pertaining to this exception. The other three sides of the lot will meet Design Guidelines requirements pertaining to perimeter landscaping.

Fifth Third Bank Site Plan Amendment

The Fifth Third Bank portion of the property will be subdivided into its own lot with this proposed resubdivision. The parking lot for the Fifth Third Bank building will be contained within the new lot.

Because this property will now have property lines on all four sides (instead of just one property line on the north side), the lot is considered to be a stand alone lot according to the C-MU Design Guidelines and Standards; therefore, the proposed lot was analyzed to see if it met the C-MU Design Guidelines and Standards. The following information was found in this analysis.

The C-MU Design Guidelines and Standards require one shade tree for each 40 linear feet of perimeter landscape buffer. Based on the 1,065 linear feet for the lot, 27 shade trees would be required. Fourteen shade trees were planted on the site with the original site plan approval; therefore, an exception of 13 shade trees is required. A condition of approval has been recommended pertaining to this exception.

The proposed west and south property lines for the Fifth Third Bank property will traverse through the center of the drive aisles on the south and west sides of the property. Because of this, the C-MU Design Guidelines and Standards landscaping buffer requirement will not be met along the south and west sides. A 7 foot landscape buffer is required along all property lines. This cannot be met on the south and west sides of the property, therefore, an exception from the Design Guidelines is required for the south and west property lines. The north and east sides of the lot will meet Design Guidelines requirements.

Buffalo Wild Wings Site Plan Amendment

The Buffalo Wild Wings portion of the property will be subdivided into its own lot with this proposed resubdivision. The Buffalo Wild Wings site plan was originally approved with a shared parking concept. The restaurant shares parking with Office Max, PetSmart, and part of Michael's. The proposed property lines will be drawn such that there will only be five parking spaces on the newly create Buffalo Wild Wings lot. Through the Operation and Easement Agreement for the site, Buffalo Wild Wings will still have access to the rest of the parking spaces that the original site plan approval allowed them to use. Nothing will change as a result of this proposed resubdivision - only new lot lines will be created. The use of the parking lot will remain the same.

Because this property will now have property lines on all four sides (instead of just one property line on the south side), the lot is considered to be a stand alone lot according to the C-MU Design Guidelines and Standards; therefore, the proposed lot was analyzed to see if it met the C-MU Design Guidelines and Standards. The following information was found in this analysis.

The C-MU Design Guidelines and Standards require one shade tree for each 40 linear feet of perimeter landscape buffer. Based on the 586 linear feet for the lot, 15 shade trees would be required. Eight shade trees were planted on the site with the original site plan approval; therefore, an exception of seven shade trees is required. A condition of approval has been recommended pertaining to this exception.

The proposed north, east, and west property lines for the Buffalo Wild Wings property will traverse through the drive aisles on the north side of the building and through the parking spaces on the west side of the property. Also, on the east side of the property, the proposed property line will be located directly adjacent to a sidewalk. Because of this, the C-MU Design Guidelines and Standards landscaping buffer requirement will not be met along the north, east, and west sides. A 7 foot landscape buffer is required along all property lines. This cannot be met on the north, east, and west sides of the property; therefore, an exception from the Design Guidelines is required for the north, east, and west property lines. A condition of approval has been recommended pertaining to this exception. The south side of the lot will meet Design Guidelines requirements.

Development Services Comments

When the site plans were originally approved for the Poplar Creek Crossing Shopping Center and each of the future development sites (Fifth Third Bank, Citibank, and Buffalo Wild Wings), it was with the understanding that this property would be subdivided into three separate lots - the Target lot, the Poplar Creek Crossing lot, and the vacant lot on the southeast corner of the site. It was understood that the Poplar Creek Crossing lot, which contains most of the Center's buildings, would act as one cohesive property with single ownership. At the time of site plan approval for each development, the individual sites (Fifth Third Bank, Citibank, and Buffalo Wild Wings) were designed to meet Village code requirements for landscaping, etc. with the idea that this entire site acted as one large development, not separate developments on the site. With this in mind, the landscaping on each individual site was designed to coordinate with the rest of the overall site. If the shopping center property were subdivided into individual lots when the site plan for this site was originally approved, each outlot would have been required to have additional landscaping, as required by Village code. When new lot lines are drawn after a site has been approved and planted with the approved landscaping, the landscaping requirements usually cannot be met, therefore, exceptions to the landscaping requirements are required.

PLAN COMMISSION DISCUSSION

On December 2, 2009, the Plan Commission met to hear a request by W2001 VHE Realty, LLC and Rubina Realty Corporation for a preliminary and final plat of resubdivision of Lot 2 and 3 of Poplar Creek Crossing subdivision. At the same time, W2001 VHE Realty, LLC had four additional requests for site plan amendments to 1) Poplar Creek Crossing Shopping Center, 2) Citibank development at the Poplar Creek Crossing Shopping Center, 3) Fifth Third Bank development at the Poplar Creek Crossing Shopping Center, and 4) Buffalo Wild Wings Restaurant at the Poplar Creek Crossing Shopping Center.

The Plan Commission learned that the Poplar Creek Crossing Shopping Center is currently subdivided into three lots. The petitioner proposed resubdividing the existing 30.67 acre site into four separate lots to permit the sale of the Fifth Third Bank, Citibank, and Buffalo Wild Wings lots. The petitioner indicated to the Plan Commission there were agreements in place for the sale of the lots to both Fifth Third Bank and Citibank. No agreement, as yet, has been made between the petitioner and a prospective buyer of the Buffalo Wild Wings lot. The current Buffalo Wild Wings lease (20 years, with approximately 17 years left) would stay in place for the new owner of the lot.

The Plan Commission's major concern regarding the petitioner's requests were in relation to each of the three lots expected to be sold did not meet some of the C-MU Design Guidelines and Standards. If the lots came before the Plan Commission for approval on their own, they would not have met some of the C-MU Design Guidelines and Standards for landscaping. The Buffalo Wild Wings lot will only have five parking spaces on the lot itself. The petitioner did indicate that it had access agreements in place regarding all the parking that would allow owners of any of the lots to park on each other's lots in perpetuity.

Village staff voiced a concern when questioned regarding the enforcement of property maintenance violations. Staff indicated that it was essential the Village have a mechanism in place to quickly have violations corrected. The petitioner indicated that it would have agreements in place between all the parties regarding maintenance responsibilities. The petitioner also agreed to the revision of the first two conditions, as presented to the Plan Commission on December 2, 2009, by Village staff.

In conjunction with the maintenance issue, the petitioner was asked how the sale of the lots would affect the snow removal. The petitioner indicated the service they have now would remain in effect for all lots.

After all questions and concerns were voiced, the Plan Commission voted 4-2 in favor of all of the petitioner's requests.

AUDIENCE PARTICIPATION

None.

RECOMMENDATION

A. Approval of a request by W2001 VHE Realty, LLC. and Rubina Realty Corporation for preliminary and final plat of resubdivision of Lot 2 and 3 of Poplar Creek Crossing Subdivision, subject following conditions:

1. Each lot within Poplar Creek Crossing Resubdivision #1, with the exception of lot 5, shall be responsible for the maintenance of all shared areas within the resubdivision, including, but not limited to, parking lot/drive aisle pavement, exterior lighting, landscaping, sidewalks, etc. Site plan approvals on lots 1, 2, 3, and 4 shall only be in compliance if all site improvements within the resubdivision are in compliance with Village code. Any property covered by this condition shall be considered to be in violation of their site plan approval so long as a violation exists on any portion of the lots covered by this condition.
2. Petitioner W2001 VHE Realty, LLC and the successor owners of lots 2, 3, and 4 shall maintain valid agreements to be legally binding upon the owners of lots 1, 2, 3, and 4. The agreement(s) shall address shared maintenance, parking, lighting, landscaping, signs, and access issues, and give each lot owner the authority to correct violations on any other property within the resubdivision. Each site plan approval on lot 1, 2, 3, and 4 shall only be valid if the agreement(s) are in place.
3. Approval of Poplar Creek Crossing resubdivision #1 shall be subject to setback variation approval on the Citibank lot.

B. Approval of a request by W2001 VHE Realty, LLC for a site plan amendment to the Poplar Creek Crossing Shopping Center, subject to the following conditions:

1. Each lot within Poplar Creek Crossing Resubdivision #1, with the exception of lot 5, shall be responsible for the maintenance of all shared areas within the resubdivision, including, but not limited to, parking lot/drive aisle pavement, exterior lighting, landscaping, sidewalks, etc. Site plan approvals on lots 1, 2, 3, and 4 shall only be in compliance if all site improvements within the resubdivision are in compliance with Village code. Any property covered by this condition shall be considered to be in violation of their site plan approval so long as a violation exists on any portion of the lots covered by this condition.
2. The following exceptions are granted for the Poplar Creek Crossing property only:
 - a. An exception is granted to Section 2.10.1 of the C-MU Design Guidelines and Standards which pertains to the perimeter landscaping requirement, specifically, the requirement of a 7 foot landscape buffer between Lot 1 and the proposed Lot 2, 3, and 4. A 0 foot landscape buffer is granted due to the property lines traversing through the existing drive aisles and parking lots on the property.
 - b. An exception is granted to Section 2.10.1 of the C-MU Design Guidelines and Standards pertaining to the perimeter landscaping requirement of 20 feet abutting a public road right of way. A minimum 15 foot landscape buffer is granted along the east side along Route 59.
3. All conditions of approval of the original site plan approval for the Poplar Creek Crossing Shopping Center shall remain in effect.

C. Approval of a request by W2001 VHE Realty, LLC for a site plan amendment to the Citibank development at the Poplar Creek Crossing Shopping Center, subject to the following conditions:

1. Each lot within Poplar Creek Crossing Resubdivision #1, with the exception of lot 5, shall be responsible for the maintenance of all shared areas within the resubdivision, including, but not limited to, parking lot/drive aisle pavement, exterior lighting, landscaping, sidewalks, etc. Site plan approvals on lots 1, 2, 3, and 4 shall only be in compliance if all site improvements within the resubdivision are in compliance with Village code. Any property covered by this condition shall be considered to be in violation of their site plan approval so long as a violation exists on any portion of the lots covered by this condition.

2. The following exception is granted for the Citibank property only:
 - a. An exception is granted to Section 2.10.1 of the C-MU Design Guidelines and Standards which pertains to the perimeter landscaping requirement, specifically, the requirement of a 7 foot landscape buffer on the north side of the building. A 0 foot landscape buffer is granted due to the property line traversing through the existing drive aisle north of the building.
3. All conditions of approval of the original site plan approval for Citibank shall remain in effect.

D. Approval of a request by W2001 VHE Realty, LLC for a site plan amendment to the Fifth Third Bank development at the Poplar Creek Crossing Shopping Center, subject to the following conditions:

1. Each lot within Poplar Creek Crossing Resubdivision #1, with the exception of lot 5, shall be responsible for the maintenance of all shared areas within the resubdivision, including, but not limited to, parking lot/drive aisle pavement, exterior lighting, landscaping, sidewalks, etc. Site plan approvals on lots 1, 2, 3, and 4 shall only be in compliance if all site improvements within the resubdivision are in compliance with Village code. Any property covered by this condition shall be considered to be in violation of their site plan approval so long as a violation exists on any portion of the lots covered by this condition.
2. The following exceptions are granted for the Fifth Third Bank property only:
 - a. An exception is granted to Section 2.10.1 of the C-MU Design Guidelines and Standards which pertains to the perimeter landscaping requirement, specifically, the requirement of a 7 foot landscape buffer on the west and south sides of the property. A 0 foot landscape buffer is granted due to the property line traversing through the existing drive aisle west and south of the building.
 - b. An exception is granted to Section 2.10.2 of the C-MU Design Guidelines and Standards which pertains to the perimeter shade tree requirement, specifically the requirement of one shade tree for each 40 linear feet of perimeter landscape buffer. Based on the 1,065 linear feet, 27 shade trees would be required. Fourteen shade trees were planted on the site, therefore, an exception of 13 shade trees is granted.
3. All conditions of approval of the original site plan approval for Fifth Third Bank shall remain in effect.

E. Approval of a request by W2001 VHE Realty, LLC for a site plan amendment to the Buffalo Wild Wings restaurant at the Poplar Creek Crossing Shopping Center, subject to the following conditions:

1. Each lot within Poplar Creek Crossing Resubdivision #1, with the exception of lot 5, shall be responsible for the maintenance of all shared areas within the resubdivision, including, but not limited to, parking lot/drive aisle pavement, exterior lighting, landscaping, sidewalks, etc. Site plan approvals on lots 1, 2, 3, and 4 shall only be in compliance if all site improvements within the resubdivision are in compliance with Village code. Any property covered by this condition shall be considered to be in violation of their site plan approval so long as a violation exists on any portion of the lots covered by this condition.
2. The following exceptions are granted for the Buffalo Wild Wings property only:
 - a. An exception is granted to Section 2.10.1 of the C-MU Design Guidelines and Standards which pertains to the perimeter landscaping requirement, specifically, the requirement of a 7 foot landscape buffer on the north, west and east sides of the building. A 0 foot landscape buffer is granted due to the property line traversing through the existing drive aisle north of the building, through the parking spaces on the west side of the building, and through a sidewalk on the east side of the building.
 - b. An exception is granted to Section 2.10.2 of the C-MU Design Guidelines and Standards which pertains to the perimeter shade tree requirement, specifically the requirement of one shade tree for each 40 linear feet of perimeter landscape buffer. Based on the 586 linear feet of the perimeter of the proposed lot, 15 shade trees would be required. Eight shade trees are planted on the site, therefore, an exception of seven shade trees is granted.
3. All conditions of approval of the original site plan approval for the Buffalo Wild Wings shall remain in effect.

cc: J. Norris, M. Koplin, P. Gugliotta, M. Hankey, G. Salavitch, D. Plass, S. Neil,
B. Skowronski, Petitioner

VILLAGE OF HOFFMAN ESTATES
ZONING BOARD OF APPEALS

FINDING OF FACT

DATE OF PUBLIC HEARING(S): December 8, 2009

DATE OF PRESENTATION TO VILLAGE BOARD: December 14, 2009

PETITION: Hearing held at the request of the Village of Hoffman Estates (Lessor) and The Everest Group/Sprint, Clearwire (Lessee) to consider a special use under the Zoning Code to permit the installation of communication antennas and associated equipment to be located in an existing shelter on the property located at 4690 Olmstead Drive.

DISTRICT IN WHICH PROPERTY IS LOCATED: R-9, Planned Development District Subzone F-2

ZONING CODE SECTION(S) FOR SPECIAL USE: 9-5-9-D-9 and 9-3-9-A

FINDING-OF-FACT: The ZBA found that the Standards for a Special Use (Section 9-1-18) were met.

MOTION: Request to grant the Village of Hoffman Estates (Lessor) and The Everest Group/Sprint, Clearwire (Lessee), *a special use under Sections 9-5-9-D-9 and 9-3-9-A to permit the installation of two (2) microwave dishes and associated equipment to be no greater than one hundred and thirty three (133) feet high on a Village water tank at 4690 Olmstead Drive.*

The following conditions shall apply:

1. This special use shall be subject to approval of the final lease agreement with the Village of Hoffman Estates.
2. No advertising shall be allowed on the equipment or structures.
3. The petitioner shall pay all costs associated with the third party review and inspections, as required by the Village's Public Works Department policy.
4. Should the operation of these microwave dishes cease for a period of six (6) months, the dishes and associated equipment shall be removed per Zoning Code Section 9-1-18-L.

The petitioner was agreeable to the above listed conditions.

RECOMMENDATION: The Zoning Board of Appeals (ZBA) recommends approval of this request.

The petitioner, Mr. Faber representing Sprint-Clearwire, requested a special use to install two microwave dish antennas on the water tank. The dishes operate under a different radio band from the existing cellular antennas and are used to carry the increased volumes of data (video

and internet) common in modern cell phones. The Zoning Board confirmed that the structural report was completed and was accepted by the Village. The Zoning Board confirmed that each dish weighs approximately 45 pounds including the mounting equipment. The Zoning Board recommended approval of the request.

AUDIENCE COMMENTS

For the benefit of the audience, Chairman Weaver read the following into the record: The Federal Communications Act of 1996 expressly pre-empts local governments from regulating the placement, construction or modification of personal wireless services on the basis of environmental or health issues. Such concerns cannot be discussed or used as a basis of the decision whether to grant a special use.

Alice Kot, of 4695 Olmstead, inquired when the installation would occur and Mr. Faber replied that the installation takes typically 1-2 days and occurs during the daytime, and not during inclement weather. Ms. Kot inquired about the difference between these new dishes and the existing cellular antennas, and Mr. Faber replied that microwave dishes have been in use since the 1950s and these dishes will be used to transmit data between dishes at different sites, unlike the panel antennas that provide voice coverage. The new dishes will not increase the geographic coverage area, but will improve existing coverage. The dishes and equipment will not create noise onto surrounding properties.

VOTE:

7 Ayes
0 Nays
0 Absent

ZONING BOARD OF APPEALS

Chairman William Weaver
Vice-Chairman Ronald Jehlik
Denise Wilson
Michael Ciffone
Masoom Ali
Donna Boomgarden
Michael Gaeta

*** IMMEDIATE AUTHORIZATION TO APPLY FOR PERMITS IS REQUESTED ***

**THIS SPECIAL USE WILL EXPIRE UNLESS ACTED UPON WITHIN ONE (1) YEAR
OF VILLAGE BOARD APPROVAL**

FINDING OF FACT WRITTEN BY DEVELOPMENT SERVICES STAFF

VILLAGE OF HOFFMAN ESTATES

Memo

TO: William Weaver, Zoning Board of Appeals Chairman
FROM: Josh Edwards, Assistant Planner
RE: SPRINT-CLEARWIRE COMMUNICATIONS - 4690 OLMSTEAD DRIVE - SPECIAL USE - COMMUNICATION ANTENNAS AND ASSOCIATED EQUIPMENT
DATE: December 3, 2009
HEARING DATE: December 8, 2009

1. REQUEST SUMMARY

Request by Village of Hoffman Estates (owner) and Sprint-Clearwire Communications (lessee) for a special use to permit the installation of two microwave dishes and associated equipment on a Village owned water tank at 4690 Olmstead Drive.

2. BACKGROUND

The existing water tank at 4690 Olmstead Drive currently contains communication antennas installed by Sprint, Verizon, T-Mobile, and Cricket. One set of existing antennas is mounted on the neck of the structure at a height of 70 feet and the other sets are mounted on top of the tank at 130, 138, and 145 feet. The equipment for the existing antennas is located near the base of the tank in two adjacent shelters to the north (Cricket and T-Mobile), in a shelter to the northeast (Verizon), and in a shelter to the west (Sprint).

3. PETITIONER PROPOSAL

The petitioner is proposing to install two, 30 inch high microwave dishes at 133 feet above grade on top of the bulb of the water tank. The installation requires a special use approval. The two dishes would be attached to the existing array of Sprint cellular antennas located at 130 feet high, which includes 9 antennas.

The function of the new dishes is to support and upgrade the Sprint network with voice and data transmission as an alternative to installing miles of underground fiber optic wires throughout the coverage area.

No separate building or equipment is proposed on the ground. The new equipment for the dishes would be installed entirely within the existing brick equipment shelter located west of the water tank. Wiring would be installed underground between the shelter and water tank; and within the water tank to the antenna array.

4. SITE CONDITIONS

The subject property is zoned R-9 Planned Development District (Subzone F-2 Business). The properties to the south are zoned R-9 (Subzone "D" and contains townhomes). The properties to the east across Olmstead Drive are zoned R-9 (Subzones "C" and "D" and contains single family homes). The properties to the north and northwest are zoned R-9 (Subzone F-2 Business and contains the Rose Plaza Shopping Center and commercial outbuildings including a bank and daycare).

5. APPLICABLE REQUIREMENTS

- a) Section 9-5-9-D-9 (p. CD9:87) regarding antennas and communication towers in the R-9 District refers to Section 9-3-9.
- b) Section 9-3-9-A (p. CD9:58) states that the total antenna height of any communications tower, antenna or combination thereof of any height over 45 feet upon Village owned or leased facilities shall be permitted only as a special use.

6. ADJACENT OWNER COMMENTS

Standard notification letters have been mailed and as of this writing no comments have been received.

7. RELEVANT SPECIAL USE HISTORY

Subject Property

- a) Ordinance No. 4067-2008 was granted to allow Cricket Communications to install 3 antennas at a height of 138 feet.
- b) Ordinance No. 3265-2001 was granted to allow Voicestream Wireless (T-Mobile) to install 12 antennas at a height of 145 feet. The existing black wrought iron fence around the water tank site was constructed at this time, which replaced a chain link fence.

- c) Ordinance No. 2978-1998 was granted to allow Sprint PCS to install 9 antennas. This ordinance was replaced by Ordinance No. 3023-1998, which was granted to allow Sprint PCS to install 9 antennas at a height of 130 feet.
- d) Emergency radio dispatch antennas as part of the Northwest Central Dispatch System were installed on the water tank in 1998, which did not require a special use.
- e) Ordinance No. 2910-1997 was granted to allow Ameritech (Verizon Wireless) to install 9 antennas at a height of 70 feet. This ordinance was replaced by Ordinance No. 3670-2004, which was granted to allow Verizon Wireless to install 12 antennas at a height of 116 feet. The existing Verizon Wireless antennas are currently installed at 70 feet.

Similar Properties

Over the past decade various communication antennas have been installed on all 6 water tanks throughout the Village.

8. PUBLIC WORKS COMMENTS

A thorough structural analysis and engineering review is required prior to the zoning review for Village owned water tanks. A structural analysis prepared by a licensed engineer on behalf of the petitioner has been reviewed by Public Works and the Village's third party engineering consultants. The structural analysis concluded that the installation will have no adverse impact on the water tank. Public Works has accepted the report.

9. IMMEDIATE AUTHORIZATION TO APPLY FOR PERMIT(S)

The petitioner has requested an Immediate Authorization to Apply for Permit(s).

10. DEVELOPMENT SERVICES COMMENTS

The proposal to install dishes onto a water tank rather than erecting a separate antenna tower will minimize visual clutter. The proposed dishes are designed to upgrade the existing antennas. The height and shape of the water tank and the relatively small size of the two dishes will reduce the visibility of the dishes from surrounding properties. The dish is smaller than standard cellular antenna arrays and is comparable in size to a residential television satellite dish and no more aesthetically obtrusive.

The equipment on the ground supporting the new dishes will be located entirely within an existing brick equipment shelter.

The existing special use approval and lease for the existing 9 antennas will stay in effect. A new lease will be required for this new installation.

The Federal Telecommunications Act of 1996 expressly preempts local governments from regulating the placement, construction, or modification of personal wireless services on the basis of environmental or health concerns. Such concerns cannot be discussed or used as a basis of the decision whether to grant a special use.

11. MOTION

Should the Zoning Board find that the Standards for a Special Use are met, the following motion is provided with conditions:

A special use under Sections 9-5-9-D-9 and 9-3-9-A to permit the installation of two (2) microwave dishes and associated equipment to be no greater than one hundred and thirty three (133) feet high on a Village water tank at 4690 Olmstead Drive.

The following conditions shall apply:

- 1. This special use shall be subject to approval of the final lease agreement with the Village of Hoffman Estates.*
- 2. No advertising shall be allowed on the equipment or structures.*
- 3. The petitioner shall pay all costs associated with the third party review and inspections, as required by the Village's Public Works Department policy.*
- 4. Should the operation of these microwave dishes cease for a period of six (6) months, the dishes and associated equipment shall be removed per Zoning Code Section 9-1-18-L.*

cc: Corporation Counsel, D. O'Malley, D. Plass, R. Norton, Petitioner

VILLAGE OF HOFFMAN ESTATES
ZONING BOARD OF APPEALS

REQUEST FOR SPECIAL USE HEARING

FOR VILLAGE USE ONLY

Hearing Fee \$ 400.00 Date Paid 7/8/09 Received By P. Moore
Hearing Date: 12/8/09 Time: 7:30 pm Legal Published
11/23/09
Receipt Number 293400 Check No. 104883 Zoning District R-9, F-2

PLEASE PRINT OR TYPE

Village of Hoffman Estates

1. Name of Property Owner(s)* _____
E-Mail Address joshua.edwards@hoffmanestates.org Fax 847-490-6868
1900 Hassell Road Phone 847-882-9100
Owner's Address _____
Hoffman Estates State IL Zip 60169
City _____

Subject Property's Address (if different than #1): 4690 Olmstead

2. Person applying if other than owner:*
Name Will Faber Company The Everest Group/ Sprint, Clearwire
E-Mail Address wfaber85@gmail.com Fax 773/409-5426
Address 7013 W. 111th St. Phone 773-987-5299
City Worth State IL Zip 60482
02-19-119-052-8001

3. Property Index Number (PIN) _____

5. Please describe the proposed use, or attach a letter.

Addition of 2 dish antennas to existing telecommunications facility

* If "owner" is an entity other than an individual(s), then an Economic Disclosure Statement must also be filed.

6. **An Immediate Authorization to Apply for Permits** allows the Code Enforcement Division to begin the building permit review process prior to the adoption of the Ordinance by the Village Board. The Immediate Authorization makes it possible for you to expedite the plan review process by approximately two weeks. Contact the Code Enforcement Division at 847/781-2631 to discuss the building permit application and review process.

Please check one of the following

Yes, I request Immediate Authorization to Apply for Permits upon approval of my application by the Village Board, allowing me to begin the building permit review process prior to adoption of the Ordinance approving my special use.

Or

No, I do not request Immediate Authorization to Apply for Permits.

7. I, the undersigned, certify the information and submissions provided accurately represent the current conditions and proposed improvement(s) requiring a special use.

Owner's Signature _____

Name (Please Print) _____

Applicant's Signature  _____

Name (Please Print) William Fok _____

All requests for a hearing must be accompanied by the items required according to the nature of the request. All fees must be paid before Zoning Board can hear any case. Any additional fees must be paid before any findings or reports are given to the Village Board.



THE EVEREST GROUP
Representing Sprint/ Clearwire

7013 W 111th St
Worth, IL 60482

773-987-5299

wfaber85@gmail.com

July 8, 2009
Village of Hoffman Estates
1900 Hassell Road
Hoffman Estates, IL 60169

This application for a special use permit on behalf of Sprint/ Clearwire is for the purpose of installing two additional antennas and two lines of associated coaxial cable on the water tank at 4690 Olmstead Drive. The purpose of the communications system upgrade is to facilitate wireless backhaul coverage over the existing Sprint telecommunications network. Effectively, the current T1 system will be replaced with wireless connections between individual communication sites and the central switch. The new antenna will make this linkage possible, thereby lowering costs to consumers and increasing the reliability and robustness of the existing and future data network.

Sincerely yours,

Will Faber

The Everest Group

DEPARTMENT OF PUBLIC WORKS

Memo

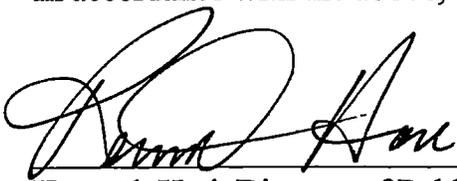
TO: Don Plass, Director of Code Enforcement
FROM: Kenneth Hari, Director of Public Works
RE: CLEARWIRE ANTENNA INSTALLATION
DATE: October 27, 2009

Urgent For Review Please Comment Please Reply Enclosure(s)

I have reviewed the proposed projects, including the upgrade load evaluations report issued by Robert Wozniak and the plan review completed by SEH, and concluded that the installation will have no adverse impact on the respective tower located at Olmstead Drive. Accordingly, a construction permit may be issued for the Olmstead Drive facility subject to the following:

- *Before construction may begin, the installation contractor must attend a pre-construction meeting with Public Works Staff.*
- *During construction, the tower site at ground level must be clean at all times with no equipment or construction materials stored on site at any time.*

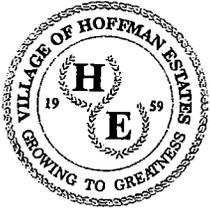
In accordance with the above, an antenna installation permit(s) may be released.



Kenneth Hari, Director of Public Works

Dir/Corr09/ClearwireAntennaInstall 102709

cc: w/o Attachments
Joseph Nebel, Superintendent of Operations
Haileng Xiao, Superintendent of Water & Sewer
Josh Edwards, Planner



HOFFMAN ESTATES

GROWING TO GREATNESS

November 25, 2009

To All Interested Parties:

Please be advised the Zoning Board of Appeals of the Village of Hoffman Estates will conduct a public hearing at the request of The Everest Group/Sprint, Clearwire (Lessee) and the Village of Hoffman Estates (Owner) to consider a special use under the Zoning Code to permit the installation of two dish antennas and associated equipment to be located in an existing shelter on the property located at 4690 Olmstead Drive.

The hearing will be held in the Municipal Building, 1900 Hassell Road, Hoffman Estates, Illinois, **Tuesday, December 8, 2009 at 7:30 p.m.**

A map designating the subject site is included on the back of this letter. Appropriate time will be available for questions/comments from interested parties. Should you wish any additional information, please feel free to contact the Planning Division at 847/781-2660. Material is available at the office for review.

Should it become necessary to continue this hearing to an additional date, it will be announced at the conclusion of testimony at the hearings. No further notification of this review will be sent.

Sincerely,

Josh Edwards, Assistant Planner
Department of Development Services

JE/pm

1900 Hassell Road
Hoffman Estates, Illinois 60169
www.hoffmanestates.org

Phone: 847-882-9100
Fax: 847-843-4822

William D. McLeod
MAYOR

Raymond M. Kincaid
TRUSTEE

Gary J. Pilafas
TRUSTEE

Karen V. Mills
TRUSTEE

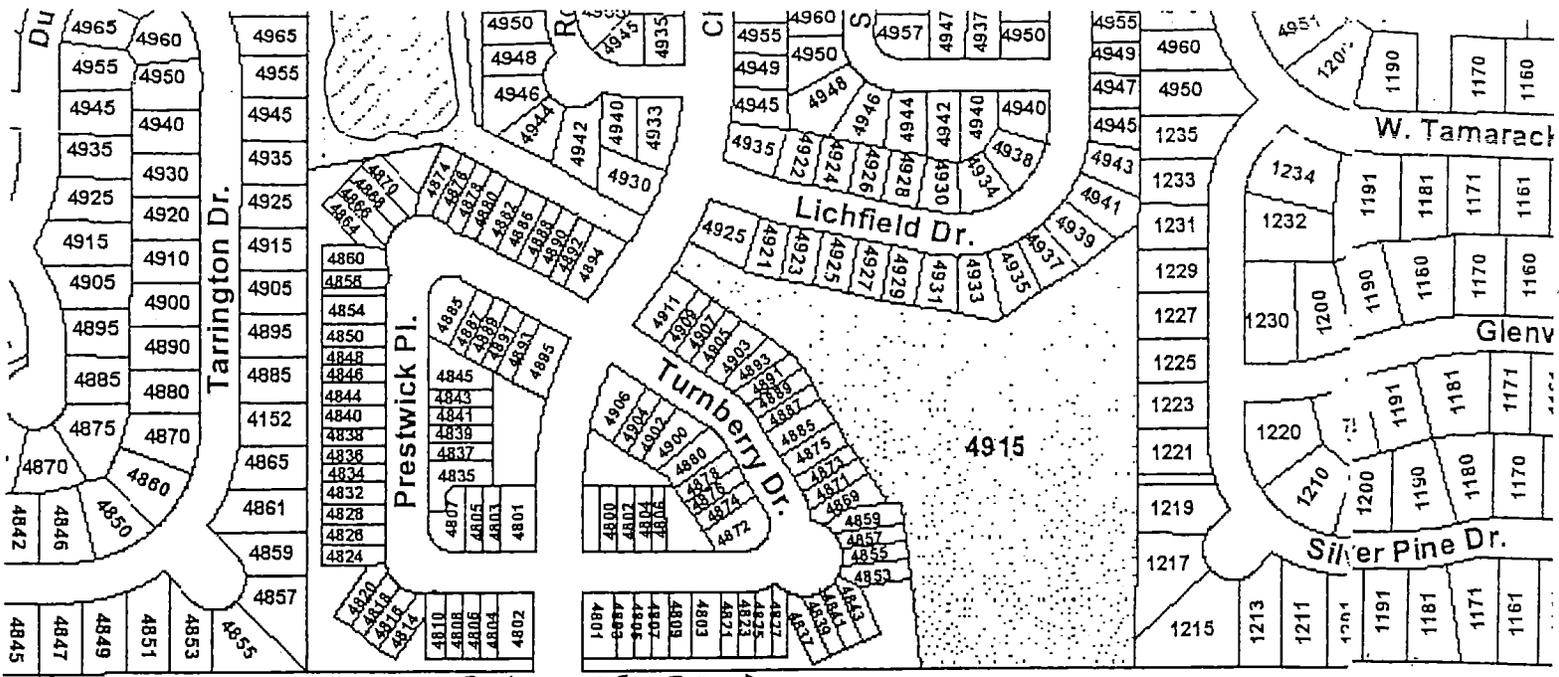
Jacquelyn Green
TRUSTEE

Bev Romanoff
VILLAGE CLERK

Cary J. Collins
TRUSTEE

Anna Newell
TRUSTEE

James H. Norris
VILLAGE MANAGER



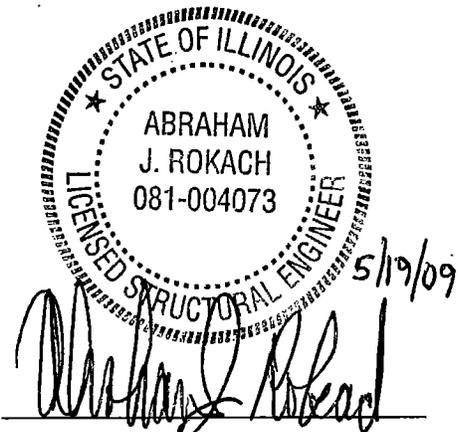
Structural Report

Prepared for: Clear Wireless LLC.

Existing 125 ft. Pedisphere

Clearwire Site No. IL-CHI5444A (IL7171)
Hoffman Estates 5
4690 Olmstead Drive
Hoffman Estates, Cook Co., IL 30195

May 19, 2009



Abraham J. Rokach S.E.

Illinois SE License No. 081-04073
Expires 11/30/2010

I certify that this report was prepared by me, or under my direct supervision and control, and, to the best of my knowledge and belief, complies with the requirements of the applicable building code.



Project Summary

Scope:

Structural analysis of existing 125 ft. water tower under proposed antenna configuration.

Design Criteria:

1. International Building Code, 2006 Edition
2. AWWA-D100-05
3. TIA/EIA-222-F 1996

New Equipment Loads:

Antenna: (1) - 2 ft. Dish @ centerline 131' AGL
(1) - 2.5 ft. Dish @ centerline 131' AGL
(2) - 2" Diameter Innerduct w/ cables

Design Loads:

Basic Wind Speed 76 mph (fastest mile) / 90 mph (3-second gust), for appurtenance, per TIA/EIA-222-F 1996.

100 mph (fastest mile) / 120 mph (3-second gust), for water tower structure, per ANSI/AWWA D100-96.

Reference:

Water Tower Drawing, by CB&I, contract no. 871774, dated 1978.

Conclusion:

The structural analysis shows that new dish antenna installation will increase the stresses on the existing water tank. The structural analysis shows that the existing water tank is **adequate** for new loads. However, the anchor bolts are slightly overstressed, but still **acceptable** by engineering judgment.

Structural Analysis on Existing Water Tank

Wind Speed

Effective Wind Speed for Water Tank Supports (AWWA-D100-05)

$V_{\text{eff}} := 100\text{mph}$

Wind Speed For Antennas (TIA-EIA-222-F)

$V_{\text{app}} := 76\text{mph}$

Material Properties

Material

(36 ksi)

Modulus of Elasticity

$E := 29000\text{ksi}$

Shape Coefficient for Water Tank Structure

Effective Area Coefficient (Flat)

$C_{d_flat} := 1$

Effective Area Coefficient (Cylindrical)

$C_{d_cyl} := 0.6$

Effective Area Coefficient (Sphere)

$C_{d_sphere} := 0.5$

Water Tank Dimensions

Water Tank

Tank Capacity

$\text{Vol} := 500000\text{gal}$

Average Tank Diameter

$d_{\text{tank}} := 55.5\text{ft}$

Tank Height

$h_{\text{tank}} := 37.5\text{ft}$

Top of Roof

$H_{\text{roof}} := 123\text{ft}$

Top of Capacity Level

$H_{\text{TCL}} := 115.4\text{ft}$

Bottom of Capacity Level

$H_{\text{BCL}} := 77.9\text{ft}$

Mean Height of Water Tank

$$H_{\text{mean}} := \frac{H_{\text{TCL}} + H_{\text{BCL}}}{2}$$

$H_{\text{mean}} = 96.65\text{ft}$

Shaft

Diameter of Shaft

$d_{\text{inner}} := 10\text{ft}$

Shaft Wall Thickness

$t_{\text{shaft}} := 1.2035\text{in}$

Inner Diameter of Shaft

$$d_{\text{shaft}} := d_{\text{inner}} + 2 \cdot t_{\text{shaft}}$$

$d_{\text{shaft}} = 10.2\text{ft}$

Length of Shaft

$L_{\text{shaft}} := 49.5\text{ft}$

Bell

Height of Bell

$h_{\text{bell}} := 22\text{ft}$

Diameter of Bell

$d_{\text{bell}} := 23.5\text{ft}$

Bolt Circle

Diameter of Bolt Circle

$d_{\text{circle}} := 24.6\text{ft}$

Number of Bolts

$N_{\text{bolt}} := 14$

Diameter of Bolts

$d_{\text{bolt}} := 1.5\text{in}$

Shaft Properties

Radius of Gyration	$r_{\text{shaft}} := \sqrt{\frac{d_{\text{shaft}}^2 + d_{\text{inner}}^2}{4}}$	$r_{\text{shaft}} = 85.71 \cdot \text{in}$
Slenderness Ratio	$KL_r := \frac{K \cdot L_{\text{shaft}}}{r_{\text{shaft}}}$	$KL_r = 13.86$
Elastic Section Modulus	$S_{\text{shaft}} := \frac{\pi \cdot (d_{\text{shaft}}^4 - d_{\text{inner}}^4)}{32 \cdot d_{\text{shaft}}}$	$S_{\text{shaft}} = 13750.5 \cdot \text{in}^3$
Gross Area	$A_{g_shaft} := \pi \cdot d_{\text{inner}} \cdot t_{\text{shaft}}$	$A_{g_shaft} = 453.71 \cdot \text{in}^2$
Radius to Thickness Ratio	$R_{\text{shaft}} := \frac{d_{\text{shaft}}}{2}$	$\frac{t_{\text{shaft}}}{R_{\text{shaft}}} = 0.0197$

Wind Load on Structures

Wind Pressure

Flat	$P_{\text{flat}} := 30 \cdot C_{d_flat} \cdot \left(\frac{V_{\text{eff}}}{100 \text{mph}} \right)^2 \cdot \text{psf}$	$P_{\text{flat}} = 30 \cdot \text{psf}$
Cylindrical	$P_{\text{cyl}} := 30 \cdot C_{d_cyl} \cdot \left(\frac{V_{\text{eff}}}{100 \text{mph}} \right)^2 \cdot \text{psf}$	$P_{\text{cyl}} = 18 \cdot \text{psf}$
Sphere	$P_{\text{sphere}} := 30 \cdot C_{d_sphere} \cdot \left(\frac{V_{\text{eff}}}{100 \text{mph}} \right)^2 \cdot \text{psf}$	$P_{\text{sphere}} = 15 \cdot \text{psf}$

Wind Projected Area

Tank	$A_{\text{tank}} := \frac{\pi \cdot d_{\text{tank}}^2}{4}$	$A_{\text{tank}} = 2419.22 \text{ft}^2$
Shaft	$A_{\text{shaft}} := L_{\text{shaft}} \cdot d_{\text{shaft}}$	$A_{\text{shaft}} = 504.93 \text{ft}^2$
Bell	$A_{\text{bell}} := \frac{d_{\text{shaft}} + d_{\text{bell}}}{2} \cdot h_{\text{bell}}$	$A_{\text{bell}} = 370.71 \text{ft}^2$

Wind Load

Tank	$F_{\text{tank}} := A_{\text{tank}} \cdot P_{\text{sphere}}$	$F_{\text{tank}} = 36.29 \cdot \text{kip}$
Shaft	$F_{\text{shaft}} := A_{\text{shaft}} \cdot P_{\text{cyl}}$	$F_{\text{shaft}} = 9.09 \cdot \text{kip}$
Bell	$F_{\text{bell}} := A_{\text{bell}} \cdot P_{\text{cyl}}$	$F_{\text{bell}} = 6.67 \cdot \text{kip}$

Antenna Load Summary

Carrier =	<ul style="list-style-type: none"> "Carrier 1" "Cricket" "Sprint 1" "Sprint 2" "Clearwire 1" "Clearwire 2" "Carrier 2" 	Elev_Ant =	<ul style="list-style-type: none"> 141.5 134 128 128 131 131 65 	ft	Num_Ant =	<ul style="list-style-type: none"> 9 6 6 3 1 1 12 	Num_Cable =	<ul style="list-style-type: none"> 9 6 6 3 1 1 12 					
Antenna =	<ul style="list-style-type: none"> "6 ft. Panel Antennas" "KMW HB-X-AW-19-65-00T" "RFS APL199016-42TO" "KMW HB-X-WM-17-65-00T" "2 ft. Dish" "2.5 ft. Dish" "6 ft. Panel Antennas" 	Height =	<ul style="list-style-type: none"> 72 72 72 44.8 24 30 72 	in	Width =	<ul style="list-style-type: none"> 8 6.2 5 6.14 24 30 8 	in	Depth =	<ul style="list-style-type: none"> 4 6.2 4 6.14 6 8 4 	in	Weight =	<ul style="list-style-type: none"> 32 32 12 25 15 32 32 	lb
Mount =	<ul style="list-style-type: none"> "Mini Monopole" "Pre-Fabricated Sectorized Mounts" "Pod Mount" "None" "Single Water Tank Mount" "Single Water Tank Mount" "Pre-Fabricated Sectorized Mounts" 	Num_Mnt =	<ul style="list-style-type: none"> 1 3 1 1 1 1 3 	A_Mnt =	<ul style="list-style-type: none"> 16 4 20 0 3 3 4 	ft ²	Wt_Mnt =	<ul style="list-style-type: none"> 1532 500 3000 0 80 80 500 	lb				

Force Coefficient on Appurtenance $Ca(As, F) :=$

$$\begin{aligned}
 & Ca \leftarrow 0 \\
 & \text{if } F = 1 \\
 & \quad \begin{cases} Ca \leftarrow 1.4 & \text{if } As < 7 \\ Ca \leftarrow 2.0 & \text{if } As > 25 \\ Ca \leftarrow 1.4 + \frac{(As - 7)}{18} \cdot 0.6 & \text{otherwise} \end{cases} \\
 & \text{otherwise} \\
 & \quad \begin{cases} Ca \leftarrow 0.8 & \text{if } As < 7 \\ Ca \leftarrow 1.2 & \text{if } As > 25 \\ Ca \leftarrow 0.8 + \frac{(As - 7)}{18} \cdot 0.4 & \text{otherwise} \end{cases} \\
 & Ca
 \end{aligned}$$

Wind Pressure on Appurtenance

$$P_Ant := \text{for } i \in 1..N$$

$$Kz_i \leftarrow \left(\frac{Elev_Ant_i}{33ft} \right)^{0.2857}$$

$$qz_i \leftarrow \left[0.00256 \cdot Kz \cdot \left(\frac{V_app}{mph} \right)^2 \right] \cdot psf$$

$$Gh_i \leftarrow 0.65 + \frac{0.6}{\left(\frac{Elev_Ant_i}{33ft} \right)^{0.1429}}$$

$$P \leftarrow qz_i \cdot Gh_i$$

26.77
26.36
26.02
P_Ant = 26.02 psf
26.19
26.19
21.44

New Appurtenance

Antenna 1

Carrier_C = "Carrier 1"

C_w := 1
n := 1 F_w := 1

Model / Quantity

Antenna_n = "6 ft Panel Antennas"

Elev_Ant_n = 141.5 ft

Dimension

Height_n = 72 in Width_n = 8 in Depth_n = 4 in Weight_n = 32 lb

Number of Antennas & Cables

Num_Ant_n = 9 Num_Cable_n = 9

Aspect Ratio & Force Coefficient

As := $\frac{Height_n}{Width_n}$ As = 9 CA := Ca(As, F) CA = 1.47

Area / Antenna:

A_ant_n := CA Height_n · Width_n A_ant_n = 5.87 ft²

Platform

Model & Quantity

Mount_C = "Mini Monopole"

Num_Mnt_C = 1

Wt_Mnt_C = 1532 lb A_Mnt_C = 16 ft²

New Appurtenance Load

Total Area of Appurtenance

$$A_appC := \sum_{i=1}^1 (Num_Ant_i \cdot A_ant_i + Num_Mnt_i \cdot A_Mnt_i) \quad A_appC = 68.8 ft^2$$

Total Weight of Appurtenance

$$W_appC := \sum_{i=1}^1 (Num_Ant_i \cdot Weight_i + Num_Mnt_i \cdot Wt_Mnt_i) \quad W_appC = 1820 lb$$

Existing Appurtenance

Antenna 2

Carrier_C = "Cricket"

C_W := 2

n_W := 2 F_W = 0

Model

Antenna_n = "KMW HB-X-AW-19-65-00T"

Elev_Ant_n = 134 ft

Dimension

Height_n = 72.in Width_n = 6.2.in Depth_n = 6.2.in

Weight_n = 32 lb

Number of Antennas & Cables

Num_Ant_n = 6

Num_Cable_n = 6

Aspect Ratio:

$$As := \frac{Height_n}{Width_n}$$

As = 11.61

Force Coefficient

$$CA := Ca(As, F)$$

CA = 0.9

Area / Antenna:

$$A_{ant_n} := CA \cdot Height_n \cdot Width_n$$

A_{ant_n} = 2.8 ft²

Platform

Model & Quantity

Mount_C = "Pre-Fabricated Sectorized Mounts"

Num_Mnt_C = 3

Wt_Mnt_C = 500 lb

A_Mnt_C = 4 ft²

Existing Appurtenance Load

Total Area of Appurtenance

$$A_{appC} := \sum_{i=2}^2 (Num_Ant_i \cdot A_{ant_i} + Num_Mnt_i \cdot A_{Mnt_i})$$

A_{appC} = 28.79 ft²

Total Weight of Appurtenance

$$W_{appC} := \sum_{i=2}^2 (Num_Ant_i \cdot Weight_i + Num_Mnt_i \cdot Wt_Mnt_i)$$

W_{appC} = 1692 lb

Existing Appurtenance

Antenna 3

Carrier_C = "Sprint 1"

C_W := 3

n_W := 3 F_W = 1

Model / Quantity

Antenna_n = "RFS APL 199016-42TO"

Elev_Ant_n = 128 ft

Dimension

Height_n = 72.in Width_n = 5.in Depth_n = 4.in

Weight_n = 12 lb

Number of Antennas & Cables

Num_Ant_n = 6

Num_Cable_n = 6

Aspect Ratio & Force Coefficient

$$As := \frac{Height_n}{Width_n}$$

As = 14.4

$$CA := Ca(As, F)$$

CA = 1.65

Area / Antenna:

$$A_{ant_n} := CA \cdot Height_n \cdot Width_n$$

A_{ant_n} = 4.12 ft²

Platform

Model & Quantity

Mount_C = "Pod Mount"

Num_Mnt_C = 1

Wt_Mnt_C = 3000 lb

A_Mnt_C = 20 ft²

Existing Appurtenance Load

Total Area of Appurtenance $A_{appC} := \sum_{i=3}^3 (\text{Num_Ant}_i \cdot A_{ant}_i + \text{Num_Mnt}_i \cdot A_{Mnt}_i)$ $A_{appC} = 44.7 \text{ft}^2$

Total Weight of Appurtenance $W_{appC} := \sum_{i=3}^3 (\text{Num_Ant}_i \cdot \text{Weight}_i + \text{Num_Mnt}_i \cdot \text{Wt_Mnt}_i)$ $W_{appC} = 3072 \text{lb}$

Existing Appurtenance

Antenna 4

Carrier_C = "Sprint 2"

C_{ww} := 4

n_{ww} := 4 F_{ww} := 0

Model / Quantity

Antenna_n = "KMW-HB-X-WM-17-65-00T"

Elev_Ant_n = 128 ft

Dimension

Height_n = 44.8-in Width_n = 6.14-in Depth_n = 6.14-in Weight_n = 25 lb

Number of Antennas & Cables

Num_Ant_n = 3

Num_Cable_n = 3

Aspect Ratio & Force Coefficient

$As := \frac{\text{Height}_n}{\text{Width}_n}$ As = 7.3

$CA := Ca(As, F)$ CA = 0.81

Area / Antenna:

$A_{ant}_n := CA \cdot \text{Height}_n \cdot \text{Width}_n$

$A_{ant}_n = 1.54 \text{ft}^2$

Platform

Model & Quantity

Mount_C = "None"

Num_Mnt_C = 1

Wt_Mnt_C = 0 lb

A_Mnt_C = 0 ft²

Existing Appurtenance Load

Total Area of Appurtenance $A_{appC} := \sum_{i=4}^4 (\text{Num_Ant}_i \cdot A_{ant}_i + \text{Num_Mnt}_i \cdot A_{Mnt}_i)$ $A_{appC} = 4.62 \text{ft}^2$

Total Weight of Appurtenance $W_{appC} := \sum_{i=4}^4 (\text{Num_Ant}_i \cdot \text{Weight}_i + \text{Num_Mnt}_i \cdot \text{Wt_Mnt}_i)$ $W_{appC} = 75 \text{lb}$

Existing Appurtenance

Antenna 5

Carrier_C = "Clearwire 1"

C_{ww} := 5

n_{ww} := 5 F_{ww} := 1

Model / Quantity

Antenna_n = "2-ft. Dish"

Elev_Ant_n = 131 ft

Dimension

Height_n = 24-in Width_n = 24-in Depth_n = 6-in Weight_n = 15 lb

Number of Antennas & Cables

Num_Ant_n = 1

Num_Cable_n = 1

Aspect Ratio & Force Coefficient

$As := \frac{\text{Height}_n}{\text{Width}_n}$ As = 1

$CA := Ca(As, F)$ CA = 1.4

Area / Antenna:

$$A_{ant_n} := CA \cdot Height_n \cdot Width_n$$

$$A_{ant_n} = 5.6 \text{ ft}^2$$

Platform

Model & Quantity

$$Mount_C = \text{"Single Water Tank Mount"}$$

$$Num_Mnt_C = 1$$

$$Wt_Mnt_C = 80 \text{ lb}$$

$$A_Mnt_C = 3 \text{ ft}^2$$

Existing Appurtenance Load

Total Area of Appurtenance

$$A_{app_C} := \sum_{i=5}^5 (Num_Ant_i \cdot A_{ant_i} + Num_Mnt_i \cdot A_{Mnt_i})$$

$$A_{app_C} = 8.6 \text{ ft}^2$$

Total Weight of Appurtenance

$$W_{app_C} := \sum_{i=5}^5 (Num_Ant_i \cdot Weight_i + Num_Mnt_i \cdot Wt_Mnt_i)$$

$$W_{app_C} = 95 \text{ lb}$$

Existing Appurtenance

Antenna 6

$$Carrier_C = \text{"Clearwire 2"}$$

$$C_w := 6$$

$$n_w := 6 \quad F_w := 1$$

Model / Quantity

$$Antenna_n = \text{"2.5 ft. Dish"}$$

$$Elev_Ant_n = 131 \text{ ft}$$

Dimension

$$Height_n = 30 \text{ in}$$

$$Width_n = 30 \text{ in}$$

$$Depth_n = 8 \text{ in}$$

$$Weight_n = 32 \text{ lb}$$

Number of Antennas & Cables

$$Num_Ant_n = 1$$

$$Num_Cable_n = 1$$

Aspect Ratio & Force Coefficient

$$As_w := \frac{Height_n}{Width_n}$$

$$As = 1$$

$$CA_w := Ca(As, F)$$

$$CA = 1.4$$

Area / Antenna:

$$A_{ant_n} := CA \cdot Height_n \cdot Width_n$$

$$A_{ant_n} = 8.75 \text{ ft}^2$$

Platform

Model & Quantity

$$Mount_C = \text{"Single Water Tank Mount"}$$

$$Num_Mnt_C = 1$$

$$Wt_Mnt_C = 80 \text{ lb}$$

$$A_Mnt_C = 3 \text{ ft}^2$$

Existing Appurtenance Load

Total Area of Appurtenance

$$A_{app_C} := \sum_{i=6}^6 (Num_Ant_i \cdot A_{ant_i} + Num_Mnt_i \cdot A_{Mnt_i})$$

$$A_{app_C} = 11.75 \text{ ft}^2$$

Total Weight of Appurtenance

$$W_{app_C} := \sum_{i=6}^6 (Num_Ant_i \cdot Weight_i + Num_Mnt_i \cdot Wt_Mnt_i)$$

$$W_{app_C} = 112 \text{ lb}$$

Existing Appurtenance

$C := 7$

Antenna 7

$Carrier_C = "Carrier 2"$

$n := 7$ $F := 1$

Model / Quantity

$Antenna_n = "6\text{-ft. Panel Antennas}"$

$Elev_Ant_n = 65\text{ft}$

Dimension

$Height_n = 72\text{-in}$ $Width_n = 8\text{-in}$ $Depth_n = 4\text{-in}$

$Weight_n = 32\text{lb}$

Number of Antennas & Cables

$Num_Ant_n = 12$

$Num_Cable_n = 12$

Aspect Ratio & Force Coefficient

$As := \frac{Height_n}{Width_n}$ $As = 9$

$CA := Ca(As, F)$ $CA = 1.47$

Area / Antenna:

$A_ant_n := CA \cdot Height_n \cdot Width_n$

$A_ant_n = 5.87\text{ft}^2$

Platform

Model & Quantity

$Mount_C = "Pre-Fabricated Sectorized Mounts"$

$Num_Mnt_C = 3$

$Wt_Mnt_C = 500\text{lb}$

$A_Mnt_C = 4\text{ft}^2$

Existing Appurtenance Load

Total Area of Appurtenance

$A_app_C := \sum_{i=7}^7 (Num_Ant_i \cdot A_ant_i + Num_Mnt_i \cdot A_Mnt_i)$ $A_app_C = 82.4\text{ft}^2$

Total Weight of Appurtenance

$W_app_C := \sum_{i=7}^7 (Num_Ant_i \cdot Weight_i + Num_Mnt_i \cdot Wt_Mnt_i)$ $W_app_C = 1884\text{lb}$

Summary of Loads

Appurtenance Wind Load & Weight

Carrier = $\left(\begin{array}{l} \text{"Carrier 1"} \\ \text{"Cricket"} \\ \text{"Sprint 1"} \\ \text{"Sprint 2"} \\ \text{"Clearwire 1"} \\ \text{"Clearwire 2"} \\ \text{"Carrier 2"} \end{array} \right)$

$$F_{Ant} := \left\{ \begin{array}{l} \text{for } i \in 1..C \\ F_i \leftarrow A_{app_i} \cdot P_{Ant_i} \\ F \end{array} \right.$$

1.84
0.76
1.16
0.12
0.23
0.31
1.77

F_Ant = 0.12 kip

1820
1692
3072
75
95
112
1884

W_app = 75 lb

Total Antenna Wind Load

$$F_{Ant_{total}} := \sum_{i=1}^C F_{Ant_i}$$

$$F_{Ant_{total}} = 6.18 \cdot \text{kip}$$

Total Weight of Appurtenance

$$W_{App_{total}} := \sum_{i=1}^C W_{app_i}$$

$$W_{App_{total}} = 8.75 \cdot \text{kip}$$

Structure

Wind Load

$$F_{tank} = 36.29 \cdot \text{kip}$$

$$F_{shaft} = 9.09 \cdot \text{kip}$$

$$F_{bell} = 6.67 \cdot \text{kip}$$

Weight

$$W_{water} := 62.4 \cdot \text{pcf} \cdot \text{Vol}$$

$$W_{structure} := 10\% \cdot W_{water}$$

$$W_{water} = 4170.83 \cdot \text{kip}$$

$$W_{structure} = 417.08 \cdot \text{kip}$$

Overturning Moment right above the Bell

$$M_{tank} := F_{tank} \cdot (H_{mean} - h_{bell})$$

$$M_{tank} = 2708.92 \cdot \text{kip} \cdot \text{ft}$$

$$M_{shaft} := F_{shaft} \cdot \frac{L_{shaft}}{2}$$

$$M_{shaft} = 224.95 \cdot \text{kip} \cdot \text{ft}$$

$$M_{Ant_{bell1}} := F_{Ant1} \cdot (Elev_{Ant1} - h_{bell})$$

$$M_{Ant_{bell1}} = 220.13 \cdot \text{kip} \cdot \text{ft}$$

$$M_{Ant_{bell2}} := F_{Ant2} \cdot (Elev_{Ant2} - h_{bell})$$

$$M_{Ant_{bell2}} = 84.99 \cdot \text{kip} \cdot \text{ft}$$

$$M_{Ant_{bell3}} := F_{Ant3} \cdot (Elev_{Ant3} - h_{bell})$$

$$M_{Ant_{bell3}} = 123.28 \cdot \text{kip} \cdot \text{ft}$$

$$M_{Ant_{bell4}} := F_{Ant4} \cdot (Elev_{Ant4} - h_{bell})$$

$$M_{Ant_{bell4}} = 12.75 \cdot \text{kip} \cdot \text{ft}$$

$$M_{Ant_{bell5}} := F_{Ant5} \cdot (Elev_{Ant5} - h_{bell})$$

$$M_{Ant_{bell5}} = 24.55 \cdot \text{kip} \cdot \text{ft}$$

$$M_{Ant_{bell6}} := F_{Ant6} \cdot (Elev_{Ant6} - h_{bell})$$

$$M_{Ant_{bell6}} = 33.54 \cdot \text{kip} \cdot \text{ft}$$

$$M_{Ant_{bell7}} := F_{Ant7} \cdot (Elev_{Ant7} - h_{bell})$$

$$M_{Ant_{bell7}} = 75.96 \cdot \text{kip} \cdot \text{ft}$$

Total Overturning Moment

$$M_{total_bell} := M_{tank} + M_{shaft} + \sum_{i=1}^C M_{Ant_bell_i}$$

$$M_{total_bell} = 3509.08 \cdot \text{kip} \cdot \text{ft}$$

Overtuning Moment at Anchor Bolts Level

$M_{\text{tank}} := F_{\text{tank}} \cdot H_{\text{mean}}$	$M_{\text{tank}} = 3507.27 \cdot \text{kip} \cdot \text{ft}$
$M_{\text{shaft}} := F_{\text{shaft}} \cdot \left(\frac{L_{\text{shaft}}}{2} + h_{\text{bell}} \right)$	$M_{\text{shaft}} = 424.9 \cdot \text{kip} \cdot \text{ft}$
$M_{\text{bell}} := F_{\text{bell}} \cdot \frac{h_{\text{bell}}}{2}$	$M_{\text{bell}} = 73.4 \cdot \text{kip} \cdot \text{ft}$
$M_{\text{Ant_anch1}} := F_{\text{Ant1}} \cdot \text{Elev_Ant1}$	$M_{\text{Ant_anch1}} = 260.66 \cdot \text{kip} \cdot \text{ft}$
$M_{\text{Ant_anch2}} := F_{\text{Ant2}} \cdot \text{Elev_Ant2}$	$M_{\text{Ant_anch2}} = 101.69 \cdot \text{kip} \cdot \text{ft}$
$M_{\text{Ant_anch3}} := F_{\text{Ant3}} \cdot \text{Elev_Ant3}$	$M_{\text{Ant_anch3}} = 148.87 \cdot \text{kip} \cdot \text{ft}$
$M_{\text{Ant_anch4}} := F_{\text{Ant4}} \cdot \text{Elev_Ant4}$	$M_{\text{Ant_anch4}} = 15.39 \cdot \text{kip} \cdot \text{ft}$
$M_{\text{Ant_anch5}} := F_{\text{Ant5}} \cdot \text{Elev_Ant5}$	$M_{\text{Ant_anch5}} = 29.51 \cdot \text{kip} \cdot \text{ft}$
$M_{\text{Ant_anch6}} := F_{\text{Ant6}} \cdot \text{Elev_Ant6}$	$M_{\text{Ant_anch6}} = 40.31 \cdot \text{kip} \cdot \text{ft}$
$M_{\text{Ant_anch7}} := F_{\text{Ant7}} \cdot \text{Elev_Ant7}$	$M_{\text{Ant_anch7}} = 114.83 \cdot \text{kip} \cdot \text{ft}$

Total Overtuning Moment $M_{\text{total_anchor}} := M_{\text{tank}} + M_{\text{shaft}} + M_{\text{bell}} + \sum_{i=1}^C M_{\text{Ant_anch}i}$ $M_{\text{total_anchor}} = 4716.82 \cdot \text{kip} \cdot \text{ft}$

Allowable Stresses on Shaft

Allowable Local Buckling Stress
per AWWA-D100-96 Class II Material

$F_L := 18 \text{ksi}$

Slenderness Reduction Factor

$$K_{\psi} := \begin{cases} C_c \leftarrow \sqrt{\frac{\pi^2 \cdot E}{F_L}} \\ K \leftarrow 1 - \frac{1}{2} \cdot \left(\frac{KL_r}{C_c} \right)^2 & \text{if } 25 < KL_r \leq C_c \\ K \leftarrow \frac{1}{2} \cdot \left(\frac{KL_r}{C_c} \right)^2 & \text{if } KL_r > C_c \\ K \leftarrow 1 & \text{if } KL_r \leq 25 \\ K \end{cases}$$

$K_{\psi} = 1$

Allowable Axial Stress $F_{a_shaft} := F_L \cdot K_{\psi}$ $F_{a_shaft} = 18 \cdot \text{ksi}$

Allowable Shear Stress $F_{v_shaft} := F_L$ $F_{v_shaft} = 18 \cdot \text{ksi}$

Allowable Bending Stress $F_{b_shaft} := F_L$ $F_{b_shaft} = 18 \cdot \text{ksi}$

Stress Analysis

Shear Stress

$$f_{v_shaft} := \frac{F_{tank} + F_{shaft} + F_{Ant_{total}}}{A_{g_shaft}}$$

$$f_{v_shaft} = 0.11 \cdot \text{ksi}$$

$$f_{v_shaft} < F_{v_shaft} = 1$$

CHECK = "Shaft is adequate for shear loads"

Axial Stress

$$f_{a_shaft} := \frac{W_{water} + W_{structure} + W_{App_{total}}}{A_{g_shaft}}$$

$$f_{a_shaft} = 10.13 \cdot \text{ksi}$$

$$f_{a_shaft} < F_{a_shaft} = 1$$

CHECK = "Shaft is adequate for axial loads"

Bending Stress

$$M_{total} := M_{total_bell}$$

$$M_{total} = 3509.08 \cdot \text{kip} \cdot \text{ft}$$

$$f_{b_shaft} := \frac{M_{total}}{S_{shaft}}$$

$$f_{b_shaft} = 3.06 \cdot \text{ksi}$$

$$f_{b_shaft} < F_{b_shaft} = 1$$

CHECK = "Shaft is adequate for bending"

Combined Stresses (Increased by 1.33 for wind)

$$\text{Stress} := \frac{f_{a_shaft}}{1.33F_{a_shaft}} + \frac{f_{b_shaft}}{1.33F_{b_shaft}}$$

$$\text{Stress} = 55.11\%$$

Load Increases Due to Antennas

Increase in Lateral Load

$$\%V := \frac{F_{Ant_{total}}}{F_{tank} + F_{shaft} + F_{bell}}$$

$$\%V = 11.88\%$$

Increase in Axial Load

$$\%P := \frac{W_{App_{total}}}{W_{water} + W_{structure}}$$

$$\%P = 0.19\%$$

Increase in Moment Load

$$\%M := \frac{\sum_{i=1}^C M_{Ant_anch_i}}{M_{tank} + M_{shaft}}$$

$$\%M = 18.09\%$$

Check Anchor Bolts

Diameter of Bolts & Number of Bolts

$$d_{\text{bolt}} = 1.5 \cdot \text{in}$$

$$N_{\text{bolt}} = 14$$

Cross Section Area

$$A_{\text{gbolt}} := \frac{\pi \cdot d_{\text{bolt}}^2}{4}$$

$$A_{\text{gbolt}} = 1.77 \cdot \text{in}^2$$

Diameter of Bolt Circle

$$d_{\text{circle}} = 24.6 \text{ ft}$$

Allowable Strength on Anchor Bolt (AWWA-D100-05)

$$F_{t,\text{bolt}} := 31.25 \text{ ksi}$$

Moment of Intertia

$$I_{\text{bolt}} := \begin{cases} I_{\text{total}} \leftarrow 0 \text{ in}^4 \\ \text{for } i \in 1..N_{\text{bolt}} \\ \quad d_i \leftarrow \frac{d_{\text{circle}}}{2} \cdot \sin \left[\frac{360 \text{ deg}}{N_{\text{bolt}}} \cdot (i - 1) \right] \\ \quad I_i \leftarrow A_{\text{gbolt}} \cdot (d_i)^2 \\ \quad I_{\text{total}} \leftarrow I_{\text{total}} + I_i \\ I_{\text{total}} \end{cases}$$

$$I_{\text{bolt}} = 269490.31 \cdot \text{in}^4$$

Section Modulus of Bolt Configuration

$$S_{\text{bolt}} := \frac{I_{\text{bolt}}}{d_{\text{circle}}}$$

$$S_{\text{bolt}} = 912.91 \cdot \text{in}^3$$

Overturning Moment at Anchor Bolt Level

$$M_{\text{total}} := M_{\text{total_anchor}}$$

$$M_{\text{total}} = 4716.82 \cdot \text{kip} \cdot \text{ft}$$

Stresses on Anchor Bolts due to Bending

$$f_b := \frac{M_{\text{total}}}{S_{\text{bolt}}}$$

$$f_b = 62 \cdot \text{ksi}$$

Stresses on Anchor Bolts due to Axial Loads

$$f_t := \frac{W_{\text{structure}} + W_{\text{Apptotal}}}{N_{\text{bolt}} \cdot A_{\text{gbolt}}}$$

$$f_t = 17.21 \cdot \text{ksi}$$

Uplift are possible to occur

$$T_w := f_b - f_t$$

$$T = 44.79 \cdot \text{ksi}$$

Allowable Tension Stress

$$F_t := 1.33 \cdot F_{t,\text{bolt}}$$

$$F_t = 41.56 \cdot \text{ksi}$$

CHECK = "Make Changes!"

$$\frac{T}{F_t} = 107.76 \%$$

Conclusions

(2) New dish antennas will be installed on the existing antenna mount, by extending the support pipe of existing installation.

Further structural analysis shows that existing water tower structure is still **adequate** for new loads. Stresses above the bell and possibility of uplift are checked. Anchor bolts are slightly overstressed, but still **acceptable** by engineering judgment.

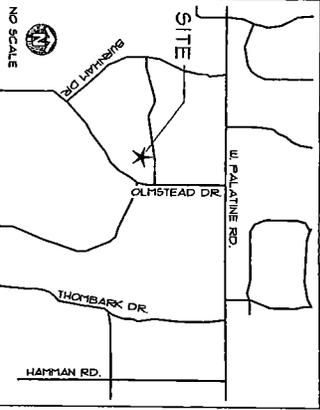
SHEET INDEX

NO.	DESCRIPTION
T-1	TITLE SHEET
C-1	SITE PLAN
C-2	BLANKED SITE PLAN
C-3	SHELTER/ANTENNA PLAN
C-4	SITE ELEVATION & DETAILS
C-5	DETAILS
E-1	SINGLE LINE & NOTES

DRIVING DIRECTIONS

DEPART FROM CLEARVIEW OFFICE,
54602 N. RIVER RD., ROSELLE, IL 60208
DEPART ON N RIVER RD (NORTH), ROAD NAME CHANGES TO
DES PLAINES RIVER RD, N RIVER RD, TURN LEFT (WEST) ONTO
ONTO WILSONS RD, TURN RIGHT ONTO RAMP 1-30, TRAVEL
WEST ON RAMP 1-30, TURN LEFT ONTO ROSELLE RD, KEEP RIGHT
ROSELLE RD, KEEP RIGHT TO STAY ON RAMP PALMATIVE BEAR
RIGHT (NORTH) ONTO (N) ROSELLE RD, TURN LEFT (WEST) BEAR
RIGHT ON PALMATIVE RD, TURN LEFT (SOUTH) ONTO OLNSTEAD DR
ARRIVE AT SITE.

VICINITY MAP



clear w'reless LLC

a Nevada limited liability company, a Sprint affiliate

SITE NAME

HOFFMAN ESTATES 5

SITE NUMBERS

IL-CHI5444

SITE ADDRESS

4690 OLNSTEAD DRIVE
HOFFMAN ESTATES, IL 60195

PROJECT TYPE

BACKHAUL PROJECT

PROJECT TEAM

clear w'reless LLC
a Nevada limited liability company,
a Sprint affiliate
54602 N. RIVER RD.
SUITE 300
ROSELLE, IL 60208
(847) 385-3000
APPLICANT

EVEREST GROUP
THE EVEREST GROUP, LLC
1015 W. 11TH ST.
NORTH, IL 60462
TEL: (708) 573-1883
REAL ESTATE

ENGINEERS/STRUCTURAL
Fullerton Engineering Consultants
5100 W. HIGGINS RD.
SUITE 800
ROSEMONT, ILLINOIS 60018
TEL: 641-231-2009
FAX: 641-231-2009

- HANDICAP ACCESS REQUIREMENTS ARE NOT REQUIRED
- FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION
- FACILITY HAS NO PLUMBING OR REFRIGERANTS
- THIS FACILITY SHALL MEET OR EXCEED ALL FAA AND FCC REGULATORY REQUIREMENTS
- ALL WORK MUST CONFORM TO CLEARVIEW CLEARVIEW CONSTRUCTION INSTALLATION GUIDE - "UTILITY"

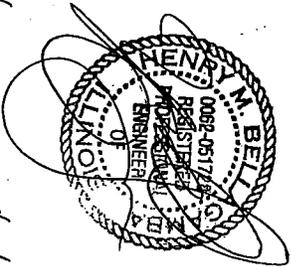
- SCOPE OF WORK:**
- NEW CLEARVIEW BACKHAUL DISH ANTENNAS INSTALLED ON EXISTING WATER TANK
 - NEW CLEARVIEW BACKHAUL EQUIPMENT INSTALLED INSIDE EXISTING SHELTER

PROJECT SUMMARY

SITE NAME:	HOFFMAN ESTATES 5
SITE NO.:	IL-CHI5444
SERIAL LOG# NO.:	IL1711
SITE ADDRESS:	4690 OLNSTEAD DRIVE HOFFMAN ESTATES, IL 60195
COUNTY:	COOK
CITY:	CITY OF HOFFMAN ESTATES
LANDLORD ADDRESS:	
SITE COORDINATES (FROM CLEARVIEW):	
LATITUDE:	N 42.0281°
LONGITUDE:	W 88.1156°
ZONE/ID:	(NAD 83) (NAD 83)
JURISDICTION:	
BUILDING CODE:	INTERNATIONAL BUILDING CODE 2003 EDITION
ELECTRICAL CODE:	NATIONAL ELECTRICAL CODE 2009 EDITION

ENGINEER'S LICENSE

I CERTIFY THAT THESE DRAWINGS WERE PREPARED BY THE OR UNDER MY DIRECT SUPERVISION AND CONTROL, AND TO THE BEST OF MY KNOWLEDGE AND BELIEF COMPLY WITH THE REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE, 2003 EDITION
LICENSED ENGINEER - STATE OF ILLINOIS



11/30/09
5/24/09

APPROVALS

CLEARVIEW CONST.	DATE
CLEARVIEW PER	DATE
CLEARVIEW OPS	DATE
LANDLORD	DATE

DRAWING SCALED TO 11"x17"

clear w'reless LLC
a Nevada limited liability company,
a Sprint affiliate
54602 N. RIVER RD.
SUITE 300
ROSELLE, IL 60208
(847) 385-3000

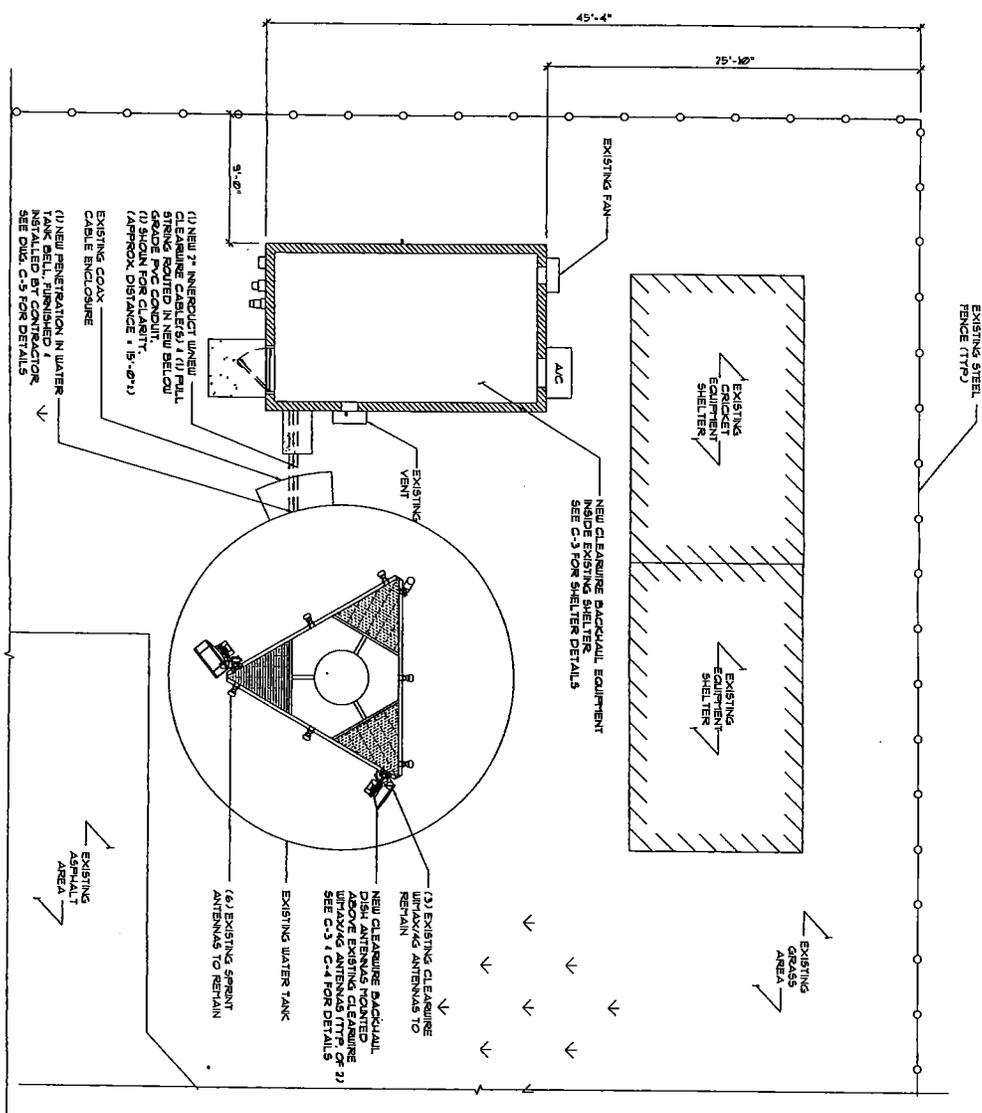


Fullerton Engineering Consultants
5100 W. HIGGINS RD.
ROSEMONT, ILLINOIS 60018
TEL: 641-231-2009
FAX: 641-231-2009
DESIGN PERM NO. 184-020439

DATE	DESCRIPTION	INITIALS
03/14/09	SEALED	SA
03/13/09	FINAL	SE

SHEET NAME	TITLE SHEET
SHEET NUMBER	T-1

ENLARGED SITE PLAN



SCALE: 1/8" = 1'-0"



clear
wireless LLC.
a Nevada limited liability company,
a Sprint affiliate
1600 N. RIVER RD.
SUITE 300
ROSEMONT, IL 60016
(641) 38-5600

Fullerton Engineering Consultant
5100 W. HIGGINS RD.
ROSEMONT, IL 60016
TEL: 641-331-0700
FAX: 641-331-0705
DESIGN FIRM NO. 184-027459

REVISION	DATE	DESCRIPTION
01/04/09	SM REVIEW	SM
07/27/09	FINAL	SM

CHECKED BY: JP
APPROVED BY: TMB
DATE: 01/04/09
DESCRIPTION: SM REVIEW
BY: SM
DATE: 07/27/09
FINAL
BY: SM

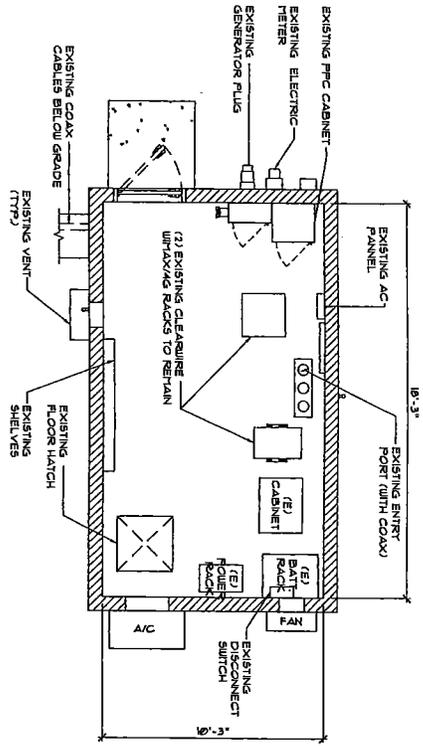
SITE NAME
HOFFMAN
ESTATES 5

SITE NO.
IL-CH15444

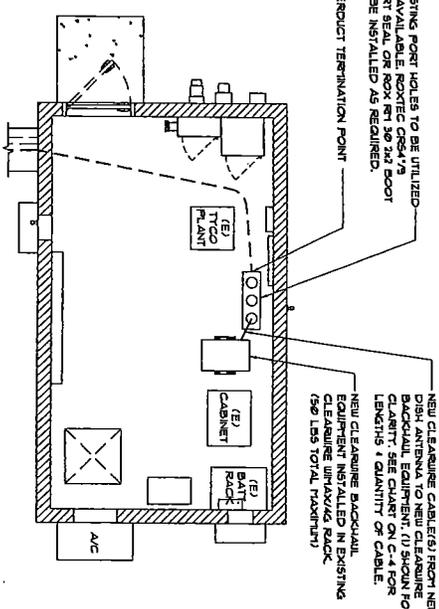
SITE ADDRESS
4480 CLAYSTEAD DRIVE
HOFFMAN ESTATES, IL 60135

SHEET NAME
ENLARGED
SITE PLAN

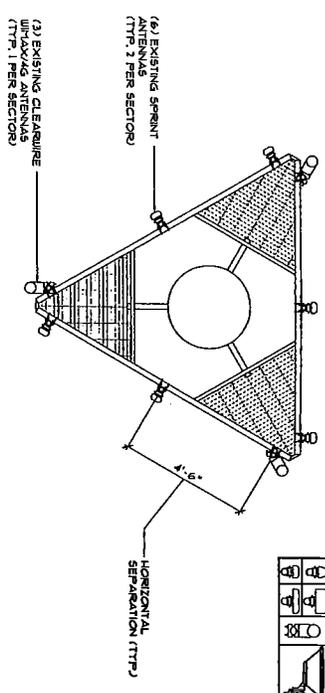
SHEET NUMBER
C-2



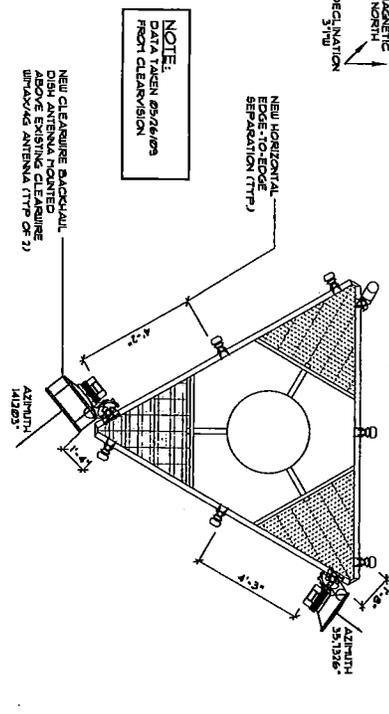
EXISTING SHELTER PLAN
SCALE: 3/16" = 1'-0"



NEW SHELTER PLAN
SCALE: 3/16" = 1'-0"



SECTOR 1 2 3
AZIMUTH 60° 180° 300°
PER SPRINT SCHEDULE - SEE NOTE
PER SPRINT SCHEDULE - SEE NOTE
NOTE:
CONTRACTOR IS REQUIRED TO DISMANTLE EXISTING CLEARANCE ANTENNA FOR NEW MOUNTING PIPE INSTALLATION. CONTRACTOR SHALL FIELD VERIFY CLEARANCE ANTENNA AZIMUTH & REPLACE TO EXISTING AZIMUTH.
EXISTING ANTENNA CONFIGURATION/BACKHAUL SCHEDULE
SCALE: 3/16" = 1'-0"



NOTE:
DATA TAKEN 09/04/09
FROM CLEARVISION
NEW HORIZONTAL SEPARATION (TTP)
NEW CLEARANCE BACKHAUL
DISH ANTENNA MOUNTED ABOVE EXISTING CLEARANCE UTILITY RACK (TTP OR 2)
ANTENNA AZIMUTH 142.83°
ANTENNA MODEL ANTENNA MODEL SIZE ANTENNA POINT DISTANCE RECEIVING RADIO MODEL
AZIMUTH 351336 351336 351336 24" 30" 181 181 181 2.485 IL-CH15355 AIRCELL LP
VAZ:PT:UB VHL:PT:UB 30" 30" 181 181 181 2.485 IL-CH15355 AIRCELL LP
142.83° 142.83° 142.83°
NEW ANTENNA CONFIGURATION/BACKHAUL SCHEDULE
SCALE: 3/16" = 1'-0"

clear
Wireless LLC.
a Veridia limited liability company,
a Sprint affiliate
5662 N. RIVER RD.
SUITE 300
ROSELAND, IL 60468
(815) 316-5800

Fullerton Engineering Consultants
5700 W. MICHIGAN RD.
ROSEMONT, ILLINOIS 60018
TEL: 847-253-0700
FAX: 847-253-0700
DESIGN: F817101, 04-002448

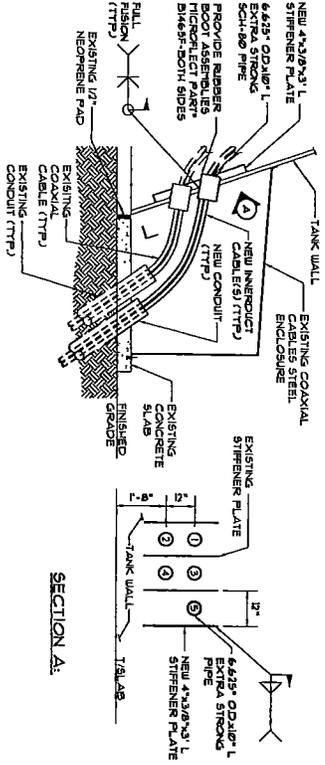
DATE	DESCRIPTION	BY	CHK
07/27/09	REV	JK	JK
07/27/09	FINAL	JK	JK

DATE NO. 1003-0000
PROJECT NO. 03-0000
ENGINEER OF ILLINOIS
M. BELLA
REGISTERED PROFESSIONAL ENGINEER
NO. 031-000000

SITE NAME
HOFFMAN
ESTATES B
SITE NO.
IL-CH15444
SITE ADDRESS
4690 CLYSTEAD DRIVE
HOFFMAN ESTATES, IL 60143

SHEET NAME
SHELTER/
ANTENNA
PLAN
SHEET NUMBER
C-3

- NOTES:
- 1) REDUCE TANK CONTENTS BY 50% OR MORE BEFORE
 - 2) CUTTING SHELL FOR PENETRATION.
 - 3) CUT SLEEVE HOLE ONE 4 WELD PIPE SLEEVE IN PLACE
 - 4) CUT SLEEVE HOLE 2 4 WELD PIPE SLEEVE IN PLACE
 - 5) REPEAT SEQUENCE FOR ALL HOLES



BELL/PEDESTAL PLATE PENETRATION

SCALE: N.T.S. |

clear wireless LLC
a Nevada limited liability company,
a Sprint affiliate

5600 N. RIVER RD.
ROSEMONT, IL 60018
(631) 310-3000

Fullerton Engineering Consultants
5100 W. HICKMAN RD.
ROSEMONT, IL 60018
TEL: 631-733-0200
FAX: 631-733-0200
DESIGN FILE NO. 04-0202439

CHECKED BY:	JP	APPROVED BY:	TJB	DATE:	DISCREPANCY:	INT.
	07/24/05		07/24/05		NEW REVIEW:	NAI
	07/29/05		07/29/05		FINAL:	SG

0082-081723
REGISTERED PROFESSIONAL ENGINEER
STATE OF ILLINOIS

SITE NAME HOEFFMAN ESTATES B	SITE NO. IL-CH15444	SITE ADDRESS 4840 CLINTON DRIVE HOEFFMAN ESTATES, IL 60135
SHEET NAME DETAILS		
SHEET NUMBER C-5		

VILLAGE OF HOFFMAN ESTATES
ZONING BOARD OF APPEALS

FINDING OF FACT

DATE OF PUBLIC HEARING: November 17 and December 8, 2009

DATE OF PRESENTATION TO VILLAGE BOARD: December 14, 2009

PETITION: Hearing held at the request of the Village of Hoffman Estates (Lessor) and The Everest Group/Sprint, Clearwire (Lessee) to consider a special use under the Zoning Code to permit the installation of communication antennas and accompanying equipment on the property located at 95 Aster Lane.

DISTRICT IN WHICH PROPERTY IS LOCATED: B-2, Community Business District

ZONING CODE SECTION(S) FOR SPECIAL USE: 9-3-9-A and 9-8-2-C-6

FINDING-OF-FACT: The Zoning Board of Appeals (ZBA) found that the Standards for a Special Use (Section 9-1-18-I) were met.

MOTION: Request to grant the Village of Hoffman Estates (Lessor) and The Everest Group/Sprint, Clearwire (Lessee), *a special use under Sections 9-3-9-A and 9-8-2-C-6 to permit the installation of two (2) microwave dishes and associated equipment to be no greater than one hundred and six (106) feet high on a Village water tank located at 95 Aster Lane.* The following conditions shall apply:

1. This special use shall be subject to approval of the final lease agreement with the Village of Hoffman Estates.
2. No advertising shall be allowed on the equipment or structures.
3. The petitioner shall pay all costs associated with the third party review and inspections, as required by the Village's Public Works Department policy.
4. Should the operation of these microwave dishes cease for a period of six (6) months, the dishes and associated equipment shall be removed per Zoning Code Section 9-1-18-L.

The petitioner was agreeable to the above listed conditions.

RECOMMENDATION: The Zoning Board of Appeals (ZBA) recommends approval of this request.

The petitioner, Mr. Faber representing Sprint-Clearwire, requested a special use to install two microwave dish antennas on the water tank. The dishes operate under a different radio band from the existing cellular antennas and are used to carry the increased volumes of data (video and internet) common in modern cell phones. The Zoning Board confirmed that the structural report was completed and was accepted by the Village. The Zoning Board confirmed that each

dish weighs approximately 45 pounds including the mounting equipment. Zoning Board members noted that there are over 40 existing antennas on the water tank, including public and private installations, and questioned whether the water tank was becoming overly cluttered with communications equipment. The concern was not related to the structural integrity of the installation, but was related to aesthetics. However, the water tank is a tall, existing structure that has been used for this purpose and is less aesthetically offensive than a new monopole cell tower. The Zoning Board recommended approval of the request.

AUDIENCE COMMENTS

None.

VOTE:

5 Ayes

2 Nays (Ciffone, Gaeta)

0 Absent

ZONING BOARD OF APPEALS

Chairman William Weaver

Vice-Chairman Ronald Jehlik

Denise Wilson

Michael Ciffone

Masoom Ali

Donna Boomgarden

Michael Gaeta

**THIS SPECIAL USE WILL EXPIRE UNLESS ACTED UPON WITHIN
ONE (1) YEAR OF VILLAGE BOARD APPROVAL**

***AN IMMEDIATE AUTHORIZATION TO APPLY FOR PERMITS*
IS REQUESTED**

FINDING OF FACT WRITTEN BY DEVELOPMENT SERVICES STAFF

VILLAGE OF HOFFMAN ESTATES

Memo

TO: William Weaver, Zoning Board of Appeals Chairman
FROM: Josh Edwards, Assistant Planner *JAE*
RE: **SPRINT-CLEARWIRE COMMUNICATIONS - 95 ASTER LANE -
SPECIAL USE - COMMUNICATION ANTENNAS AND
ASSOCIATED EQUIPMENT**

DATE: December 3, 2009

HEARING DATE: December 8, 2009

1. REQUEST SUMMARY

Request by Village of Hoffman Estates (owner) and Sprint-Clearwire Communications (lessee) for a special use to permit the installation of two microwave dishes and associated equipment on a Village owned water tank at 95 Aster Lane.

2. BACKGROUND

The existing water tank at 95 Aster Lane currently contains communication antennas installed by Sprint, Nextel (Sprint), T-Mobile, Verizon, JAWA, the Village of Schaumburg, and the Village of Hoffman Estates. The equipment for the existing antennas is located near the base of the tank in shelters and equipment cabinets. The existing Sprint ground equipment is located within a masonry shelter in equipment cabinets near the base of the water tank.

3. PETITIONER PROPOSAL

The petitioner is proposing to install two, 26 inch high microwave dishes at 106 feet above grade on the support structure of the water tank. The installation requires a special use approval. The two dishes would be attached near the existing Sprint (originally approved as Nextel) cellular antennas centered at 100 feet high.

The function of the new dishes is to support and upgrade the Sprint network with voice and data transmission as an alternative to installing miles of underground fiber optic wires throughout the coverage area.

No separate building or equipment is proposed on the ground. The new equipment for the dishes would be installed entirely within the existing equipment shelter located northeast of the water tank. Wiring would be installed underground between the shelter and water tank; and cables would be run on existing cable supports on the water tank to the dishes.

4. SITE CONDITIONS

The subject property is zoned B-2, Community Business District. The surrounding properties to the north, south, and west are zoned B-2, Community Business District. Adjacent to the south is the Hoffman Plaza shopping center. To the east is the Parcel A neighborhood zoned R-2, One Family Residential District. The site is accessed from Aster Lane, which connects with the rear service aisle behind Hoffman Plaza.

5. APPLICABLE REQUIREMENTS

- a) Section 9-8-2-C-6 (p. CD9:124) states that public utility and public service uses shall be permitted in the B-2 District subject to the issuance of a special use permit in accordance with the provisions of Section 9-1-18.
- b) Section 9-3-9-A (p. CD9:58) states that the total antenna height of any communications tower, antenna or combination thereof of any height over 45 feet upon Village owned or leased facilities shall be permitted only as a special use.

6. ADJACENT OWNER COMMENTS

Standard notification letters have been mailed and as of this writing no comments have been received.

7. RELEVANT SPECIAL USE HISTORY

Subject Property

- a) Ordinance No. 3939-2007 – The Village Board approved a special use for the Village of Schaumburg to install two (2) antennas at 158 feet above grade.
- b) Ordinance No. 3894-2006 – The Village Board approved a special use for T-Mobile to install nine (9) antennas at 158 feet above grade.
- c) Ordinance No. 3670-2004 – The Village Board approved a special use for Verizon Wireless (formerly Ameritech) to replace eight (8) existing antennas (originally approved by Ordinance 1825-1987) with twelve (12) antennas at 116 feet above

grade. A proposal has recently been approved to lower the Verizon antennas on the water tank, which did not require an amendment to the special use.

- d) Ordinance No. 3119-1999 – The Village Board approved a special use for Nextel to install nine (9) antennas at 100 feet above grade.
- e) Ordinance No. 2974-1998 – The Village Board approved a special use for Sprint PCS to install nine (9) antennas at 88 feet above grade.
- f) Ordinance No. 1543-1984 – The Village Board approved a special use for JAWA to install two antennas on the top of the tank with two repeaters and an uninterruptible power supply.
- g) Ordinance No. 1361-1982 – The Village Board approved a special use for the Village of Hoffman Estates to install antennas for police personnel communications systems.

Similar Properties

Over the past decade various communication antennas have been installed on all 6 water tanks throughout the Village.

8. PUBLIC WORKS COMMENTS

A thorough structural analysis and engineering review is required prior to the zoning review for Village owned water tanks. A structural analysis prepared by a licensed engineer on behalf of the petitioner has been reviewed by Public Works and the Village's third party engineering consultants. The structural analysis concluded that the installation will have no adverse impact on the water tank. Public Works has accepted the report.

9. IMMEDIATE AUTHORIZATION TO APPLY FOR PERMIT(S)

The petitioner has requested an Immediate Authorization to Apply for Permit(s).

10. DEVELOPMENT SERVICES COMMENTS

The proposal to install dishes onto a water tank rather than erecting a separate antenna tower will minimize visual clutter. The proposed dishes are designed to upgrade the existing antennas. The height above grade and the relatively small size of the two dishes will reduce the visibility of the dishes from surrounding properties. The dish is smaller than standard cellular antenna arrays and is comparable in size to a residential television satellite dish and no more aesthetically obtrusive.

The equipment on the ground supporting the new dishes will be located entirely within an existing brick equipment shelter.

The existing special use approvals and leases for the existing Sprint and Nextel antennas will stay in effect. A new lease will be required for this new installation.

The Federal Telecommunications Act of 1996 expressly preempts local governments from regulating the placement, construction, or modification of personal wireless services on the basis of environmental or health concerns. Such concerns cannot be discussed or used as a basis of the decision whether to grant a special use.

11. MOTION

Should the Zoning Board find that the Standards for a Special Use are met, the following motion is provided with conditions:

A special use under Sections 9-3-9-A and 9-8-2-C-6 to permit the installation of two (2) microwave dishes and associated equipment to be no greater than one hundred and six (106) feet high on a Village water tank at 95 Aster Lane.

The following conditions shall apply:

- 1. This special use shall be subject to approval of the final lease agreement with the Village of Hoffman Estates.*
- 2. No advertising shall be allowed on the equipment or structures.*
- 3. The petitioner shall pay all costs associated with the third party review and inspections, as required by the Village's Public Works Department policy.*
- 4. Should the operation of these microwave dishes cease for a period of six (6) months, the dishes and associated equipment shall be removed per Zoning Code Section 9-1-18-L.*

cc: Corporation Counsel, D. O'Malley, D. Plass, R. Norton, Petitioner

VILLAGE OF HOFFMAN ESTATES
ZONING BOARD OF APPEALS

REQUEST FOR SPECIAL USE HEARING

FOR VILLAGE USE ONLY

Hearing Fee \$ 400.00 Date Paid 7/8/09 Received By P. Moore

Hearing Date: 12/8/09 Time: 7:30 pm Legal Published

Receipt Number 293 400 Check No. 104880 Zoning District B.2

PLEASE PRINT OR TYPE

Village of Hoffman Estates

1. Name of Property Owner(s)* _____
E-Mail Address joshua.edwards@hoffmanestates.org Fax 847-490-6868
1900 Hassell Road
Owner's Address _____ Phone 847-862-9100
Hoffman Estates City _____ State IL Zip 60169

Subject Property's Address (if different than #1): 95 Aster Lane

2. Person applying if other than owner:*
Name Will Faber Company The Everest Group/ Sprint, Clearwire
E-Mail Address wfaber85@gmail.com Fax 773/409-5426
Address 7013 W. 111th St. Phone 773-987-5299
City Worth State IL Zip 60482

3. Property Index Number (PIN) 07-15-200-026-0060

5. Please describe the proposed use, or attach a letter.

Addition of 2 dish antennas to existing telecommunications facility

* If "owner" is an entity other than an individual(s), then an Economic Disclosure Statement must also be filed.

6. **An Immediate Authorization to Apply for Permits** allows the Code Enforcement Division to begin the building permit review process prior to the adoption of the Ordinance by the Village Board. The Immediate Authorization makes it possible for you to expedite the plan review process by approximately two weeks. Contact the Code Enforcement Division at 847/781-2631 to discuss the building permit application and review process.

Please check one of the following

Yes, I request Immediate Authorization to Apply for Permits upon approval of my application by the Village Board, allowing me to begin the building permit review process prior to adoption of the Ordinance approving my special use.

Or

No, I do not request Immediate Authorization to Apply for Permits.

7. I, **the undersigned**, certify the information and submissions provided accurately represent the current conditions and proposed improvement(s) requiring a special use.

Owner's Signature _____

Name (Please Print) _____

Applicant's Signature *William Pbe* _____

Name (Please Print) William Pbe _____

All requests for a hearing must be accompanied by the items required according to the nature of the request. All fees must be paid before Zoning Board can hear any case. Any additional fees must be paid before any findings or reports are given to the Village Board.

EXHIBIT A

DESCRIPTION OF LAND

to the Agreement dated October, 11 1999, by and between Village of Hoffman Estates, and Illinois municipal corporation, as Lessor, and Nextel West Corp., a Delaware corporation, d/b/a Nextel Communications, as Lessee.

The Land is described and/or depicted as follows:

LEGAL DESCRIPTION:

As recorded per document no. 934546, in volume 1874, page 274, in Cook County.

That part of LOT FOURTEEN (14) in Block One (1) in Hoffman Estates 1, described as follows: Commencing at the Northeast corner of said LOT Fourteen (14); thence South along the East line of said Lot Fourteen (14) a distance of 200 feet and thence West along a line perpendicular to said East Line of Lot Fourteen (14), a distance of 217 feet, for a place beginning; thence South from said place of beginning along a line perpendicular to the last described course a distance 150 feet; thence East along a line perpendicular to the last described course, a distance of 150 feet; thence North along a line perpendicular to the last described course, a distance of 150 feet; thence West along a line perpendicular to the last described course, a distance of 150 feet more or less to the place of beginning.

and commonly known as: 95 Aster, Hoffman Estates, IL 60195
P.I.N. #: 07-15-200-026



THE EVEREST GROUP
Representing Sprint/ Clearwire

7013 W 111th St
Worth, IL 60482

773-987-5299

wfaber85@gmail.com

July 8, 2009
Village of Hoffman Estates
1900 Hassell Road
Hoffman Estates, IL 60169

This application for a special use permit on behalf of Sprint/ Clearwire is for the purpose of installing two additional antennas and two lines of associated coaxial cable on the water tank at 95 Aster Lane. The purpose of the communications system upgrade is to facilitate wireless backhaul coverage over the existing Sprint telecommunications network. Effectively, the current T1 system will be replaced with wireless connections between individual communication sites and the central switch. The new antenna will make this linkage possible, thereby lowering costs to consumers and increasing the reliability and robustness of the existing and future data network.

Sincerely yours,

Will Faber

The Everest Group



HOFFMAN ESTATES

GROWING TO GREATNESS

November 5, 2009

To All Interested Parties:

Please be advised the Zoning Board of Appeals of the Village of Hoffman Estates will conduct a public hearing at the request of the Village of Hoffman Estates (Lessor) and The Everest Group/Sprint, Clearwire (Lessee) to consider a special use under the Zoning Code to permit the installation of communication antennas and accompanying equipment on the property located at 95 Aster Lane.

The hearing will be held in the Municipal Building, 1900 Hassell Road, Hoffman Estates, Illinois, **Tuesday, November 17, 2009 at 7:30 p.m.**

A map designating the subject site is included on the back of this letter. Appropriate time will be available for questions/comments from interested parties. Should you wish any additional information, please feel free to contact the Planning Division at 847/781-2660. Material is available at the office for review.

Should it become necessary to continue this hearing to an additional date, it will be announced at the conclusion of testimony at the hearings. No further notification of this review will be sent.

Sincerely,

Josh Edwards, Assistant Planner
Department of Development Services

JE/pm

1900 Hassell Road
Hoffman Estates, Illinois 60169
www.hoffmanestates.org

Phone: 847-882-9100
Fax: 847-843-4822

William D. McLeod
MAYOR

Raymond M. Kincaid
TRUSTEE

Gary J. Pilafas
TRUSTEE

Karen V. Mills
TRUSTEE

Jacquelyn Green
TRUSTEE

Bev Romanoff
VILLAGE CLERK

Cary J. Collins
TRUSTEE

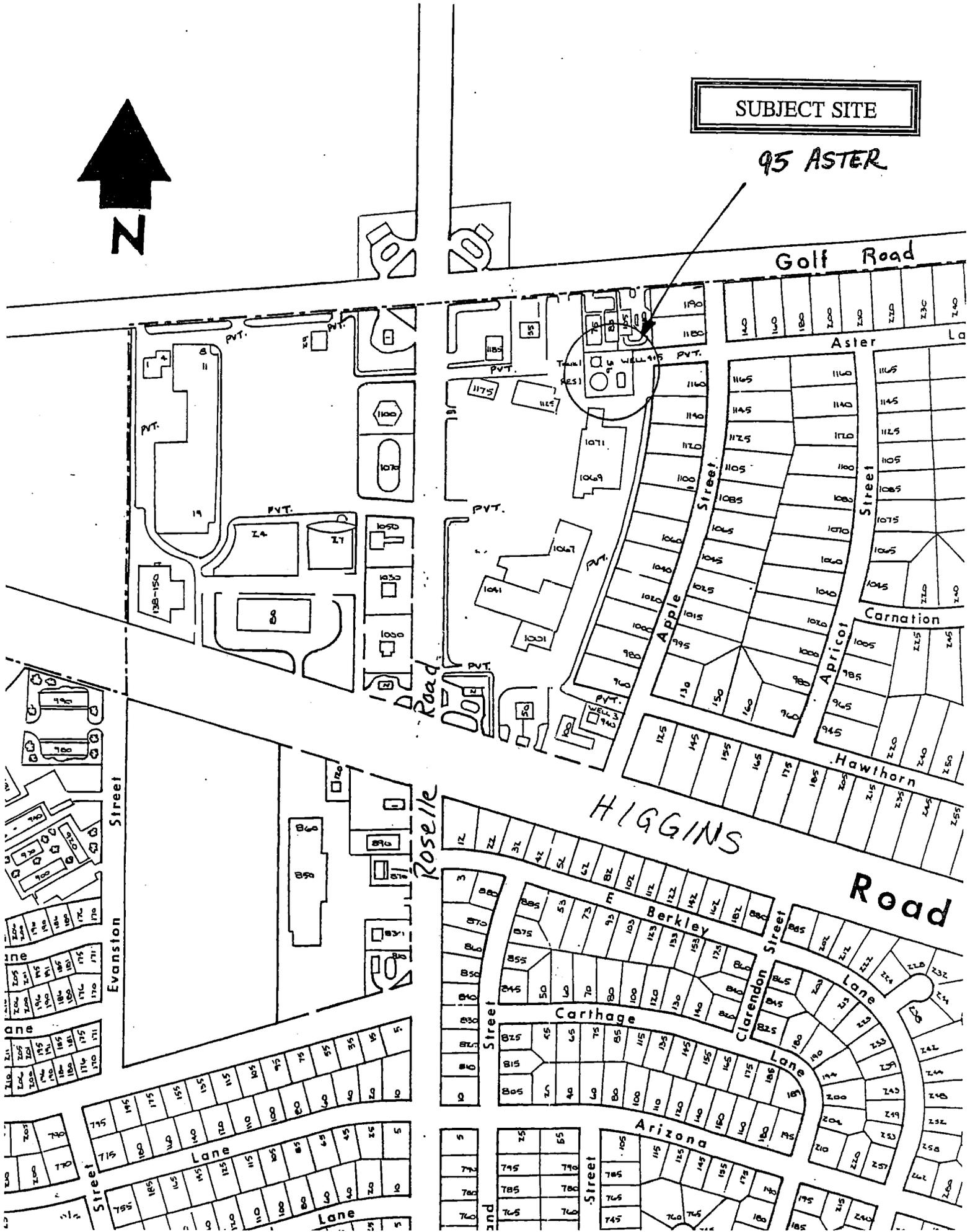
Anna Newell
TRUSTEE

James H. Norris
VILLAGE MANAGER



SUBJECT SITE

95 ASTER



Structural Report

Prepared for: Clear Wireless LLC.

Existing 176' Water Tower

Clearwire Site No. IL-CHI5290 (IL3093)
Golf Center
95 Aster Lane
Hoffman Estates, Cook Co., IL 60169

June 1, 2009



Abraham J. Rokach

Illinois SE License No. 081-004073
Expires 11/30/2010

I certify that this report was prepared by me, or under my direct supervision and control, and, to the best of my knowledge and belief, complies with the requirements of the applicable building code.



Fullerton
Engineering Consultants

9700 West Higgins Road, Suite 800, Rosemont, IL 60018
Phone: (847) 292 0200 Fax: (847) 292 0205

Table of Contents

Project Summary	3
Structural Analysis on Existing Water Tower		
Water Tank Data & Properties	4
Existing Water Tank Structure & New Equipment Scheme	4
Wind Load & Dead Load on Water Tank Structure	5
Existing Antennas & Mounts	8
New Dish	14
Structural Adequacy Checks for Additional Dish Loads	20
Conclusion	63
Appendix		
A. Existing Water Tank Drawings	A.1
B. Manufacturer Data Sheets	B.1

Summary

The Water Tower has been checked with the new antenna loads and is found to be **adequate** in all areas.

The maximum increase in column stress is about 4.07%, which is acceptable by engineering judgment. The maximum rod stress is 8.26 ksi, which is less than the allowable of 18 ksi x 1.33 (wind allowable) = 24 ksi. The maximum strut stress is 0.74 ksi, which is less than the allowable of 3.98 ksi.

Per perimeter pier, the uplift will be increased by 18.6%. The uplift of 103 kips per perimeter is less than the uplift resistance of 124 kips. Therefore the foundation of the tank is adequate for the additional uplift.

The existing (2) 2" dia. anchor bolts per pier will see an increase in stress of 18.6% and are still adequate.

Project Summary

Project: Clearwire Site No. IL-CHI5290 / Hoffman Estates Water Tower
95 Aster Lane
Hoffman Estates, IL 60196

Owner: City of Hoffman Estates

Scope:

1. Water Tank Structural Analysis for Additional Loads due to New Clearwire Dishes.

Design Criteria:

1. International Building Code 2003.
2. TIA/EIA-222-F 1996 (Antennas & Antenna Supporting Structure).
3. ANSI/AWWA D100-96 (Welded Steel Tanks for Water Storage).
4. AISC ASD 9TH Edition.
5. ACI-318-89.

New Equipment Loads:

- (1) VHLP2-23 Dish
- (1) VHLP2-18 Dish
- (2) 1/2" Diameter Cable

Design Load:

- Basic Wind Speed for :

Antennas & Antenna Mounts - 75 mph Fastest Mile / 90 mph (3-second gust) (TIA/EIA-222-F 1996)
Water Tank Structure - 100 mph Fastest Mile / 120 mph (3-second gust) (ANSI/AWWA D100-96)

Reference:

Drawings of the Existing Water Tank :

See Appendix A

Geotechnical report :

N / A

Existing Water Tank Structural Analysis

Water Tank Data & Properties:

Shape Coefficient for Water Tank Structure

Effective Area Coefficient (Flat)	$C_{d_flat} := 1$
Effective Area Coefficient (Cylindrical)	$C_{d_cyl} := 0.6$
Effective Area Coefficient (Sphere)	$C_{d_sphere} := 0.5$

Wind Speed

Effective Wind Speed for Water Tank Structure (AWWA-D100-96)	$V_{eff_wt} := 100\text{mph}$
--	--------------------------------

Water Tank Dimensions

Water Tank

Tank Capacity	$Vol := 100000\text{gal}$
Tank Diameter	$d_{tank} := 28.0\text{ft}$

All Elevations are from Top Existing Grade

Top of Water Tank	$H_{top} := 176\text{ft}$
Top of Capacity Level	$H_{TCL} := 170\text{ft}$
Bottom of Capacity Level	$H_{BCL} := 154\text{ft}$
Mean Height of Water Tank	$H_{mean} := \frac{H_{TCL} + H_{BCL}}{2}$ $H_{mean} = 162\text{ft}$
Bottom of Water Tank	$H_{bot} := 148\text{ft}$

Water Tank Supporting Structure Dimensions

Number of Legs	$N_{leg} := 4$
Number of Panels	$N_{panel} := 4$
Center Column Outer Diameter	$OD_{cen} := 48\text{in}$
Center Column Height	$H_{cen} := 148\text{ft}$
Perimeter Column Inner Diameter	$OD_{leg} := 22\text{in}$
Perimeter Column Height	$H_{leg} := 154\text{ft}$

Water Tank Foundation Dimensions

Weight of Concrete per Perimeter Pier	$W_{leg.pier} := 19.93\text{kip}$
Weight of Earth per Perimeter Pier	$W_{leg.soil} := 3.475\text{kip}$
Number of Anchor Bolts per Perimeter Pier	$N_{leg.bolt} := 2$
Diameter of Anchor Bolts & Threads per Inch	$n_{trd} := 6 \cdot \text{in}^{-1}$ $D_{bolt} := 2\text{in}$
Minimum Root Area of Each Bolt	$A_{bolt} := 0.7854 \cdot \left(D_{bolt} - \frac{1.3}{n_{trd}} \right)^2$ $A_{bolt} \approx 5 \cdot \text{in}^2$

Material Properties

Modulus of Elasticity	$E := 29000\text{ksi}$
Density of Steel	$\text{Density} = 490\text{pcf}$

Wind Load on Water Tank Structure:

Wind Pressure

Pressure := 30psf

Flat $P_{flat} := \text{Pressure} \cdot C_{d_flat} \cdot \left(\frac{V_{eff_wt}}{100\text{mph}} \right)^2$ $P_{flat} = 30 \cdot \text{psf}$

Cylindrical $P_{cyl} := \text{Pressure} \cdot C_{d_cyl} \cdot \left(\frac{V_{eff_wt}}{100\text{mph}} \right)^2$ $P_{cyl} = 18 \cdot \text{psf}$

Sphere $P_{sphere} := \text{Pressure} \cdot C_{d_sphere} \cdot \left(\frac{V_{eff_wt}}{100\text{mph}} \right)^2$ $P_{sphere} = 15 \cdot \text{psf}$

Wind Load, Wind Projected Area & Elevation of Centroid Above Foundation

Water Tank Top
 $A_{top} := 16256\text{in}^2$ $A_{top} = 112.89\text{ft}^2$
 $y_{top} := 2064\text{in}$ $y_{top} = 172\text{ft}$
 $F_{top} := A_{top} \cdot P_{sphere}$ $F_{top} = 1.69 \cdot \text{kip}$

Water Tank Middle
 $A_{mid} := 64512\text{in}^2$ $A_{mid} = 448\text{ft}^2$
 $y_{mid} := 1953.6\text{in}$ $y_{mid} = 162.8\text{ft}$
 $F_{mid} := A_{mid} \cdot P_{cyl}$ $F_{mid} = 8.06 \cdot \text{kip}$

Water Tank Bottom
 $A_{bot} := 16256\text{in}^2$ $A_{bot} = 112.89\text{ft}^2$
 $y_{bot} := 1833.6\text{in}$ $y_{bot} = 152.8\text{ft}$
 $F_{bot} := A_{bot} \cdot P_{sphere}$ $F_{bot} = 1.69 \cdot \text{kip}$

Water Tank Handrail & Balcony
 $A_{rail} := 10\% \cdot (A_{top} + A_{mid} + A_{bot})$ $A_{rail} = 67.38\text{ft}^2$
 $y_{rail} := 156.8\text{ft}$ $y_{rail} = 156.8\text{ft}$
 $F_{rail} := A_{rail} \cdot P_{cyl}$ $F_{rail} = 1.21 \cdot \text{kip}$

Water Tank Perimeter Leg
 $A_{leg} := N_{leg} \cdot OD_{leg}$ $A_{leg} = 7.33 \cdot \frac{\text{ft}^2}{\text{ft}}$
 $F_{leg} := A_{leg} \cdot P_{cyl}$ $F_{leg} = 132 \cdot \text{plf}$

Water Tank Perimeter Leg
 $A_{cen} := OD_{cen}$ $A_{cen} = 4 \cdot \frac{\text{ft}^2}{\text{ft}}$
 $F_{cen} := A_{cen} \cdot P_{cyl}$ $F_{cen} = 72 \cdot \text{plf}$

Water Tank Rods & Struts
 $F_{rod.strut} := 10\% \cdot (F_{cen} + F_{leg})$ $F_{rod.strut} = 20.4 \cdot \text{plf}$

Snow Load on Balcony:

Snow Load	SL := 25psf	SL = 25 · psf
Inside Diameter	ID _{Balcony} := 28ft	ID _{Balcony} = 28 ft
Outside Diameter	OD _{Balcony} := 30.5ft	OD _{Balcony} = 30.5 ft
Area of balcony	$A_{\text{Balcony}} := \frac{\pi \cdot (OD_{\text{Balcony}}^2 - ID_{\text{Balcony}}^2)}{4}$	A _{Balcony} = 114.86 ft ²
Weight of Snow	W _{Snow} := A _{Balcony} · SL	W _{Snow} = 2.87 · kip

Dead Load from Water Tank Structure & Contained Water:

Unit Weight of Water	Unit_w = 62.43 · pcf	
Weight of Water	W _{water} := Unit_w · Vol	W _{water} = 834.54 · kip
Weight of Structure	W _{structure} := 10% · W _{water}	W _{structure} = 83.45 · kip
Weight of Water (Central Pier)	W _{cen.water} := 19% · W _{water}	W _{cen.water} = 158.56 · kip
Weight of Structure (Central Pier)	W _{cen.structure} := 19% · W _{structure}	W _{cen.structure} = 15.86 · kip
Weight of Water (Perimeter Pier)	$W_{\text{leg.water}} := \frac{100\% - 19\%}{N_{\text{leg}}} \cdot W_{\text{water}}$	W _{leg.water} = 168.99 · kip
Weight of Structure (Perimeter Pier)	$W_{\text{leg.structure}} := \frac{100\% - 19\%}{N_{\text{leg}}} \cdot W_{\text{structure}} + \frac{W_{\text{Snow}}}{N_{\text{leg}}}$	W _{leg.structure} = 17.62 · kip

Wind Load on Existing Appurtenances:

Aspect Ratio of Members > 25:

Use $C_{aF} := 2.0$ For Flat Members

Use $C_{aR} := 1.2$ For Round

Members

Existing Standard Post to Balcony Ladder:

Diameter Step(s)

$$d_{\text{step}} := 1\text{in}$$

Size Step Step(s) - Length

$$l_{\text{step}} := 2\text{ft}$$

Size Step(s) - Width

$$w_{\text{step}} := 1\text{ft}$$

Distance between Steps

$$\text{dist}_{\text{step}} := 12\text{in}$$

Number of Step(s) (Conservatively Full Length)

$$n_{\text{step}} := \text{ceil}\left(\frac{y_{\text{rail}}}{\text{dist}_{\text{step}}}\right)$$

$$n_{\text{step}} = 157$$

Weight of Steps

$$\text{Volume}_{\text{step}} := \frac{\pi \cdot d_{\text{step}}^2}{4} \cdot (2 \cdot w_{\text{step}} + l_{\text{step}})$$

$$\text{Volume}_{\text{step}} = 0.02 \cdot \text{ft}^3$$

Density = 490 · pcf

$$\text{Weight}_{\text{step}} := \text{Density} \cdot \text{Volume}_{\text{step}} \cdot \text{ft}^{-1}$$

$$\text{Weight}_{\text{step}} = 10.69 \cdot \text{plf}$$

Wind Load on Step Irons

$$\text{Area}_{\text{step}} := l_{\text{step}} \cdot w_{\text{step}}$$

$$\text{Area}_{\text{step}} = 2 \text{ft}^2$$

pressure := 20psf

$$\text{Wind}_{\text{step}} := C_{aR} \cdot \text{pressure} \cdot \text{Area}_{\text{step}} \cdot \text{ft}^{-1}$$

$$\text{Wind}_{\text{step}} = 48 \cdot \text{plf}$$

Wind Load on Existing Antennas & Equipment :

Wind Pressure @ Antenna Mount Level

- Calculate Wind Loads at Antenna Frame Based on TIA-EIA-222. Use Current Edition "F" For TIA / EIA Standard

$$F_c = q_z \cdot G_H \cdot (\Sigma C_a \cdot A_c) \quad \text{Wind Velocity } v := 75 \text{ mph. Height of Antenna from ground } z_{SL} := 79\text{ft} \quad \text{max.}$$

$$\text{Elevation Coefficient} \quad K_z := \left(\frac{z_{SL}}{33\text{ft}} \right)^{\frac{2}{7}} \quad K_z = 1.28$$

$$\text{Velocity Pressure} \quad q_z := 0.00256 K_z \cdot v^2 \cdot \text{psf} \quad q_z = 18.48 \cdot \text{psf}$$

$$\text{Gust Response Factor} \quad G_H := 0.65 + \frac{0.6}{\left(\frac{z_{SL}}{33\text{ft}} \right)^{\frac{1}{7}}} \quad G_H = 1.18$$

Check that wind pressure (product of velocity pressure and gust response factor) is greater than or equal to 20 psf. If wind pressure is less than 20 psf, use 20 psf, If not use actual.

$$\text{Pressure} := \text{if}(G_H \cdot q_z < 20\text{psf}, 20\text{psf}, G_H \cdot q_z)$$

$$\text{Pressure} = 21.8 \cdot \text{psf}$$

Antennas Properties For All Sectors

Number of Sectors	sector _{SL} := 3	Elev_Ant := z _{SL}	Elev_Ant = 79 ft
Antennas per Sector	n _{Ant,SL} := 2	ANTENNA _{SL} := "CSS-XS4-65-R"	
Antennas Properties	Length _{SL} = 4ft	Width _{SL} = 6.7in	Depth _{SL} = 4.1in
Mounting Brackets			W _{A,SL} = 15lb
			W _{mountSL} = 30lb

$$\text{Aspect Ratio of Antennas} \quad A_{\text{spect,SL}} := \frac{\text{Length}_{SL}}{\text{Width}_{SL}} \quad A_{\text{spect,SL}} = 7.16$$

$$C_{aA,SL} := \begin{cases} C_{aA,SL} \leftarrow 1.4 & \text{if } A_{\text{spect,SL}} < 7 \\ C_{aA,SL} \leftarrow 2.0 & \text{if } A_{\text{spect,SL}} > 25 \\ C_{aA,SL} \leftarrow 1.4 + \frac{A_{\text{spect,SL}} - 7}{30} & \text{otherwise} \end{cases}$$

Therefore Use $C_{aA,SL} = 1.41$ for Antennas

Wind Pressure on Antennas $WL_{A,SL} := \text{Pressure} \cdot C_{aA,SL} \quad WL_{A,SL} = 31 \cdot \text{psf}$

Wind Pressure on Flat Members $WL_F := \text{Pressure} \cdot C_{aF} \quad WL_F = 44 \cdot \text{psf}$

Wind Pressure on Round Members $WL_R := \text{Pressure} \cdot C_{aR} \quad WL_R = 26 \cdot \text{psf}$

For Each Cable :

Number of Cables	$N_{CAB.SL} := \text{sector}_{SL} \cdot n_{Ant.SL}$	$h_{c.cab} := 2\text{in}$ $N_{CAB.SL} = 6$
Weight of Cable (for 1 5/8" Coaxial Cable)		$\text{Weight}_{\text{cable}SL} := 1\text{plf}$
Length of Cable	$L_{CAB.SL} := Z_{SL}$	$L_{CAB.SL} = 79\text{-ft}$
Wind Load per Foot of Cable	$F_{CABSL.ft} := W_{LR} \cdot h_{c.cab}$	$F_{CABSL.ft} = 4.36\text{-plf}$
Dead Load per Foot of Cable	$W_{CABSL.ft} := \text{Weight}_{\text{cable}SL}$	$W_{CABSL.ft} = 1\text{-plf}$
Total Wind Load :	$F_{CAB.SL} := N_{CAB.SL} \cdot F_{CABSL.ft} \cdot L_{CAB.SL}$	$F_{CAB.SL} = 2066.54\text{-lb}$
Total Dead Load :	$W_{CAB.SL} := N_{CAB.SL} \cdot W_{CABSL.ft} \cdot L_{CAB.SL}$	$W_{CAB.SL} = 474\text{-lb}$

For Each Antenna :

	$\text{ANTENNA}_{SL} = \text{"CSS-XS4-65-R"}$	
Area of Each Antenna	$A_{cA.SL} := \text{Width}_{SL} \cdot \text{Length}_{SL}$	$A_{cA.SL} = 2.23\text{ft}^2$
Wind Load for Each Antenna	$F_{cA.SL} := W_{LA.SL} \cdot A_{cA.SL}$	$F_{cA.SL} = 68.42\text{lb}$
Dead Load Each Antenna	$DL_{A.SL} := W_{A.SL} + W_{\text{mount}SL}$	$DL_{A.SL} = 45\text{lb}$
Total Wind Load	$F_{ANT.SL} := n_{Ant.SL} \cdot \text{sector}_{SL} \cdot F_{cA.SL}$	$F_{ANT.SL} = 410.55\text{lb}$
Total Dead Load	$W_{ANT.SL} := n_{Ant.SL} \cdot \text{sector}_{SL} \cdot DL_{A.SL}$	$W_{ANT.SL} = 270\text{lb}$

For Each Antenna Mounting Pipe

	$i := 6$	$\text{pipe}_i = \text{"SW 2" STD Dia"}$	$n_{AMP.SL} := 9$
	$L_{AMP.SL} := 6.0\text{ft}$	$S_{AMP} := S_{\text{pipe}_i}$	$I_{AMP} := I_{\text{pipe}_i}$
		$OD_{AMP} := OD_{\text{pipe}_i}$	
Area of Each Pipe	$A_{AMP.SL} := OD_{AMP} \cdot L_{AMP.SL}$	$A_{AMP.SL} = 1.19\text{ft}^2$	
Wind Load for Each Pipe	$WL_{AMP.SL} := W_{LR} \cdot A_{AMP.SL}$	$WL_{AMP.SL} = 31.06\text{-lb}$	
Dead load for Each Pipe	$DL_{AMP.SL} := \text{Weight}_{\text{pipe}_i} \cdot L_{AMP.SL}$	$DL_{AMP.SL} = 21.90\text{-lb}$	
Total Wind Load	$F_{AMP.SL} := n_{AMP.SL} \cdot WL_{AMP.SL}$	$F_{AMP.SL} = 279.57\text{lb}$	
Total Dead Load	$W_{AMP.SL} := n_{AMP.SL} \cdot DL_{AMP.SL}$	$W_{AMP.SL} = 197.1\text{-lb}$	

Total Loading For Existing Antennas and Mounting Pipes (Other Carriers)

Total Wind Load	$F_{app.SL} := F_{ANT.SL} + F_{AMP.SL}$	$F_{app.SL} = 690.12\text{lb}$
Total Dead Load	$W_{app.SL} := W_{ANT.SL} + W_{AMP.SL}$	$W_{app.SL} = 467.1\text{lb}$

Wind Load on Existing Antennas & Equipment:

Wind Pressure @ Antenna Mount Level

- Calculate Wind Loads at Antenna Frame Based on TIA-EIA-222. Use Current Edition "F" For TIA / EIA Standard

$$F_c = q_z \cdot G_H \cdot (\sum C_a \cdot A_c) \quad \text{Wind Velocity } v_{\text{ref}} := 75 \text{ mph. Height of Antenna from ground } z := 100\text{ft} \quad \text{max.}$$

$$\text{Elevation Coefficient } K_{zz} := \left(\frac{z}{33\text{ft}} \right)^{\frac{2}{7}} \quad K_z = 1.37$$

$$\text{Velocity Pressure } q_z := 0.00256 K_z \cdot v^2 \cdot \text{psf} \quad q_z = 19.77 \cdot \text{psf}$$

$$\text{Gust Response Factor } G_H := 0.65 + \frac{0.6}{\left(\frac{z}{33\text{ft}} \right)^{\frac{1}{7}}} \quad G_H = 1.16$$

Check that wind pressure (product of velocity pressure and gust response factor) is greater than or equal to 20 psf. If wind pressure is less than 20 psf, use 20 psf, If not use actual.

$$\text{Pressure} := \text{if}(G_H \cdot q_z < 20\text{psf}, 20\text{psf}, G_H \cdot q_z)$$

$$\text{Pressure} = 22.97 \cdot \text{psf}$$

Antennas Properties For All Sectors

Number of Sectors	sector := 3	Elev_Ant := z	Elev_Ant = 100 ft
Antennas per Sector	n_Ant := 1	ANTENNA = "HB-X-WM-17-65-00T"	
Antennas Properties	Length = 48.0in	Diameter = 7.32in	W _A = 30.0lb
Mounting Brackets			W _{mount} = 15lb

$$\text{Aspect Ratio of Antennas } A_{\text{spect}} := \frac{\text{Length}}{\text{Diameter}} \quad A_{\text{spect}} = 6.56$$

$$C_{aA} := \begin{cases} C_{aA} \leftarrow 0.8 & \text{if } A_{\text{spect}} < 7 \\ 1.2 & \text{if } A_{\text{spect}} > 25 \\ 0.8 + \frac{A_{\text{spect}} - 7}{30} & \text{otherwise} \end{cases}$$

Therefore Use $C_{aA} = 0.8$ for Antennas

$$\text{Wind Pressure on Antennas } WL_A := \text{Pressure} \cdot C_{aA} \quad WL_A = 18 \cdot \text{psf}$$

$$\text{Wind Pressure on Flat Members } WL_F := \text{Pressure} \cdot C_{aF} \quad WL_F = 46 \cdot \text{psf}$$

$$\text{Wind Pressure on Round Members } WL_R := \text{Pressure} \cdot C_{aR} \quad WL_R = 28 \cdot \text{psf}$$

For Each Cable :

Number of Cables	$N_{CAB} := \text{sector} \cdot n_{Ant}$	$n_{c.cab} := 2in$ W.CAB $N_{CAB} = 3$
Weight of Cable (for 1 5/8" Coaxial Cable)		$\text{Weight}_{cable} := 1\text{plf}$
Length of Cable	$L_{CAB} := z$	$L_{CAB} = 100 \cdot \text{ft}$
Wind Load per Foot of Cable	$F_{CAB.ft} := WL_R \cdot h_{c.cab}$	$F_{CAB.ft} = 4.59 \cdot \text{plf}$
Dead Load per Foot of Cable	$W_{CAB.ft} := \text{Weight}_{cable}$	$W_{CAB.ft} = 1 \cdot \text{plf}$
Total Wind Load :	$F_{CAB} := N_{CAB} \cdot F_{CAB.ft} \cdot L_{CAB}$	$F_{CAB} = 1378.26 \cdot \text{lb}$
Total Dead Load :	$W_{CAB} := N_{CAB} \cdot W_{CAB.ft} \cdot L_{CAB}$	$W_{CAB} = 300 \cdot \text{lb}$

For Each Antenna :

	ANTENNA = "HB-X-WM-17-65-00T"	
Area of Each Antenna	$A_{cA} := \text{Diameter} \cdot \text{Length}$	$A_{cA} = 2.44 \text{ft}^2$
Wind Load for Each Antenna	$F_{cA} := WL_A \cdot A_{cA}$	$F_{cA} = 44.84 \text{lb}$
Dead Load for Each Antenna	$DL_A := W_A + W_{mount}$	$DL_A = 45 \text{lb}$
Total Wind Load	$F_{ANT} := n_{Ant} \cdot \text{sector} \cdot F_{cA}$	$F_{ANT} = 134.52 \text{lb}$
Total Dead Load	$W_{ANT} := n_{Ant} \cdot \text{sector} \cdot DL_A$	$W_{ANT} = 135 \text{lb}$

For Each Antenna Mounting Pipe

	$j := 6$	pipe _j = "SW 2" STD Dia"	$n_{AMP} := 3$
$L_{AMP} := 7.0\text{ft}$	S.AMP $S_{AMP} := S_{pipe_j}$	OD.AMP $OD_{AMP} := OD_{pipe_j}$	L.AMP $L_{AMP} := l_{pipe_j}$
Area of Each Pipe	$A_{AMP} := OD_{AMP} \cdot L_{AMP}$		$A_{AMP} = 1.39 \text{ft}^2$
Wind Load for Each Pipe	$WL_{AMP} := WL_R \cdot A_{AMP}$		$WL_{AMP} = 38.19 \cdot \text{lb}$
Dead Load for Each Pipe	$DL_{AMP} := L_{AMP} \cdot \text{Weight}_{pipe_j}$		$DL_{AMP} = 25.55 \cdot \text{lb}$
Total Wind Load	$F_{AMP} := n_{AMP} \cdot WL_{AMP}$		$F_{AMP} = 114.57 \text{lb}$
Total Dead Load	$W_{AMP} := n_{AMP} \cdot DL_{AMP}$		$W_{AMP} = 76.65 \cdot \text{lb}$

Total Loading For Existing Antennas and Mounting Pipes (4G)

Total Wind Load	$F_{app} := F_{ANT} + F_{AMP}$	$F_{app} = 249.09 \text{lb}$
Total Dead Load	$W_{app} := W_{ANT} + W_{AMP}$	$W_{app} = 211.65 \text{lb}$

Wind Load on Existing Antennas & Equipment:

Wind Pressure @ Antenna Mount Level

- Calculate Wind Loads at Antenna Frame Based on TIA-EIA-222. Use Current Edition "F" For TIA / EIA Standard

$$F_c = q_z \cdot G_H \cdot (\sum C_a \cdot A_c) \quad \text{Wind Velocity } v_{\text{max}} := 75 \text{ mph. Height of Antenna from ground } z_{\text{SN}} := 100\text{ft} \quad \text{max.}$$

$$\text{Elevation Coefficient } K_{zz} := \left(\frac{z_{\text{SN}}}{33\text{ft}} \right)^{\frac{2}{7}} \quad K_z = 1.37$$

$$\text{Velocity Pressure } q_z := 0.00256 K_z \cdot v^2 \cdot \text{psf} \quad q_z = 19.77 \cdot \text{psf}$$

$$\text{Gust Response Factor } G_H := 0.65 + \frac{0.6}{\left(\frac{z_{\text{SN}}}{33\text{ft}} \right)^{\frac{1}{7}}} \quad G_H = 1.16$$

Check that wind pressure (product of velocity pressure and gust response factor) is greater than or equal to 20 psf. If wind pressure is less than 20 psf, use 20 psf, if not use actual.

$$\text{Pressure} := \text{if}(G_H \cdot q_z < 20\text{psf}, 20\text{psf}, G_H \cdot q_z)$$

$$\text{Pressure} = 22.97 \cdot \text{psf}$$

Antennas Properties For All Sectors

Number of Sectors	sector _{SN} := 3	Elev_Ant _{SN} := z	Elev_Ant _{SN} = 100 ft
Antennas per Sector	n _{Ant,SN} := 2	ANTENNA _{SN} := "RR90-11-XXXBL"	
Antennas Properties	Length _{SN} = 48.0in	Width _{SN} = 12in	Depth _{SN} = 7in
Mounting Brackets			W _{A,SN} = 18.0lb
			W _{mount,SN} = 15lb

$$\text{Aspect Ratio of Antennas } A_{\text{spect,SN}} := \frac{\text{Length}_{\text{SN}}}{\text{Width}_{\text{SN}}} \quad A_{\text{spect,SN}} = 4$$

$$C_{aA,SN} := \begin{cases} C_{aA,SN} \leftarrow 1.4 & \text{if } A_{\text{spect,SN}} < 7 \\ 2.0 & \text{if } A_{\text{spect,SN}} > 25 \\ 1.4 + \frac{A_{\text{spect,SN}} - 7}{30} & \text{otherwise} \end{cases}$$

Therefore Use $C_{aA,SN} = 1.4$ for Antennas

Wind Pressure on Antennas $WL_{A,SN} := \text{Pressure} \cdot C_{aA,SN} \quad WL_A = 18 \cdot \text{psf}$

Wind Pressure on Flat Members $WL_{FM} := \text{Pressure} \cdot C_{aF} \quad WL_F = 46 \cdot \text{psf}$

Wind Pressure on Round Members $WL_{RM} := \text{Pressure} \cdot C_{aR} \quad WL_R = 28 \cdot \text{psf}$

For Each Cable :

Number of Cables	$N_{CABS.N} := \text{sector}_{SN} \cdot n_{Ant.SN}$	$h_{c.cab} := 2in$ $N_{CABS.N} = 6$
Weight of Cable (for 1 5/8" Coaxial Cable)		$Weight_{CABS.N} := 1plf$
Length of Cable	$L_{CABS.N} := Z_{SN}$	$L_{CABS.N} = 100 \cdot ft$
Wind Load per Foot of Cable	$F_{CABS.N.ft} := WL_R \cdot h_{c.cab}$	$F_{CABS.N.ft} = 4.59 \cdot plf$
Dead Load per Foot of Cable	$W_{CABS.N.ft} := Weight_{CABS.N}$	$W_{CABS.N.ft} = 1 \cdot plf$
Total Wind Load :	$F_{CABS.N} := N_{CABS.N} \cdot F_{CABS.N.ft} \cdot L_{CABS.N}$	$F_{CABS.N} = 2756.51 \cdot lb$
Total Dead Load :	$W_{CABS.N} := N_{CABS.N} \cdot W_{CABS.N.ft} \cdot L_{CABS.N}$	$W_{CABS.N} = 600 \cdot lb$

For Each Antenna :

ANTENNA_{SN} = "RR90-11-XXXBL"

Area of Each Antenna	$A_{cA.SN} := Width_{SN} \cdot Length_{SN}$	$A_{cA.SN} = 4ft^2$
Wind Load for Each Antenna	$F_{cA.SN} := WL_{A.SN} \cdot A_{cA.SN}$	$F_{cA.SN} = 128.64 \cdot lb$
Dead Load for Each Antenna	$DL_{A.SN} := W_{A.SN} + W_{mount.SN}$	$DL_{A.SN} = 33 \cdot lb$
Total Wind Load	$F_{ANT.SN} := n_{Ant.SN} \cdot \text{sector}_{SN} \cdot F_{cA.SN}$	$F_{ANT.SN} = 771.82 \cdot lb$
Total Dead Load	$W_{ANT.SN} := n_{Ant.SN} \cdot \text{sector}_{SN} \cdot DL_{A.SN}$	$W_{ANT.SN} = 198 \cdot lb$

For Each Antenna Mounting Pipe

$j := 6$ pipe_j = "SW 2" STD Dia"

$L_{AMP.SN} := 6.0ft$ $S_{AMP.SN} := S_{pipe_j}$ $OD_{AMP.SN} := OD_{pipe_j}$ $n_{AMP.SN} := 6$
 $I_{AMP.SN} := I_{pipe_j}$

Area of Each Pipe	$A_{AMP.SN} := OD_{AMP.SN} \cdot L_{AMP.SN}$	$A_{AMP.SN} = 1.19ft^2$
Wind Load for Each Pipe	$WL_{AMP.SN} := WL_R \cdot A_{AMP.SN}$	$WL_{AMP.SN} = 32.73 \cdot lb$
Dead Load for Each Pipe	$DL_{AMP.SN} := Weight_{pipe_j} \cdot L_{AMP.SN}$	$DL_{AMP.SN} = 21.90 \cdot lb$
Total Wind Load	$F_{AMP.SN} := n_{AMP.SN} \cdot WL_{AMP.SN}$	$F_{AMP.SN} = 196.4 \cdot lb$
Total Dead Load	$W_{AMP.SN} := n_{AMP.SN} \cdot DL_{AMP.SN}$	$W_{AMP.SN} = 131.4 \cdot lb$

For Each Horizontal Pipe

$j := 8$ pipe_j = "SW 3" STD Dia"

$L_{HP.SN} := 10.0ft$ $S_{HP.SN} := S_{pipe_j}$ $OD_{HP.SN} := OD_{pipe_j}$ $n_{HP.SN} := 4$
 $I_{HP.SN} := I_{pipe_j}$

Area of Each Pipe	$A_{HP.SN} := OD_{HP.SN} \cdot L_{HP.SN}$	$A_{HP.SN} = 2.92ft^2$
Wind Load for Each Pipe	$WL_{HP.SN} := WL_R \cdot A_{HP.SN}$	$WL_{HP.SN} = 80.40 \cdot lb$
Dead Load for Each Pipe	$DL_{HP.SN} := Weight_{pipe_j} \cdot L_{HP.SN}$	$DL_{HP.SN} = 75.80 \cdot lb$
Total Wind Load	$F_{HP.SN} := n_{HP.SN} \cdot WL_{HP.SN}$	$F_{HP.SN} = 321.59 \cdot lb$
Total Dead Load	$W_{HP.SN} := n_{HP.SN} \cdot DL_{HP.SN}$	$W_{HP.SN} = 303.2 \cdot lb$

Total Loading For Existing Antennas and Mounting Pipes (Sprint/Nextel)

Total Wind Load	$F_{app.SN} := F_{ANT.SN} + F_{AMP.SN} + F_{HP.SN}$	$F_{app.SN} = 1289.82 \cdot lb$
Total Dead Load	$W_{app.SN} := W_{ANT.SN} + W_{AMP.SN} + W_{HP.SN}$	$W_{app.SN} = 632.6 \cdot lb$

Wind Load on New Dish Antenna:

Wind Pressure @ Antenna Mount Level

- Calculate Wind Loads at Antenna Frame Based on TIA-EIA-222. Use Current Edition "F" For TIA / EIA Standard

$$F_c = q_z \cdot G_H \cdot (\Sigma C_a \cdot A_c) \quad \text{Wind Velocity } v_{max} := 75 \text{ mph. Height of Antenna from ground } z_{Dish} := 112.0 \text{ft max.}$$

$$\text{Elevation Coefficient } K_{zz} := \left(\frac{z_{Dish}}{33 \text{ft}} \right)^{\frac{2}{7}} \quad K_z = 1.42$$

$$\text{Velocity Pressure } q_{zw} := 0.00256 K_z \cdot v^2 \cdot \text{psf} \quad q_z = 20.42 \cdot \text{psf}$$

$$\text{Gust Response Factor } G_{HW} := 0.65 + \frac{0.6}{\left(\frac{z_{Dish}}{33 \text{ft}} \right)^{\frac{1}{7}}} \quad G_H = 1.15$$

Check that wind pressure (product of velocity pressure and gust response factor) is greater than or equal to 20 psf. If wind pressure is less than 20 psf, use 20 psf, If not use actual.

$$\text{Pressure} := \text{if}(G_H \cdot q_z < 20 \text{psf}, 20 \text{psf}, G_H \cdot q_z) \\ \text{Pressure} = 23.56 \cdot \text{psf}$$

Antennas Properties For All Sectors

$$\text{Number of Sectors } \text{sector}_{Dish1} := 2 \quad \text{Elev_Ant} := z_{Dish} \quad \text{Elev_Ant} = 112 \text{ft}$$

$$\text{Antennas per Sector } n_{AntDish1} := 1 \quad \text{ANTENNA}_{Dish1} := \text{"VHLP2-23"}$$

$$\text{Antennas Properties } \text{Diameter}_{Dish1} := 26 \text{in} \quad W_{ADish1} := 14.0 \text{lb}$$

$$\text{Mounting Brackets } W_{Dishmount1} := 11.5 \text{lb}$$

$$\text{Aspect Ratio of Antennas } A_{spectDish1} := 1 \quad A_{spectDish1} = 1 \text{ For flat members}$$

$$C_{aADish1} := \begin{cases} C_{aADish1} \leftarrow 1.4 & \text{if } A_{spectDish1} < 7 \\ 2.0 & \text{if } A_{spectDish1} > 25 \\ 1.4 + \frac{A_{spectDish1} - 7}{30} & \text{otherwise} \end{cases} \quad \text{Therefore Use } C_{aADish1} = 1.4 \text{ for Dish Antenna}$$

Wind Pressure on Dish 1

$$WL_{ADish1} := \text{Pressure} \cdot C_{aADish1}$$

$$WL_{ADish1} = 33 \cdot \text{psf}$$

Coaxial Cable Properties For All Sectors Routed Outside

$h_{c.cab} := 1in$

Number of Cables	$N_{CABDish} := \text{sector}_{SN} \cdot n_{Ant.SN}$	$N_{CABDish} := 2$
Weight of Cable (for 1/2" Coaxial Cable)		$Weight_{CABDish} := 0.25plf$
Length of Cable	$L_{CABDish} := Z_{Dish}$	$L_{CABDish} = 112 \cdot ft$
Wind Load per Foot of Cable	$F_{CABDish.ft} := WL_R \cdot h_{c.cab}$	$F_{CABDish.ft} = 2.3 \cdot plf$
Dead Load per Foot of Cable	$W_{CABDish.ft} := Weight_{CABDish}$	$W_{CABDish.ft} = 0.25 \cdot plf$
Total Wind Load :	$F_{CABDish} := N_{CABDish} \cdot F_{CABDish.ft} \cdot L_{CABDish}$	$F_{CABDish} = 514.55 \cdot lb$
Total Dead Load :	$W_{CABDish} := N_{CABDish} \cdot W_{CABDish.ft} \cdot L_{CABDish}$	$W_{CABDish} = 56 \cdot lb$

For Dish 1 :

$ANTENNA_{Dish1} = "VHLP2-23"$

Area of Each Dish	$A_{cADish1} := \pi \left(\frac{Diameter_{Dish1}}{2} \right)^2$	$A_{cADish1} = 3.69ft^2$
Wind Load for Each Dish	$F_{cADish1} := WL_{ADish1} \cdot A_{cADish1}$	$F_{cADish1} = 121.61 \cdot lb$
Dead Load for Each Dish	$DL_{ADish1} := W_{ADish1} + W_{Dishmount1}$	$DL_{ADish1} = 25.5 \cdot lb$
Total Wind Load	$F_{ANTDish1} := n_{AntDish1} \cdot \text{sector}_{Dish1} \cdot F_{cADish1}$	$F_{ANTDish1} = 243.21 \cdot lb$
Total Dead Load	$W_{Dish1} := n_{AntDish1} \cdot \text{sector}_{Dish1} \cdot DL_{ADish1}$	$W_{Dish1} = 51 \cdot lb$

Total Loading For Proposed Dishes and Mounting Pipes (Clearwire)

Total Wind Load	$F_{appDish} := F_{ANTDish1}$	$F_{appDish} = 243.21 \cdot lb$
Total Dead Load	$W_{appDish} := W_{Dish1}$	$W_{appDish} = 51 \cdot lb$

New dish is to be mounted on existing mounting pipe with existing antenna, therefore, calculations to determine wind force on mounting frame are shown in previous existing antenna calculations.

Wind Load on Existing Antennas & Equipment:

Wind Pressure @ Antenna Mount Level

- Calculate Wind Loads at Antenna Frame Based on TIA-EIA-222. Use Current Edition "F" For TIA / EIA Standard

$F_c = q_z \cdot G_H \cdot (\sum C_a \cdot A_c)$ Wind Velocity $v_{max} := 75$ mph. Height of Antenna from ground $z_3 := 117$ ft max.

Elevation Coefficient $K_{zz} := \left(\frac{z_3}{33\text{ft}} \right)^{\frac{2}{7}}$ $K_z = 1.44$

Velocity Pressure $q_{zw} := 0.00256 K_z \cdot v^2 \cdot \text{psf}$ $q_z = 20.67 \cdot \text{psf}$

Gust Response Factor $G_H := 0.65 + \frac{0.6}{\left(\frac{z_3}{33\text{ft}} \right)^{\frac{1}{7}}}$ $G_H = 1.15$

Check that wind pressure (product of velocity pressure and gust response factor) is greater than or equal to 20 psf. If wind pressure is less than 20 psf, use 20 psf, If not use actual.

$\text{Pressure} := \text{if}(G_H \cdot q_z < 20\text{psf}, 20\text{psf}, G_H \cdot q_z)$
 Pressure = 23.79 · psf

Antennas Properties For A II Sectors

Number of Sectors	sector ₃ := 3	Elev_Ant := z ₃	Elev_Ant = 117 ft
Antennas per Sector	n _{Ant3} := 4	ANTENNA = "Panel Antenna"	
Antennas Properties	Length = 66in	Width = 8in	Depth = 6in W _{A3} = 30.0lb
Mounting Brackets			W _{mount3} = 15lb

Aspect Ratio of Antennas $A_{\text{spect}} := \frac{\text{Length}}{\text{Width}}$ $A_{\text{spect}} = 8.25$

$C_{aA} := \begin{cases} C_{aA} \leftarrow 1.4 & \text{if } A_{\text{spect}} < 7 \\ 2.0 & \text{if } A_{\text{spect}} > 25 \\ 1.4 + \frac{A_{\text{spect}} - 7}{30} & \text{otherwise} \end{cases}$ Therefore Use $C_{aA} = 1.44$ for Antennas

<u>Wind Pressure on Antennas</u>	$WL_{A3} := \text{Pressure} \cdot C_{aA}$	$WL_{A3} = 34 \cdot \text{psf}$
<u>Wind Pressure on Flat Members</u>	$WL_F := \text{Pressure} \cdot C_{aF}$	$WL_F = 48 \cdot \text{psf}$
<u>Wind Pressure on Round Members</u>	$WL_R := \text{Pressure} \cdot C_{aR}$	$WL_R = 29 \cdot \text{psf}$

For Each Cable :

Number of Cables	$N_{CAB} := \text{sector} \cdot n_{Ant3}$	$h_{c.cab} := 2in$ $N_{CAB} = 12$
Weight of Cable (for 1 5/8" Coaxial Cable)		$Weight_{cable} := 1plf$
Length of Cable	$L_{CAB3} := z_3$	$L_{CAB3} = 117 \cdot ft$
Wind Load per Foot of Cable	$F_{CAB3.ft} := WL_R \cdot h_{c.cab}$	$F_{CAB3.ft} = 4.76 \cdot plf$
Dead Load per Foot of Cable	$W_{CAB3.ft} := Weight_{cable}$	$W_{CAB3.ft} = 1 \cdot plf$
Total Wind Load	$F_{CAB3} := N_{CAB} \cdot F_{CAB3.ft} \cdot L_{CAB3}$	$F_{CAB3} = 6680.24 \cdot lb$
Total Dead Load	$W_{CAB3} := N_{CAB} \cdot W_{CAB3.ft} \cdot L_{CAB3}$	$W_{CAB3} = 1404 \cdot lb$

For Each Antenna :

	ANTENNA = "Panel Antenna"	
Area of Each Antenna	$A_{CA3} := \text{Width} \cdot \text{Length}$	$A_{CA3} = 3.67 \text{ft}^2$
Wind Load for Each Antenna	$F_{CA3} := WL_{A3} \cdot A_{CA3}$	$F_{CA3} = 125.76 \text{lb}$
Dead Load for Each Antenna	$DL_{A3} := W_{A3} + W_{mount3}$	$DL_{A3} = 45 \text{lb}$
Total Wind Load	$F_{ANT3} := n_{Ant3} \cdot \text{sector}_3 \cdot F_{CA3}$	$F_{ANT3} = 1509.08 \text{lb}$
Total Dead Load	$W_{ANT3} := n_{Ant3} \cdot \text{sector}_3 \cdot DL_{A3}$	$W_{ANT3} = 540 \text{lb}$

For Each Antenna Mounting Pipe

	$j := 6$	pipe _j = "SW 2" STD Dia"	$n_{AMP} := 12$
	$L_{AMP} := 7.0ft$	$S_{AMP} := S_{pipe_j}$	$OD_{AMP} := OD_{pipe_j}$
Area of Each Pipe	$A_{AMP3} := OD_{AMP} \cdot L_{AMP}$		$A_{AMP3} = 1.39 \text{ft}^2$
Wind Load for Each Pipe	$WL_{AMP3} := WL_R \cdot A_{AMP3}$		$WL_{AMP3} = 39.55 \cdot lb$
Dead Load for Each Pipe	$DL_{AMP3} := L_{AMP} \cdot Weight_{pipe_j}$		$DL_{AMP3} = 25.55 \cdot lb$
Total Wind Load	$F_{AMP3} := n_{AMP} \cdot WL_{AMP3}$		$F_{AMP3} = 474.61 \text{lb}$
Total Dead Load	$W_{AMP3} := n_{AMP} \cdot DL_{AMP3}$		$W_{AMP3} = 306.6 \cdot lb$

Total Loading For Existing Antennas and Mounting Pipes (Other Carriers)

Total Wind Load	$F_{app3} := F_{ANT3} + F_{AMP3}$	$F_{app3} = 1983.69 \text{lb}$
Total Dead Load	$W_{app3} := W_{ANT3} + W_{AMP3}$	$W_{app3} = 846.6 \text{lb}$

Wind Load on Existing Antennas & Equipment:

Wind Pressure @ Antenna Mount Level

- Calculate Wind Loads at Antenna Frame Based on TIA-EIA-222. Use Current Edition "F" For TIA / EIA Standard

$F_c = q_z \cdot G_H \cdot (\sum C_a \cdot A_c)$ Wind Velocity $v_{max} := 75$ mph. Height of Antenna from ground $z_4 := 156.8$ ft max.

Elevation Coefficient $K_{zz} := \left(\frac{z_4}{33ft} \right)^{\frac{2}{7}}$ $K_z = 1.56$

Velocity Pressure $q_{zw} := 0.00256 K_z \cdot v^2 \cdot psf$ $q_z = 22.48 \cdot psf$

Gust Response Factor $G_{Hw} := 0.65 + \frac{0.6}{\left(\frac{z_4}{33ft} \right)^{\frac{1}{7}}}$ $G_H = 1.13$

Check that wind pressure (product of velocity pressure and gust response factor) is greater than or equal to 20 psf. If wind pressure is less than 20 psf, use 20 psf, If not use actual.

$Pressure := \text{if}(G_H \cdot q_z < 20psf, 20psf, G_H \cdot q_z)$
 Pressure = 25.4 · psf

Antennas Properties For All Sectors

Number of Sectors	sector ₄ := 3	Elev_Ant ₄ := z ₄	Elev_Ant ₄ = 156.8 ft
Antennas per Sector	n _{Ant4} := 2	ANTENNA ₄ ≡ "RR90-11-XXXBL"	
Antennas Properties	Length ₄ ≡ 48.0in	Width ₄ ≡ 12in	Depth ₄ ≡ 7in W _{A4} ≡ 18lb
Mounting Brackets			W _{mount4} ≡ 15lb

Aspect Ratio of Antennas $A_{spect.4} := \frac{Length_4}{Width_4}$ $A_{spect.4} = 4$

$C_{aA.4} := \left| \begin{array}{l} C_{aA.4} \leftarrow \begin{cases} 1.4 & \text{if } A_{spect.4} < 7 \\ 2.0 & \text{if } A_{spect.4} > 25 \\ 1.4 + \frac{A_{spect.4} - 7}{30} & \text{otherwise} \end{cases} \end{array} \right|$ Therefore Use $C_{aA.4} = 1.4$ for Antennas

Wind Pressure on Antennas $WL_{A.4} := Pressure \cdot C_{aA.4}$ $WL_{A.4} = 36 \cdot psf$

Wind Pressure on Flat Members $WL_F := Pressure \cdot C_{aF}$ $WL_F = 51 \cdot psf$

Wind Pressure on Round Members $WL_R := Pressure \cdot C_{aR}$ $WL_R = 30 \cdot psf$

For Each Cable :

Number of Cables	$N_{CAB4} := \text{sector}_4 \cdot n_{Ant4}$	$h_{c.cab} := 2\text{in}$ $N_{CAB4} = 6$
Weight of Cable (for 1 5/8" Coaxial Cable)		$\text{Weight}_{CAB4} := 1\text{plf}$
Length of Cable	$L_{CAB4} := Z_4$	$L_{CAB4} = 156.8 \cdot \text{ft}$
Wind Load per Foot of Cable	$F_{CAB4.ft} := W_{LR} \cdot h_{c.cab}$	$F_{CAB4.ft} = 5.08 \cdot \text{plf}$
Dead Load per Foot of Cable	$W_{CAB4.ft} := \text{Weight}_{CAB4}$	$W_{CAB4.ft} = 1 \cdot \text{plf}$
Total Wind Load :	$F_{CAB4} := N_{CAB4} \cdot F_{CAB4.ft} \cdot L_{CAB4}$	$F_{CAB4} = 4780.16 \cdot \text{lb}$
Total Dead Load :	$W_{CAB4} := N_{CAB4} \cdot W_{CAB4.ft} \cdot L_{CAB4}$	$W_{CAB4} = 940.8 \cdot \text{lb}$

For Each Antenna :

	$\text{ANTENNA}_4 = \text{"RR90-11-XXXBL"}$	
Area of Each Antenna	$A_{CA.4} := \text{Width}_4 \cdot \text{Length}_4$	$A_{CA.4} = 4 \text{ft}^2$
Wind Load for Each Antenna	$F_{CA.4} := W_{LA.4} \cdot A_{CA.4}$	$F_{CA.4} = 142.27 \text{lb}$
Dead Load for Each Antenna	$DL_{A.4} := W_{A4} + W_{\text{mount}4}$	$DL_{A.4} = 33 \text{lb}$
Total Wind Load	$F_{ANT.4} := n_{Ant4} \cdot \text{sector}_4 \cdot F_{CA.4}$	$F_{ANT.4} = 853.6 \text{lb}$
Total Dead Load	$W_{ANT.4} := n_{Ant4} \cdot \text{sector}_4 \cdot DL_{A.4}$	$W_{ANT.4} = 198 \text{lb}$

For Each Antenna Mounting Pipe

	$j := 6$	$\text{pipe}_j = \text{"SW 2" STD Dia"}$	$n_{AMP.4} := 6$
$L_{AMP.4} := 7.0\text{ft}$	$S_{AMP.4} := S_{\text{pipe}_j}$	$OD_{AMP.4} := OD_{\text{pipe}_j}$	$I_{AMP.4} := I_{\text{pipe}_j}$
Area of Each Pipe	$A_{AMP.4} := OD_{AMP.4} \cdot L_{AMP.4}$	$A_{AMP.4} = 1.39 \text{ft}^2$	
Wind Load for Each Pipe	$WL_{AMP.4} := W_{LR} \cdot A_{AMP.4}$	$WL_{AMP.4} = 42.24 \text{lb}$	
Dead Load for Each Pipe	$DL_{AMP.4} := \text{Weight}_{\text{pipe}_j} \cdot L_{AMP.4}$	$DL_{AMP.4} = 25.55 \cdot \text{lb}$	
Total Wind Load	$F_{AMP.4} := n_{AMP.4} \cdot WL_{AMP.4}$	$F_{AMP.4} = 253.41 \text{lb}$	
Total Dead Load	$W_{AMP.4} := n_{AMP.4} \cdot DL_{AMP.4}$	$W_{AMP.4} = 153.3 \text{lb}$	

Total Loading For Existing Antennas and Mounting Pipes (Other Carriers)

Total Wind Load	$F_{app.4} := F_{ANT.4} + F_{AMP.4}$	$F_{app.4} = 1107.01 \text{lb}$
Total Dead Load	$W_{app.4} := W_{ANT.4} + W_{AMP.4}$	$W_{app.4} = 351.3 \text{lb}$

Structural Adequacy Check For Additional Loads Due To New Equipment

Existing Water Tank - Panel 1 (Top Elevation):

AGL_{P0} := 154.8ft

Bending Moment

From Water Tank Top $M_{top} := F_{top} \cdot (y_{top} - AGL_{P0})$ $M_{top} = 29.13 \cdot \text{kip} \cdot \text{ft}$

From Water Tank Mid $M_{mid} := F_{mid} \cdot (y_{mid} - AGL_{P0})$ $M_{mid} = 64.51 \cdot \text{kip} \cdot \text{ft}$

From Water Tank Rail $M_{rail} := F_{rail} \cdot (y_{rail} - AGL_{P0})$ $M_{rail} = 2.43 \cdot \text{kip} \cdot \text{ft}$

From Existing Appurtenances $M_{app.Ex} := F_{app.4} \cdot (z_4 - AGL_{P0})$ $M_{app.Ex} = 2.21 \cdot \text{kip} \cdot \text{ft}$

From Existing Cables $M_{CAB.Ex} := 0.5 F_{CAB4.ft} \cdot (z_4 - AGL_{P0})^2$ $M_{CAB.Ex} = 0.01 \cdot \text{kip} \cdot \text{ft}$

Total Moment w/o Equipment $M_{0_tank} := M_{top} + M_{mid} + M_{rail}$ $M_{0_tank} = 96.06 \cdot \text{kip} \cdot \text{ft}$

Total Moment with Equipment $M_{0_total} := M_{0_tank} + M_{app.Ex} + M_{CAB.Ex}$ $M_{0_total} = 98.29 \cdot \text{kip} \cdot \text{ft}$

Increase in Bending Moment $\Delta M_0 := \frac{M_{0_total} - M_{0_tank}}{M_{0_tank}}$ $\Delta M_0 = 2.32\%$

Existing Water Tank - Panel 1 (Top Elevation):

AGL_{P0} = 154.8 ft

Shear Force

From Water Tank Top	$V_{top} := F_{top}$	$V_{top} = 1.69 \cdot \text{kip}$
From Water Tank Mid	$V_{mid} := F_{mid}$	$V_{mid} = 8.06 \cdot \text{kip}$
From Water Tank Rail	$V_{rail} := F_{rail}$	$V_{rail} = 1.21 \cdot \text{kip}$
From Existing Appurtenances	$V_{app.Ex} := F_{app.4}$	$V_{app.Ex} = 1.11 \cdot \text{kip}$
From Existing Cables	$V_{CAB.Ex} := F_{CAB4.ft} \cdot (Z_4 - AGL_{P0})$	$V_{CAB.Ex} = 0.01 \cdot \text{kip}$
Total Shear Force w/o Equipment	$V_{0_tank} := V_{top} + V_{mid} + V_{rail}$	$V_{0_tank} = 10.97 \cdot \text{kip}$
Total Shear Force with Equipment	$V_{0_total} := V_{0_tank} + V_{app.Ex} + V_{CAB.Ex}$	$V_{0_total} = 12.09 \cdot \text{kip}$
Increase in Shear Force	$\Delta V_0 := \frac{V_{0_total} - V_{0_tank}}{V_{0_tank}}$	$\Delta V_0 = 10.18 \cdot \%$

Existing Water Tank - Panel 1 (Top Elevation):

AGL_{P0} = 154.8 · ft

Axial Force

From Water Tank /Water/

$$P_{\text{water}} := W_{\text{water}}$$

$$P_{\text{water}} = 834.54 \cdot \text{kip}$$

From Water Tank Structure

$$P_{\text{structure}} := \frac{H_{\text{top}} - \text{AGL}_{\text{P0}}}{H_{\text{top}}} \cdot W_{\text{structure}}$$

$$P_{\text{structure}} = 10.05 \cdot \text{kip}$$

From Existing Appurtenances

$$P_{\text{app.Ex}} := W_{\text{app.4}}$$

$$P_{\text{app.Ex}} = 0.35 \cdot \text{kip}$$

From Existing Cables

$$P_{\text{CAB.Ex}} := W_{\text{CAB4.ft}} \cdot (z_4 - \text{AGL}_{\text{P0}})$$

$$P_{\text{CAB.Ex}} = 0 \cdot \text{kip}$$

Total Axial Force w/o Equipment

$$P_{0_tank} := P_{\text{water}} + P_{\text{structure}}$$

$$P_{0_tank} = 844.59 \cdot \text{kip}$$

Total Axial Force with Equipment

$$P_{0_total} := P_{0_tank} + P_{\text{app.Ex}} + P_{\text{CAB.Ex}}$$

$$P_{0_total} = 844.95 \cdot \text{kip}$$

Increase in Axial Force

$$\Delta P_0 := \frac{P_{0_total} - P_{0_tank}}{P_{0_tank}}$$

$$\Delta P_0 = 0.04 \cdot \%$$

Existing Water Tank - Panel 1 (Bottom Elevation) & Panel 2 (Top Elevation):AGL_{P2} := 117ft**Bending Moment**

From Water Tank Top

$$M_{top} := F_{top} \cdot (y_{top} - AGL_{P2})$$

M_{top} = 93.13 · kip · ft

From Water Tank Mid

$$M_{mid} := F_{mid} \cdot (y_{mid} - AGL_{P2})$$

M_{mid} = 369.33 · kip · ft

From Water Tank Bottom

$$M_{bot} := F_{bot} \cdot (y_{bot} - AGL_{P2})$$

M_{bot} = 60.62 · kip · ft

From Water Tank Rail

$$M_{rail} := F_{rail} \cdot (y_{rail} - AGL_{P2})$$

M_{rail} = 48.27 · kip · ft

From Water Tank Perimeter Leg

$$M_{leg} := 0.5 \left[F_{leg} \cdot (y_{rail} - AGL_{P2})^2 \right]$$

M_{leg} = 104.55 · kip · ft

From Water Tank Center Leg

$$M_{Cleg} := 0.5 \left[F_{cen} \cdot (y_{rail} - AGL_{P2})^2 \right]$$

M_{Cleg} = 57.03 · kip · ft

From Water Tank Ladder

$$M_{step} := 0.5 \left[Wind_{step} \cdot (y_{rail} - AGL_{P2})^2 \right]$$

M_{step} = 38.02 · kip · ft

From Existing Appurtenances

$$M_{app.Ex} := F_{app.4} \cdot (z_4 - AGL_{P2})$$

M_{app.Ex} = 44.06 · kip · ft

From Existing Cables

$$M_{CAB.Ex} := 0.5 \left[F_{CAB.4.ft} \cdot (z_4 - AGL_{P2})^2 \right]$$

M_{CAB.Ex} = 4.02 · kip · ft

Total Moment w/o Equipment

$$M_{1_tank} := M_{top} + M_{mid} + M_{bot} + M_{rail} + M_{leg} + M_{Cleg} + M_{step}$$

M_{1_tank} = 770.94 · kip · ft

Total Moment with Equipment

$$M_{1_total} := M_{1_tank} + M_{app.Ex} + M_{CAB.Ex}$$

M_{1_total} = 819.03 · kip · ft

Increase in Bending Moment

$$\Delta M_1 := \frac{M_{1_total} - M_{1_tank}}{M_{1_tank}}$$

ΔM₁ = 6.24 · %

Existing Water Tank - Panel 1 (Bottom Elevation) & Panel 2 (Top Elevation):AGL_{P2} = 117 ft**Shear Force**

From Water Tank Top	$V_{top} := F_{top}$	$V_{top} = 1.69 \cdot \text{kip}$
From Water Tank Mid	$V_{mid} := F_{mid}$	$V_{mid} = 8.06 \cdot \text{kip}$
From Water Tank Bottom	$V_{bot} := F_{bot}$	$V_{bot} = 1.69 \cdot \text{kip}$
From Water Tank Rail	$V_{rail} := F_{rail}$	$V_{rail} = 1.21 \cdot \text{kip}$
From Water Tank Perimeter Leg	$V_{Pleg} := F_{leg} \cdot (y_{rail} - AGL_{P2})$	$V_{Pleg} = 5.25 \cdot \text{kip}$
From Water Tank Center Leg	$V_{Cleg} := F_{cen} \cdot (y_{rail} - AGL_{P2})$	$V_{Cleg} = 2.87 \cdot \text{kip}$
From Water Tank Ladder	$V_{step} := Wind_{step} \cdot (y_{rail} - AGL_{P2})$	$V_{step} = 1.91 \cdot \text{kip}$
From Existing Appurtenances	$V_{app.Ex} := F_{app.4}$	$V_{app.Ex} = 1.11 \cdot \text{kip}$
From Existing Cables	$V_{CAB.Ex} := F_{CAB4.ft} \cdot (z_4 - AGL_{P2})$	$V_{CAB.Ex} = 0.2 \cdot \text{kip}$
Total Shear Force w/o Equipment	$V_{1_tank} := V_{top} + V_{mid} + V_{bot} + V_{rail} + V_{Pleg} + V_{Cleg} + V_{step}$	$V_{1_tank} = 22.69 \cdot \text{kip}$
Total Shear Force with Equipment	$V_{1_total} := V_{1_tank} + V_{app.Ex} + V_{CAB.Ex}$	$V_{1_total} = 24 \cdot \text{kip}$
Increase in Shear Force	$\Delta V_1 := \frac{V_{1_total} - V_{1_tank}}{V_{1_tank}}$	$\Delta V_1 = 5.77\%$

Existing Water Tank - Panel 1 (Bottom Elevation) & Panel 2 (Top Elevation):AGL_{P2} = 117 ft**Axial Force**

From Water Tank /Water/

$$P_{\text{water}} := W_{\text{water}}$$

P_{water} = 834.54 · kip

From Water Tank Structure

$$P_{\text{structure}} := \frac{H_{\text{top}} - \text{AGL}_{\text{P2}}}{H_{\text{top}}} \cdot W_{\text{structure}}$$

P_{structure} = 27.98 · kip

From Existing Appurtenances

$$P_{\text{app.Ex}} := W_{\text{app.4}}$$

P_{app.Ex} = 0.35 · kip

From Existing Cables

$$P_{\text{CAB.Ex}} := W_{\text{CAB4.ft}} \cdot (z_4 - \text{AGL}_{\text{P2}})$$

P_{CAB.Ex} = 0.04 · kip

Total Axial Force w/o Equipment

$$P_{1_tank} := P_{\text{water}} + P_{\text{structure}}$$

P_{1_tank} = 862.52 · kip

Total Axial Force with Equipment

$$P_{1_total} := P_{1_tank} + P_{\text{app.Ex}} + P_{\text{CAB.Ex}}$$

P_{1_total} = 862.91 · kip

Increase in Axial Force

$$\Delta P_1 := \frac{P_{1_total} - P_{1_tank}}{P_{1_tank}}$$

ΔP₁ = 0.05 · %

Existing Water Tank - Panel 2 (Bottom Elevation) & Panel 3 (Top Elevation):AGL_{P2B} := 78.1ft**Bending Moment**

From Water Tank Top	$M_{top} := F_{top} \cdot (y_{top} - AGL_{P2B})$	$M_{top} = 159 \cdot \text{kip} \cdot \text{ft}$
From Water Tank Mid	$M_{mid} := F_{mid} \cdot (y_{mid} - AGL_{P2B})$	$M_{mid} = 683.02 \cdot \text{kip} \cdot \text{ft}$
From Water Tank Bottom	$M_{bot} := F_{bot} \cdot (y_{bot} - AGL_{P2B})$	$M_{bot} = 126.49 \cdot \text{kip} \cdot \text{ft}$
From Water Tank Rail	$M_{rail} := F_{rail} \cdot (y_{rail} - AGL_{P2B})$	$M_{rail} = 95.45 \cdot \text{kip} \cdot \text{ft}$
From Water Tank Perimeter Leg	$M_{leg} := 0.5 \left[F_{leg} \cdot (y_{rail} - AGL_{P2B})^2 \right]$	$M_{leg} = 408.78 \cdot \text{kip} \cdot \text{ft}$
From Water Tank Center Leg	$M_{cleg} := 0.5 \left[F_{cen} \cdot (y_{rail} - AGL_{P2B})^2 \right]$	$M_{cleg} = 222.97 \cdot \text{kip} \cdot \text{ft}$
From Water Tank Ladder	$M_{step} := 0.5 \left[Wind_{step} \cdot (y_{rail} - AGL_{P2B})^2 \right]$	$M_{step} = 148.65 \cdot \text{kip} \cdot \text{ft}$
From Existing Appurtenances	$M_{app,Ex} := F_{app,4} \cdot (z_4 - AGL_{P2B}) \dots$ $+ F_{app,3} \cdot (z_3 - AGL_{P2B}) \dots$ $+ F_{app,SN} \cdot (z_{SN} - AGL_{P2B}) \dots$ $+ F_{app} \cdot (z - AGL_{P2B})$	$M_{app,Ex} = 197.99 \cdot \text{kip} \cdot \text{ft}$
From Existing Cables	$M_{CAB,Ex} := 0.5 \left[F_{CAB,4,ft} \cdot (z_4 - AGL_{P2B})^2 \dots \right]$ $+ F_{CAB,3,ft} \cdot (z_3 - AGL_{P2B})^2 \dots$ $+ F_{CAB,SN,ft} \cdot (z_{SN} - AGL_{P2B})^2 \dots$ $+ F_{CAB,ft} \cdot (z - AGL_{P2B})^2$	$M_{CAB,Ex} = 21.54 \cdot \text{kip} \cdot \text{ft}$
From New Appurtenance	$M_{appDish} := F_{appDish} \cdot (z_{Dish} - AGL_{P2B})$	$M_{appDish} = 8.24 \cdot \text{kip} \cdot \text{ft}$
From New Cables	$M_{CABDish} := 0.5 \left[F_{CABDish,ft} \cdot (z_{Dish} - AGL_{P2B})^2 \right]$	$M_{CABDish} = 1.32 \cdot \text{kip} \cdot \text{ft}$
Total Moment w/o Equipment	$M_{2_tank} := M_{top} + M_{mid} + M_{bot} \dots$ $+ M_{rail} + M_{leg} + M_{cleg} + M_{step}$	$M_{2_tank} = 1844.37 \cdot \text{kip} \cdot \text{ft}$
Total Moment with Equipment	$M_{2_total} := M_{2_tank} \dots$ $+ M_{app,Ex} + M_{appDish} + M_{CABDish} + M_{CAB,Ex}$	$M_{2_total} = 2073.46 \cdot \text{kip} \cdot \text{ft}$
Increase in Bending Moment	$\Delta M_2 := \frac{M_{2_total} - M_{2_tank}}{M_{2_tank}}$	$\Delta M_2 = 12.42 \cdot \%$

Existing Water Tank - Panel 2 (Bottom Elevation) & Panel 3 (Top Elevation):AGL_{P2B} = 78.1 ft**Shear Force**

From Water Tank Top	$V_{top} := F_{top}$	$V_{top} = 1.69 \cdot \text{kip}$
From Water Tank Mid	$V_{mid} := F_{mid}$	$V_{mid} = 8.06 \cdot \text{kip}$
From Water Tank Bottom	$V_{bot} := F_{bot}$	$V_{bot} = 1.69 \cdot \text{kip}$
From Water Tank Rail	$V_{rail} := F_{rail}$	$V_{rail} = 1.21 \cdot \text{kip}$
From Water Tank Perimeter Leg	$V_{Pleg} := F_{leg} \cdot (y_{rail} - AGL_{P2B})$	$V_{Pleg} = 10.39 \cdot \text{kip}$
From Water Tank Center Leg	$V_{Cleg} := F_{cen} \cdot (y_{rail} - AGL_{P2B})$	$V_{Cleg} = 5.67 \cdot \text{kip}$
From Water Tank Ladder	$V_{step} := Wind_{step} \cdot (y_{rail} - AGL_{P2B})$	$V_{step} = 3.78 \cdot \text{kip}$
From Existing Appurtenances	$V_{app.Ex} := F_{app.4} + F_{app3} + F_{app.SN} + F_{app}$	$V_{app.Ex} = 4.63 \cdot \text{kip}$
From Existing Cables	$V_{CAB.Ex} := F_{CAB4.ft} \cdot (z_4 - AGL_{P2B}) \dots$ $+ F_{CAB3.ft} \cdot (z_3 - AGL_{P2B}) \dots$ $+ F_{CABS.N.ft} \cdot (z_{SN} - AGL_{P2B}) \dots$ $+ F_{CAB.ft} \cdot (z - AGL_{P2B})$	$V_{CAB.Ex} = 0.79 \cdot \text{kip}$
From New Appurtenance	$V_{appDish} := F_{appDish}$	$V_{appDish} = 0.24 \cdot \text{kip}$
From New Cable	$V_{CABDish} := F_{CABDish.ft} \cdot (z_{Dish} - AGL_{P2B})$	$V_{CABDish} = 0.08 \cdot \text{kip}$
Total Shear Force w/o Equipment	$V_{2_tank} := V_{top} + V_{mid} + V_{bot} \dots$ $+ V_{rail} + V_{Pleg} + V_{Cleg} + V_{step}$	$V_{2_tank} = 32.5 \cdot \text{kip}$
Total Shear Force with Equipment	$V_{2_total} := V_{2_tank} \dots$ $+ V_{app.Ex} + V_{appDish} + V_{CABDish} + V_{CAB.Ex}$	$V_{2_total} = 38.23 \cdot \text{kip}$
Increase in Shear Force	$\Delta V_2 := \frac{V_{2_total} - V_{2_tank}}{V_{2_tank}}$	$\Delta V_2 = 17.65 \cdot \%$

Existing Water Tank - Panel 2 (Bottom Elevation) & Panel 3 (Top Elevation):AGL_{P2B} = 78.1 ft**Axial Force**

From Water Tank /Water/

$$P_{\text{water}} := W_{\text{water}}$$

P_{water} = 834.54 · kip

From Water Tank Structure

$$P_{\text{structure}} := \frac{H_{\text{top}} - \text{AGL}_{\text{P2B}}}{H_{\text{top}}} \cdot W_{\text{structure}}$$

P_{structure} = 46.42 · kip

From Existing Appurtenances

$$P_{\text{app,Ex}} := W_{\text{app,4}} + W_{\text{app,3}} + W_{\text{app,SN}} + W_{\text{app}}$$

P_{app,Ex} = 2.04 · kip

From Existing Cables

$$P_{\text{CAB,Ex}} := W_{\text{CAB4,ft}} \cdot (z_4 - \text{AGL}_{\text{P2B}}) \dots \\ + W_{\text{CAB3,ft}} \cdot (z_3 - \text{AGL}_{\text{P2B}}) \dots \\ + W_{\text{CABS,ft}} \cdot (z_{\text{SN}} - \text{AGL}_{\text{P2B}}) \dots \\ + W_{\text{CAB,ft}} \cdot (z - \text{AGL}_{\text{P2B}})$$

P_{CAB,Ex} = 0.16 · kip

From New Appurtenance

$$P_{\text{appDish}} := W_{\text{appDish}}$$

P_{appDish} = 0.05 · kip

From New Cable

$$P_{\text{CABDish}} := W_{\text{CABDish,ft}} \cdot (z_{\text{Dish}} - \text{AGL}_{\text{P2B}})$$

P_{CABDish} = 0.01 · kip

Total Axial Force w/o Equipment

$$P_{2_tank} := P_{\text{water}} + P_{\text{structure}}$$

P_{2_tank} = 880.96 · kip

Total Axial Force with Equipment

$$P_{2_total} := P_{2_tank} \dots \\ + P_{\text{app,Ex}} + P_{\text{appDish}} + P_{\text{CABDish}} + P_{\text{CAB,Ex}}$$

P_{2_total} = 883.22 · kip

Increase in Axial Force

$$\Delta P_2 := \frac{P_{2_total} - P_{2_tank}}{P_{2_tank}}$$

ΔP₂ = 0.26 · %

Existing Water Tank - Panel 3 (Bottom Elevation) & Panel 4 (Top Elevation):

$AGL_{P3B} := 39.2\text{ft}$

Bending Moment

From Water Tank Top

$M_{top} := F_{top} \cdot (y_{top} - AGL_{P3B})$

$M_{top} = 224.87 \cdot \text{kip} \cdot \text{ft}$

From Water Tank Mid

$M_{mid} := F_{mid} \cdot (y_{mid} - AGL_{P3B})$

$M_{mid} = 996.71 \cdot \text{kip} \cdot \text{ft}$

From Water Tank Bottom

$M_{bot} := F_{bot} \cdot (y_{bot} - AGL_{P3B})$

$M_{bot} = 192.36 \cdot \text{kip} \cdot \text{ft}$

From Water Tank Rail

$M_{rail} := F_{rail} \cdot (y_{rail} - AGL_{P3B})$

$M_{rail} = 142.63 \cdot \text{kip} \cdot \text{ft}$

From Water Tank Perimeter Leg

$M_{leg} := 0.5 \left[F_{leg} \cdot (y_{rail} - AGL_{P3B})^2 \right]$

$M_{leg} = 912.76 \cdot \text{kip} \cdot \text{ft}$

From Water Tank Center Leg

$M_{Cleg} := 0.5 \left[F_{cen} \cdot (y_{rail} - AGL_{P3B})^2 \right]$

$M_{Cleg} = 497.87 \cdot \text{kip} \cdot \text{ft}$

From Water Tank Ladder

$M_{step} := 0.5 \left[Wind_{step} \cdot (y_{rail} - AGL_{P3B})^2 \right]$

$M_{step} = 331.91 \cdot \text{kip} \cdot \text{ft}$

From Existing Appurtenances

$$M_{app.Ex} := F_{app.4} \cdot (z_4 - AGL_{P3B}) \dots \\ + F_{app3} \cdot (z_3 - AGL_{P3B}) \dots \\ + F_{app.SN} \cdot (z_{SN} - AGL_{P3B}) \dots \\ + F_{app} \cdot (z - AGL_{P3B})$$

$M_{app.Ex} = 378.08 \cdot \text{kip} \cdot \text{ft}$

From Existing Cables

$$M_{CAB.Ex} := 0.5 \left[F_{CAB4.ft} \cdot (z_4 - AGL_{P3B})^2 \dots \right. \\ \left. + F_{CAB3.ft} \cdot (z_3 - AGL_{P3B})^2 \dots \right. \\ \left. + F_{CABS.N.ft} \cdot (z_{SN} - AGL_{P3B})^2 \dots \right. \\ \left. + F_{CAB.ft} \cdot (z - AGL_{P3B})^2 \right]$$

$M_{CAB.Ex} = 66.52 \cdot \text{kip} \cdot \text{ft}$

From New Appurtenance

$M_{appDish} := F_{appDish} \cdot (z_{Dish} - AGL_{P3B})$

$M_{appDish} = 17.71 \cdot \text{kip} \cdot \text{ft}$

From New Cables

$M_{CABDish} := 0.5 \left[F_{CABDish.ft} \cdot (z_{Dish} - AGL_{P3B})^2 \right]$

$M_{CABDish} = 6.09 \cdot \text{kip} \cdot \text{ft}$

Total Moment w/o Equipment

$$M_{3_tank} := M_{top} + M_{mid} + M_{bot} \dots \\ + M_{rail} + M_{leg} + M_{Cleg} + M_{step}$$

$M_{3_tank} = 3299.12 \cdot \text{kip} \cdot \text{ft}$

Total Moment with Equipment

$$M_{3_total} := M_{3_tank} \dots \\ + M_{app.Ex} + M_{appDish} + M_{CABDish} + M_{CAB.Ex}$$

$M_{3_total} = 3767.51 \cdot \text{kip} \cdot \text{ft}$

Increase in Bending Moment

$$\Delta M_3 := \frac{M_{3_total} - M_{3_tank}}{M_{3_tank}}$$

$\Delta M_3 = 14.2\%$

Existing Water Tank - Panel 3 (Bottom Elevation) & Panel 4 (Top Elevation):AGL_{P3B} = 39.2 ft**Shear Force**

From Water Tank Top	$V_{top} := F_{top}$	$V_{top} = 1.69 \cdot \text{kip}$
From Water Tank Mid	$V_{mid} := F_{mid}$	$V_{mid} = 8.06 \cdot \text{kip}$
From Water Tank Bottom	$V_{bot} := F_{bot}$	$V_{bot} = 1.69 \cdot \text{kip}$
From Water Tank Rail	$V_{rail} := F_{rail}$	$V_{rail} = 1.21 \cdot \text{kip}$
From Water Tank Perimeter Leg	$V_{Pleg} := F_{leg} \cdot (y_{rail} - AGL_{P3B})$	$V_{Pleg} = 15.52 \cdot \text{kip}$
From Water Tank Center Leg	$V_{Cleg} := F_{cen} \cdot (y_{rail} - AGL_{P3B})$	$V_{Cleg} = 8.47 \cdot \text{kip}$
From Water Tank Ladder	$V_{step} := Wind_{step} \cdot (y_{rail} - AGL_{P3B})$	$V_{step} = 5.64 \cdot \text{kip}$
From Existing Appurtenances	$V_{app.Ex} := F_{app.4} + F_{app.3} + F_{app.SN} + F_{app}$	$V_{app.Ex} = 4.63 \cdot \text{kip}$
From Existing Cables	$V_{CAB.Ex} := F_{CAB4.ft} \cdot (Z_4 - AGL_{P3B}) \dots$ $+ F_{CAB3.ft} \cdot (Z_3 - AGL_{P3B}) \dots$ $+ F_{CABS.N.ft} \cdot (Z_{SN} - AGL_{P3B}) \dots$ $+ F_{CAR.ft} \cdot (Z - AGL_{P3B})$	$V_{CAB.Ex} = 1.53 \cdot \text{kip}$
From New Appurtenance	$V_{appDish} := F_{appDish}$	$V_{appDish} = 0.24 \cdot \text{kip}$
From New Cable	$V_{CABDish} := F_{CABDish.ft} \cdot (Z_{Dish} - AGL_{P3B})$	$V_{CABDish} = 0.17 \cdot \text{kip}$
Total Shear Force w/o Equipment	$V_{3_tank} := V_{top} + V_{mid} + V_{bot} \dots$ $+ V_{rail} + V_{Pleg} + V_{Cleg} + V_{step}$	$V_{3_tank} = 42.3 \cdot \text{kip}$
Total Shear Force with Equipment	$V_{3_total} := V_{3_tank} \dots$ $+ V_{app.Ex} + V_{appDish} + V_{CABDish} + V_{CAB.Ex}$	$V_{3_total} = 48.87 \cdot \text{kip}$
Increase in Shear Force	$\Delta V_3 := \frac{V_{3_total} - V_{3_tank}}{V_{3_tank}}$	$\Delta V_3 = 15.52 \cdot \%$

Existing Water Tank - Panel 3 (Bottom Elevation) & Panel 4 (Top Elevation):AGL_{P3B} = 39.2 ft**Axial Force**

From Water Tank /Water/

$$P_{\text{water}} := W_{\text{water}}$$

P_{water} = 834.54 · kip

From Water Tank Structure

$$P_{\text{structure}} := \frac{H_{\text{top}} - \text{AGL}_{\text{P3B}}}{H_{\text{top}}} \cdot W_{\text{structure}}$$

P_{structure} = 64.87 · kip

From Existing Appurtenances

$$P_{\text{app.Ex}} := W_{\text{app.4}} + W_{\text{app.3}} + W_{\text{app.SN}} + W_{\text{app}}$$

P_{app.Ex} = 2.04 · kip

From Existing Cables

$$P_{\text{CAB.Ex}} := W_{\text{CAB4.ft}} \cdot (z_4 - \text{AGL}_{\text{P3B}}) \dots \\ + W_{\text{CAB3.ft}} \cdot (z_3 - \text{AGL}_{\text{P3B}}) \dots \\ + W_{\text{CABS.N.ft}} \cdot (z_{\text{SN}} - \text{AGL}_{\text{P3B}}) \dots \\ + W_{\text{CAB.ft}} \cdot (z - \text{AGL}_{\text{P3B}})$$

P_{CAB.Ex} = 0.32 · kip

From New Appurtenance

$$P_{\text{appDish}} := W_{\text{appDish}}$$

P_{appDish} = 0.05 · kip

From New Cable

$$P_{\text{CABDish}} := W_{\text{CABDish.ft}} \cdot (z_{\text{Dish}} - \text{AGL}_{\text{P3B}})$$

P_{CABDish} = 0.02 · kip

Total Axial Force w/o Equipment

$$P_{3_tank} := P_{\text{water}} + P_{\text{structure}}$$

P_{3_tank} = 899.41 · kip

Total Axial Force with Equipment

$$P_{3_total} := P_{3_tank} \dots \\ + P_{\text{app.Ex}} + P_{\text{appDish}} + P_{\text{CABDish}} + P_{\text{CAB.Ex}}$$

P_{3_total} = 901.84 · kip

Increase in Axial Force

$$\Delta P_3 := \frac{P_{3_total} - P_{3_tank}}{P_{3_tank}}$$

ΔP₃ = 0.27 · %

Existing Water Tank - Panel 4 (Bottom Elevation):

AGL_{P4B} := 0.5ft

Bending Moment

From Water Tank Top

$$M_{top} := F_{top} \cdot (y_{top} - AGL_{P4B})$$

M_{top} = 290.41 · kip · ft

From Water Tank Mid

$$M_{mid} := F_{mid} \cdot (y_{mid} - AGL_{P4B})$$

M_{mid} = 1308.79 · kip · ft

From Water Tank Bottom

$$M_{bot} := F_{bot} \cdot (y_{bot} - AGL_{P4B})$$

M_{bot} = 257.89 · kip · ft

From Water Tank Rail

$$M_{rail} := F_{rail} \cdot (y_{rail} - AGL_{P4B})$$

M_{rail} = 189.56 · kip · ft

From Water Tank Perimeter Leg

$$M_{leg} := 0.5 \left[F_{leg} \cdot (y_{rail} - AGL_{P4B})^2 \right]$$

M_{leg} = 1612.36 · kip · ft

From Water Tank Center Leg

$$M_{cleg} := 0.5 \left[F_{cen} \cdot (y_{rail} - AGL_{P4B})^2 \right]$$

M_{cleg} = 879.47 · kip · ft

From Water Tank Ladder

$$M_{step} := 0.5 \left[Wind_{step} \cdot (y_{rail} - AGL_{P4B})^2 \right]$$

M_{step} = 586.31 · kip · ft

From Existing Appurtenances

$$M_{app.Ex} := F_{app.4} \cdot (z_4 - AGL_{P4B}) \dots$$

$$+ F_{app3} \cdot (z_3 - AGL_{P4B}) \dots$$

$$+ F_{app.SN} \cdot (z_{SN} - AGL_{P4B}) \dots$$

$$+ F_{app} \cdot (z - AGL_{P4B}) \dots$$

$$+ F_{app.SL} \cdot (z_{SL} - AGL_{P4B})$$

M_{app.Ex} = 611.42 · kip · ft

From Existing Cables

$$M_{CAB.Ex} := 0.5 \left[F_{CAB4.ft} \cdot (z_4 - AGL_{P4B})^2 \dots \right]$$

$$+ F_{CAB3.ft} \cdot (z_3 - AGL_{P4B})^2 \dots$$

$$+ F_{CABSN.ft} \cdot (z_{SN} - AGL_{P4B})^2 \dots$$

$$+ F_{CAB.ft} \cdot (z - AGL_{P4B})^2 \dots$$

$$+ F_{CABSL.ft} \cdot (z_{SL} - AGL_{P4B})^2$$

M_{CAB.Ex} = 153.27 · kip · ft

From New Appurtenance

$$M_{appDish} := F_{appDish} \cdot (z_{Dish} - AGL_{P4B})$$

M_{appDish} = 27.12 · kip · ft

From New Cables

$$M_{CABDish} := 0.5 \left[F_{CABDish.ft} \cdot (z_{Dish} - AGL_{P4B})^2 \right]$$

M_{CABDish} = 14.28 · kip · ft

Total Moment w/o Equipment

$$M_{4_tank} := M_{top} + M_{mid} + M_{bot} \dots$$

$$+ M_{rail} + M_{leg} + M_{cleg} + M_{step}$$

M_{4_tank} = 5124.79 · kip · ft

Total Moment with Equipment

$$M_{4_total} := M_{4_tank} \dots$$

$$+ M_{app.Ex} + M_{appDish} + M_{CABDish} + M_{CAB.Ex}$$

M_{4_total} = 5930.88 · kip · ft

Increase in Bending Moment

$$\Delta M_4 := \frac{M_{4_total} - M_{4_tank}}{M_{4_tank}}$$

ΔM₄ = 15.73 · %

Existing Water Tank - Panel 4 (Bottom Elevation):AGL_{P4B} = 0.5 ft**Shear Force**

From Water Tank Top	$V_{top} := F_{top}$	$V_{top} = 1.69 \cdot \text{kip}$
From Water Tank Mid	$V_{mid} := F_{mid}$	$V_{mid} = 8.06 \cdot \text{kip}$
From Water Tank Bottom	$V_{bot} := F_{bot}$	$V_{bot} = 1.69 \cdot \text{kip}$
From Water Tank Rail	$V_{rail} := F_{rail}$	$V_{rail} = 1.21 \cdot \text{kip}$
From Water Tank Perimeter Leg	$V_{Pleg} := F_{leg} \cdot (y_{rail} - AGL_{P4B})$	$V_{Pleg} = 20.63 \cdot \text{kip}$
From Water Tank Center Leg	$V_{Cleg} := F_{cen} \cdot (y_{rail} - AGL_{P4B})$	$V_{Cleg} = 11.25 \cdot \text{kip}$
From Water Tank Ladder	$V_{step} := Wind_{step} \cdot (y_{rail} - AGL_{P4B})$	$V_{step} = 7.5 \cdot \text{kip}$
From Existing Appurtenances	$V_{app.Ex} := F_{app.4} + F_{app.3} + F_{app.SN} + F_{app} + F_{app.SL}$	$V_{app.Ex} = 5.32 \cdot \text{kip}$
From Existing Cables	$V_{CAB.Ex} := F_{CAB4.ft} \cdot (z_4 - AGL_{P4B}) \dots$ $+ F_{CAB3.ft} \cdot (z_3 - AGL_{P4B}) \dots$ $+ F_{CABS.N.ft} \cdot (z_{SN} - AGL_{P4B}) \dots$ $+ F_{CAB.ft} \cdot (z - AGL_{P4B}) \dots$ $+ F_{CABSL.ft} \cdot (z_{SL} - AGL_{P4B})$	$V_{CAB.Ex} = 2.6 \cdot \text{kip}$
From New Appurtenance	$V_{appDish} := F_{appDish}$	$V_{appDish} = 0.24 \cdot \text{kip}$
From New Cable	$V_{CABDish} := F_{CABDish.ft} \cdot (z_{Dish} - AGL_{P4B})$	$V_{CABDish} = 0.26 \cdot \text{kip}$
Total Shear Force w/o Equipment	$V_{4_tank} := V_{top} + V_{mid} + V_{bot} \dots$ $+ V_{rail} + V_{Pleg} + V_{Cleg} + V_{step}$	$V_{4_tank} = 52.05 \cdot \text{kip}$
Total Shear Force with Equipment	$V_{4_total} := V_{4_tank} \dots$ $+ V_{app.Ex} + V_{appDish} + V_{CABDish} + V_{CAB.Ex}$	$V_{4_total} = 60.48 \cdot \text{kip}$
Increase in Shear Force	$\Delta V_4 := \frac{V_{4_total} - V_{4_tank}}{V_{4_tank}}$	$\Delta V_4 = 16.18 \cdot \%$

Existing Water Tank - Panel 4 (Bottom Elevation):AGL_{P4B} = 0.5 ft**Axial Force**

From Water Tank /Water/

$$P_{\text{water}} := W_{\text{water}}$$

$$P_{\text{water}} = 834.54 \cdot \text{kip}$$

From Water Tank Structure

$$P_{\text{structure}} := \frac{H_{\text{top}} - \text{AGL}_{\text{P4B}}}{H_{\text{top}}} \cdot W_{\text{structure}}$$

$$P_{\text{structure}} = 83.22 \cdot \text{kip}$$

From Existing Appurtenances

$$P_{\text{app.Ex}} := W_{\text{app.4}} + W_{\text{app.3}} + W_{\text{app.SN}} \dots \\ + W_{\text{app}} + W_{\text{app.SL}}$$

$$P_{\text{app.Ex}} = 2.51 \cdot \text{kip}$$

From Existing Cables

$$P_{\text{CAB.Ex}} := W_{\text{CAB4.ft}} \cdot (z_4 - \text{AGL}_{\text{P4B}}) \dots \\ + W_{\text{CAB3.ft}} \cdot (z_3 - \text{AGL}_{\text{P4B}}) \dots \\ + W_{\text{CABS.N.ft}} \cdot (z_{\text{SN}} - \text{AGL}_{\text{P4B}}) \dots \\ + W_{\text{CAB.ft}} \cdot (z - \text{AGL}_{\text{P4B}}) \dots \\ + W_{\text{CABSL.ft}} \cdot (z_{\text{SL}} - \text{AGL}_{\text{P4B}})$$

$$P_{\text{CAB.Ex}} = 0.55 \cdot \text{kip}$$

From New Appurtenance

$$P_{\text{appDish}} := W_{\text{appDish}}$$

$$P_{\text{appDish}} = 0.05 \cdot \text{kip}$$

From New Cable

$$P_{\text{CABDish}} := W_{\text{CABDish.ft}} \cdot (z_{\text{Dish}} - \text{AGL}_{\text{P4B}})$$

$$P_{\text{CABDish}} = 0.03 \cdot \text{kip}$$

Total Axial Force w/o Equipment

$$P_{4_tank} := P_{\text{water}} + P_{\text{structure}}$$

$$P_{4_tank} = 917.76 \cdot \text{kip}$$

Total Axial Force with Equipment

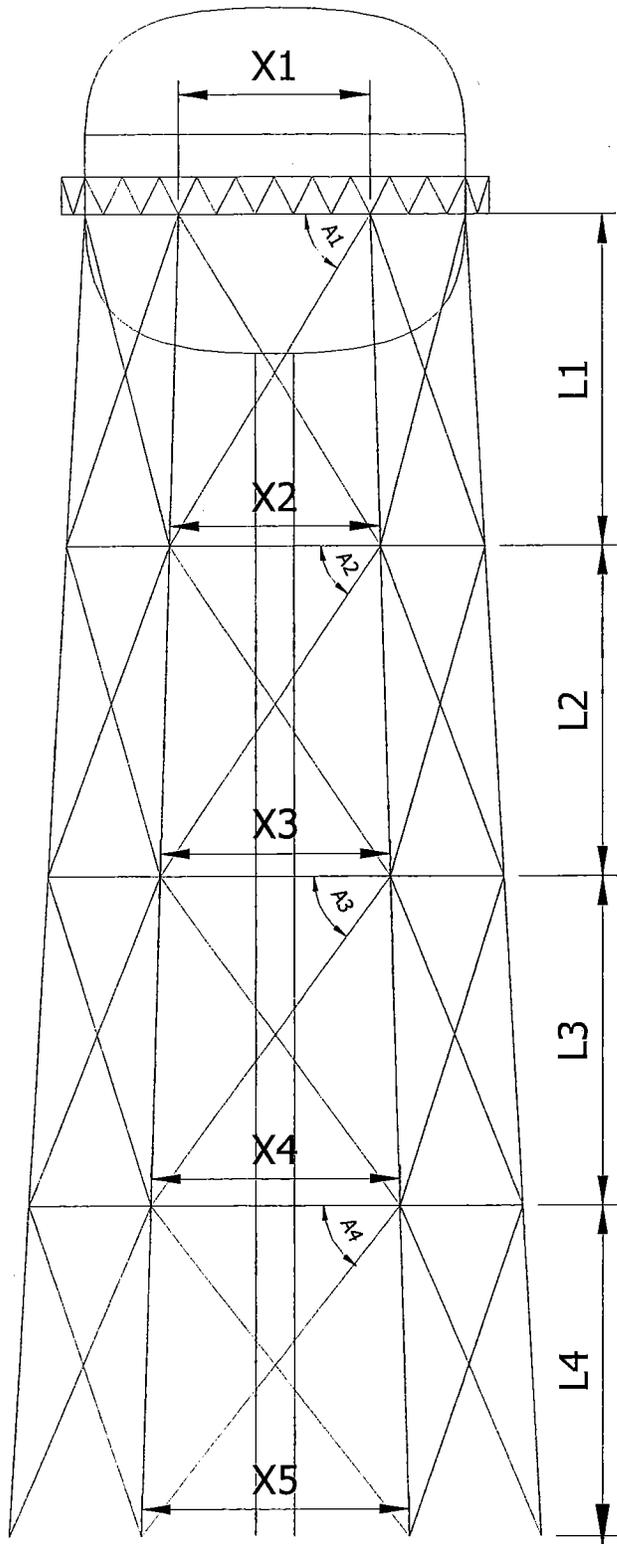
$$P_{4_total} := P_{4_tank} \dots \\ + P_{\text{app.Ex}} + P_{\text{appDish}} + P_{\text{CABDish}} + P_{\text{CAB.Ex}}$$

$$P_{4_total} = 920.9 \cdot \text{kip}$$

Increase in Axial Force

$$\Delta P_4 := \frac{P_{4_total} - P_{4_tank}}{P_{4_tank}}$$

$$\Delta P_4 = 0.34\%$$



WATER TANK ELEVATION

PANEL 1 LENGTH	$L_1 := 37.8\text{ft}$
PANEL 2 LENGTH	$L_2 := 38.92\text{ft}$
PANEL 3 LENGTH	$L_3 := 38.92\text{ft}$
PANEL 3 LENGTH	$L_4 := 39.2\text{ft}$
PANEL WIDTH	$X_1 := 28\text{ft}$
PANEL WIDTH	$X_2 := 32.725\text{ft}$
PANEL WIDTH	$X_3 := 37.591\text{ft}$
PANEL WIDTH	$X_4 := 42.457\text{ft}$
PANEL WIDTH	$X_5 := 47.35\text{ft}$
BRACE ANGLE	$A_1 := 51\text{deg}$
BRACE ANGLE	$A_2 := 48\text{deg}$
BRACE ANGLE	$A_3 := 44\text{deg}$
BRACE ANGLE	$A_4 := 41\text{deg}$

PANEL 1 DATA

COLUMN OUTER DIA.	$D_{o1} := OD_{leg}$	$D_{o1} = 22 \cdot \text{in}$
COLUMN THICKNESS.		$T_{leg} := .25 \text{in}$
COLUMN INNER DIA.	$D_{i1} := D_{o1} - 2 \cdot T_{leg}$	$D_{i1} = 21.5 \cdot \text{in}$
ROD DIA.		$D_{rod1} := 1.125 \text{in}$
STRUT DESCRIPTION		W8x24
STRUT AREA		$A_{strut1} := 7.08 \text{in}^2$

PANEL 1 ANALYSIS

$N_{leg} = 4$

$n := N_{leg}$

n = 4 - number of sides

$\phi := \frac{180^\circ}{n} \quad \phi = 45^\circ$

$M_{top1} := M_{0_tank}$

$M_{top1} = 96.06 \cdot \text{kip} \cdot \text{ft}$

$M_{topA1} := M_{0_total}$

$M_{topA1} = 98.29 \cdot \text{kip} \cdot \text{ft}$

$M_{bot1} := M_{1_tank}$

$M_{bot1} = 770.94 \cdot \text{kip} \cdot \text{ft}$

$M_{botA1} := M_{1_total}$

$M_{botA1} = 819.03 \cdot \text{kip} \cdot \text{ft}$

$R_{top1} := \max\left(\frac{M_{top1}}{3X_1}, \frac{M_{top1}}{4 \cdot X_1 \cdot \cos(\phi)}\right)$

$R_{top1} = 1.21 \cdot \text{kip}$

$R_{topA1} := \max\left(\frac{M_{topA1}}{3X_1}, \frac{M_{topA1}}{4 \cdot X_1 \cdot \cos(\phi)}\right)$

$R_{topA1} = 1.24 \cdot \text{kip}$

$R_{bot1} := \max\left(\frac{M_{bot1}}{3X_2}, \frac{M_{bot1}}{4 \cdot X_2 \cdot \cos(\phi)}\right)$

$R_{bot1} = 8.33 \cdot \text{kip}$

$R_{botA1} := \max\left(\frac{M_{botA1}}{3X_2}, \frac{M_{botA1}}{4 \cdot X_2 \cdot \cos(\phi)}\right)$

$R_{botA1} = 8.85 \cdot \text{kip}$

$W_{add.1} := W_{app.4} + W_{CAB4.ft} \cdot (z_4 - AGL_{P2})$

$W_{add.1} = 0.39 \cdot \text{kip}$

$S_{w1} := \frac{R_{bot1}}{0.25\pi \cdot (D_{o1}^2 - D_{i1}^2)}$

$S_{w1} = 0.49 \cdot \text{ksi}$

$S_{wA1} := \frac{R_{botA1}}{0.25\pi \cdot (D_{o1}^2 - D_{i1}^2)}$

$S_{wA1} = 0.52 \cdot \text{ksi}$

$S_{a1} := \frac{W_{leg.water} + W_{leg.structure}}{0.25\pi \cdot (D_{o1}^2 - D_{i1}^2)}$

$S_{a1} = 10.92 \cdot \text{ksi}$

$S_{aA1} := \frac{W_{leg.water} + W_{leg.structure} + W_{add.1}}{0.25\pi \cdot (D_{o1}^2 - D_{i1}^2)}$

$S_{aA1} = 10.95 \cdot \text{ksi}$

$S_{c1} := S_{w1} + S_{a1}$

$S_{c1} = 11.41 \cdot \text{ksi}$

$S_{cA1} := S_{wA1} + S_{aA1}$

$S_{cA1} = 11.47 \cdot \text{ksi}$

$$F_{r1} := \frac{R_{bot1} - R_{top1}}{2 \cdot \sin(A1) \cdot \sin\left(\frac{180}{N_{leg}} \cdot \text{deg}\right)} \quad F_{r1} = 6.47 \cdot \text{kip}$$

$$F_{rA1} := \frac{R_{botA1} - R_{topA1}}{2 \cdot \sin(A1) \cdot \sin\left(\frac{180}{N_{leg}} \cdot \text{deg}\right)} \quad F_{rA1} = 6.92 \cdot \text{kip}$$

$$S_{r1} := \frac{F_{r1}}{0.25\pi \cdot D_{rod1}^2} \quad S_{r1} = 6.51 \cdot \text{ksi}$$

$$S_{rA1} := \frac{F_{rA1}}{0.25\pi \cdot D_{rod1}^2} \quad S_{rA1} = 6.96 \cdot \text{ksi}$$

$$F_{s1} := \text{if}(A_{strut1} = 0, 0 \text{kip}, F_{r1} \cdot \cos(A1)) \quad F_{s1} = 4074.72 \text{ lb}$$

$$F_{sA1} := \text{if}(A_{strut1} = 0, 0 \text{kip}, F_{rA1} \cdot \cos(A1)) \quad F_{sA1} = 4356.1 \text{ lb}$$

$$S_{s1} := \text{if}\left(A_{strut1} = 0, 0 \cdot \text{ksi}, \frac{F_{s1}}{A_{strut1}}\right) \quad S_{s1} = 0.58 \cdot \text{ksi}$$

$$S_{sA1} := \text{if}\left(A_{strut1} = 0, 0 \cdot \text{ksi}, \frac{F_{sA1}}{A_{strut1}}\right) \quad S_{sA1} = 0.62 \cdot \text{ksi}$$

$$C1 := \frac{S_{cA1} - S_{c1}}{S_{c1}} \quad C1 = 0.47 \cdot \%$$

$$R1 := \frac{S_{rA1} - S_{r1}}{S_{r1}} \quad R1 = 6.91 \cdot \%$$

$$S1 := \frac{S_{sA1} - S_{s1}}{S_{s1}} \quad S1 = 6.91 \cdot \%$$

Water Tower Strut W8x24 - Combination

Allowable Compressive Stress /ASD 9TH Edition/ : $K_{\text{eff}} := 1.0$ $F_{\text{allow}} := 36 \text{ksi}$ $L_{\text{eff}} := X_2$

$I = 18.3 \text{in}^4$ $A := 7.08 \text{in}^2$ $r := \sqrt{\frac{I}{A}}$

$SR := \frac{K \cdot L}{r}$ Slenderness ratio (SR). $SR = 244$

Column slenderness ratio separating elastic and inelastic buckling (see AISC Specification, Eq. (E2-1)):

$C_c := \sqrt{2 \cdot \frac{\pi^2 \cdot E}{F_y}}$ $C_c = 126.1$

Allowable axial stress (AISC Specification, Eqs. (E2-1) and (E2-2) with SR (Slenderness Ratio) substituted for Kl/r : If SR greater than or equal to C_c use E2-2, if less than C_c use E2-1.

$$F_{1aRod} := \begin{cases} \frac{12 \cdot \pi^2 \cdot E}{23 \cdot SR^2} & \text{if } SR \geq C_c \\ \left(1 - \frac{SR^2}{2 \cdot C_c^2}\right) \cdot F_y & \text{otherwise} \\ \frac{5}{3} + \frac{3 \cdot SR}{8 \cdot C_c} - \frac{SR^3}{8 \cdot C_c^3} & \end{cases}$$

$F_{1aRod} = 2.5 \cdot \text{ksi}$

Allowable Compressive Stress (with a 1/3 increase)

$F_{1RodAllow} := F_{1aRod} \cdot \left(1 + \frac{1}{3}\right)$ $F_{1RodAllow} = 3.34 \cdot \text{ksi}$

----- REGIONS -----

Area: 7.08 sq in
 Perimeter: 41.86 in
 Bounding box: X: -3.25 -- 3.25 in
 Y: -3.965 -- 3.965 in
 Centroid: X: 0.00000 in
 Y: 0.00000 in
 Moments of inertia: X: 82.7 sq in sq in
 Y: 18.3 sq in sq in
 Product of inertia: XY: 0 sq in sq in
 Radii of gyration: X: 3.42 in
 Y: 1.61 in
 Principal moments (sq in sq in) and X-Y directions about centroid:
 I_x : 82.7 along [1 0]
 I_y : 18.3 along [0 1]

Water Tower Leg /Column/

COLUMN OUTER DIA. $D_{o1} = 22 \cdot \text{in}$ $D_o := D_{o1}$
COLUMN THICKNESS. $T_{\text{leg}} = 0.25 \cdot \text{in}$
COLUMN INNER DIA. $D_{i1} = 21.5 \cdot \text{in}$ $D_i := D_{i1}$

Allowable Compressive Stress /ASD 9TH Edition/ : $K_{\text{eff}} := 1.0$ $F_{\text{allow}} := 36 \text{ksi}$ $L_{\text{eff}} := L_1$

$$I_{\text{eff}} := \frac{\pi \cdot (D_o^4 - D_i^4)}{64} \quad I = 1010.26 \cdot \text{in}^4$$

$$A_{\text{eff}} := \frac{\pi \cdot (D_o^2 - D_i^2)}{4} \quad A = 17.08 \cdot \text{in}^2$$

$$r_{\text{eff}} := \sqrt{\frac{I}{A}} \quad r = 7.69 \cdot \text{in}$$

$$SR_{\text{eff}} := \frac{K \cdot L}{r} \quad \text{Slenderness ratio (SR).} \quad SR = 59$$

Column slenderness ratio separating elastic and inelastic buckling (see AISC Specification, Eq. (E2-1)):

$$C_c := \sqrt{2 \cdot \frac{\pi^2 \cdot E}{F_y}} \quad C_c = 126.1$$

Allowable axial stress (AISC Specification, Eqs. (E2-1) and (E2-2) with SR (Slenderness Ratio) substituted for K/r : If SR greater than or equal to C_c use E2-2, if less than C_c use E2-1.

$$F_{1a} := \begin{cases} \frac{12 \cdot \pi^2 \cdot E}{23 \cdot SR^2} & \text{if } SR \geq C_c \\ \left(1 - \frac{SR^2}{2 \cdot C_c^2}\right) \cdot F_y & \text{otherwise} \\ \frac{5}{3} + \frac{3 \cdot SR}{8 \cdot C_c} - \frac{SR^3}{8 \cdot C_c^3} & \end{cases} \quad F_{1a} = 17.53 \cdot \text{ksi}$$

Allowable Compressive Stress (with a 1/3 increase)

$$F_{1\text{Allow}} := F_{1a} \cdot \left(1 + \frac{1}{3}\right) \quad F_{1\text{Allow}} = 23.37 \cdot \text{ksi}$$

COLUMN	WITHOUT ANTENNA	WITH ANTENNA
MOMENT FROM WIND LOAD AT TOP OF PANEL	$M_{top1} = 96.06 \cdot \text{kip} \cdot \text{ft}$	$M_{topA1} = 98.29 \cdot \text{kip} \cdot \text{ft}$
COLUMN WIND REACTION AT TOP OF PANEL	$R_{top1} = 1.21 \cdot \text{kip}$	$R_{topA1} = 1.24 \cdot \text{kip}$
MOMENT FROM WIND LOAD AT BOT. OF PANEL	$M_{bot1} = 770.94 \cdot \text{kip} \cdot \text{ft}$	$M_{botA1} = 819.03 \cdot \text{kip} \cdot \text{ft}$
COLUMN WIND REACTION AT BOT. OF PANEL	$R_{bot1} = 8.33 \cdot \text{kip}$	$R_{botA1} = 8.85 \cdot \text{kip}$
COLUMN STRESS FROM WIND	$S_{w1} = 0.49 \cdot \text{ksi}$	$S_{wA1} = 0.52 \cdot \text{ksi}$
COLUMN STRESS FROM AXIAL LOAD	$S_{a1} = 10.92 \cdot \text{ksi}$	$S_{aA1} = 10.95 \cdot \text{ksi}$
TOTAL COLUMN STRESS	$S_{c1} = 11.41 \cdot \text{ksi}$	$S_{cA1} = 11.47 \cdot \text{ksi}$

BRACING ROD	WITHOUT ANTENNA	WITH ANTENNA
BRACING ROD FORCE	$F_{r1} = 6.47 \cdot \text{kip}$	$F_{rA1} = 6.92 \cdot \text{kip}$
BRACING ROD STRESS	$S_{r1} = 6.51 \cdot \text{ksi}$	$S_{rA1} = 6.96 \cdot \text{ksi}$

STRUT	WITHOUT ANTENNA	WITH ANTENNA
STRUT FORCE	$F_{s1} = 4074.72 \text{ lb}$	$F_{sA1} = 4356.1 \text{ lb}$
STRUT STRESS	$S_{s1} = 82875.7 \cdot \text{psf}$	$S_{sA1} = 88598.59 \cdot \text{psf}$

PANEL 1 STRESS SUMMARY	% INCREASE		REMARK
COLUMN STRESS	$C1 = 0.47 \cdot \%$	$S_{cA1} = 11.47 \cdot \text{ksi}$	LESS THAN 23.37ksi, OK
BRACING ROD STRESS	$R1 = 6.91 \cdot \%$	$S_{rA1} = 6.96 \cdot \text{ksi}$	LESS THAN 24ksi, OK
STRUT STRESS	$S1 = 6.91 \cdot \%$	$S_{sA1} = 0.62 \cdot \text{ksi}$	LESS THAN 3.34ksi, OK

PANEL 2 DATA

COLUMN OUTER DIA.	$D_{o2} := OD_{leg}$	$D_{o2} = 22 \cdot \text{in}$
COLUMN THICKNESS.		$T_{leg} := .25 \text{in}$
COLUMN INNER DIA.	$D_{i2} := D_{o2} - 2 \cdot T_{leg}$	$D_{i2} = 21.5 \cdot \text{in}$
ROD DIA.		$D_{rod2} := 1.25 \text{in}$
STRUT DESCRIPTION		W8x31
STRUT AREA		$A_{strut2} := 9.12 \text{in}^2$

PANEL 2 ANALYSIS

$$N_{leg} = 4$$

$$n := N_{leg}$$

$$n = 4 \text{ - number of sides}$$

$$\phi := \frac{180^\circ}{n} \quad \phi = 45^\circ$$

$$M_{top2} := M_{1_tank}$$

$$M_{top2} = 770.94 \cdot \text{kip} \cdot \text{ft}$$

$$M_{topA2} := M_{1_total}$$

$$M_{topA2} = 819.03 \cdot \text{kip} \cdot \text{ft}$$

$$M_{bot2} := M_{2_tank}$$

$$M_{bot2} = 1844.37 \cdot \text{kip} \cdot \text{ft}$$

$$M_{botA2} := M_{2_total}$$

$$M_{botA2} = 2073.46 \cdot \text{kip} \cdot \text{ft}$$

$$R_{top2} := \max\left(\frac{M_{top2}}{3X_2}, \frac{M_{top2}}{4 \cdot X_2 \cdot \cos(\phi)}\right)$$

$$R_{top2} = 8.33 \cdot \text{kip}$$

$$R_{topA2} := \max\left(\frac{M_{topA2}}{3X_2}, \frac{M_{topA2}}{4 \cdot X_2 \cdot \cos(\phi)}\right)$$

$$R_{topA2} = 8.85 \cdot \text{kip}$$

$$R_{bot2} := \max\left(\frac{M_{bot2}}{3X_3}, \frac{M_{bot2}}{4 \cdot X_3 \cdot \cos(\phi)}\right)$$

$$R_{bot2} = 17.35 \cdot \text{kip}$$

$$R_{botA2} := \max\left(\frac{M_{botA2}}{3X_3}, \frac{M_{botA2}}{4 \cdot X_3 \cdot \cos(\phi)}\right)$$

$$R_{botA2} = 19.5 \cdot \text{kip}$$

$$W_{add.2} := W_{app} + W_{CAB.ft} \cdot (z - AGL_{P2B}) + W_{appDish} \dots \\ + W_{CABDish.ft} \cdot (z_{Dish} - AGL_{P2B}) + W_{app.SN} \dots \\ + W_{CABS.N.ft} \cdot (z_{SN} - AGL_{P2B}) + W_{app3} \dots \\ + W_{CAB3.ft} \cdot (z_3 - AGL_{P2B}) + W_{app.4} \dots \\ + W_{CAB4.ft} \cdot (z_4 - AGL_{P2B})$$

$$W_{add.2} = 2.26 \cdot \text{kip}$$

$$S_{w2} := \frac{R_{bot2}}{0.25\pi \cdot (D_{o2}^2 - D_{i2}^2)}$$

$$S_{w2} = 1.02 \cdot \text{ksi}$$

$$S_{wA2} := \frac{R_{botA2}}{0.25\pi \cdot (D_{o2}^2 - D_{i2}^2)}$$

$$S_{wA2} = 1.14 \cdot \text{ksi}$$

$$S_{a2} := \frac{W_{leg.water} + W_{leg.structure}}{0.25\pi \cdot (D_{o2}^2 - D_{i2}^2)}$$

$$S_{a2} = 10.92 \cdot \text{ksi}$$

$$S_{aA2} := \frac{W_{leg.water} + W_{leg.structure} + W_{add.2}}{0.25\pi \cdot (D_{o2}^2 - D_{i2}^2)}$$

$$S_{aA2} = 11.06 \cdot \text{ksi}$$

$$S_{c2} := S_{w2} + S_{a2}$$

$$S_{c2} = 11.94 \cdot \text{ksi}$$

$$S_{cA2} := S_{wA2} + S_{aA2}$$

$$S_{cA2} = 12.2 \cdot \text{ksi}$$

$$F_{r2} := \frac{R_{\text{bot}2} - R_{\text{top}2}}{2 \cdot \sin(A2) \cdot \sin\left(\frac{180}{N_{\text{leg}}} \cdot \text{deg}\right)} \quad F_{r2} = 8.58 \cdot \text{kip}$$

$$F_{rA2} := \frac{R_{\text{bot}A2} - R_{\text{top}A2}}{2 \cdot \sin(A2) \cdot \sin\left(\frac{180}{N_{\text{leg}}} \cdot \text{deg}\right)} \quad F_{rA2} = 10.14 \cdot \text{kip}$$

$$S_{r2} := \frac{F_{r2}}{0.25\pi \cdot D_{\text{rod}2}^2} \quad S_{r2} = 6.99 \cdot \text{ksi}$$

$$S_{rA2} := \frac{F_{rA2}}{0.25\pi \cdot D_{\text{rod}2}^2} \quad S_{rA2} = 8.26 \cdot \text{ksi}$$

$$F_{s2} := \text{if}(A_{\text{strut}2} = 0, 0 \text{kip}, F_{r2} \cdot \cos(A2)) \quad F_{s2} = 5741.39 \text{lb}$$

$$F_{sA2} := \text{if}(A_{\text{strut}2} = 0, 0 \text{kip}, F_{rA2} \cdot \cos(A2)) \quad F_{sA2} = 6782.49 \text{lb}$$

$$S_{s2} := \text{if}\left(A_{\text{strut}2} = 0, 0 \cdot \text{ksi}, \frac{F_{s2}}{A_{\text{strut}2}}\right) \quad S_{s2} = 0.63 \cdot \text{ksi}$$

$$S_{sA2} := \text{if}\left(A_{\text{strut}2} = 0, 0 \cdot \text{ksi}, \frac{F_{sA2}}{A_{\text{strut}2}}\right) \quad S_{sA2} = 0.74 \cdot \text{ksi}$$

$$C2 := \frac{S_{cA2} - S_{c2}}{S_{c2}} \quad C2 = 2.17 \cdot \%$$

$$R2 := \frac{S_{rA2} - S_{r2}}{S_{r2}} \quad R2 = 18.13 \cdot \%$$

$$S2 := \frac{S_{sA2} - S_{s2}}{S_{s2}} \quad S2 = 18.13 \cdot \%$$

Water Tower Strut W8x31 - Combination

Allowable Compressive Stress /ASD 9TH Edition/ : $K_w := 1.0$ $F_w := 36\text{ksi}$ $L_w := X_3$

$I_w := 37.1\text{in}^4$ $A_w := 9.12\text{in}^2$ $r_w := \sqrt{\frac{I_w}{A_w}}$

$SR_w := \frac{K \cdot L}{r}$ Slenderness ratio (SR). $SR = 224$

Column slenderness ratio separating elastic and inelastic buckling (see AISC Specification, Eq. (E2-1)):

$C_c := \sqrt{2 \cdot \frac{\pi^2 \cdot E}{F_y}}$ $C_c = 126.1$

Allowable axial stress (AISC Specification, Eqs. (E2-1) and (E2-2) with SR (Slenderness Ratio) substituted for Kl/r : If SR greater than or equal to C_c use E2-2, if less than C_c use E2-1.

$$F_{2aRod} := \begin{cases} \frac{12 \cdot \pi^2 \cdot E}{23 \cdot SR^2} & \text{if } SR \geq C_c \\ \frac{\left(1 - \frac{SR^2}{2 \cdot C_c^2}\right) \cdot F_y}{\frac{5}{3} + \frac{3 \cdot SR}{8 \cdot C_c} - \frac{SR^3}{8 \cdot C_c^3}} & \text{otherwise} \end{cases}$$

$F_{2aRod} = 2.99 \cdot \text{ksi}$

Allowable Compressive Stress (with a 1/3 increase)

$F_{2RodAllow} := F_{2aRod} \cdot \left(1 + \frac{1}{3}\right)$ $F_{2RodAllow} = 3.98 \cdot \text{ksi}$

----- REGIONS -----

Area: 9.12 sq in
 Perimeter: 48.in
 Bounding box: X: -4 -- 4 in
 Y: -4 -- 4 in
 Centroid: X: 0.00000 in
 Y: 0.00000 in
 Moments of inertia: X: 110 sq in sq in
 Y: 37.1 sq in sq in
 Product of inertia: XY: 0 sq in sq in
 Radii of gyration: X: 3.47 in
 Y: 2.02 in
 Principal moments (sq in sq in) and X-Y directions about centroid:
 I_x : 110 along [1 0]
 I_y : 37.1 along [0 1]

Water Tower Leg /Column/

COLUMN OUTER DIA. $D_{o2} = 22 \cdot \text{in}$ $D_{\text{out}} := D_{o2}$
 COLUMN THICKNESS. $T_{\text{leg}} = 0.25 \cdot \text{in}$
 COLUMN INNER DIA. $D_{i2} = 21.5 \cdot \text{in}$ $D_{\text{in}} := D_{i2}$

Allowable Compressive Stress /ASD 9TH Edition/ : $K_{\text{eff}} := 1.0$ $F_{\text{allow}} := 36 \text{ksi}$ $L_{\text{eff}} := L_2$

$$I_{\text{net}} := \frac{\pi \cdot (D_o^4 - D_i^4)}{64} \quad I = 1010.26 \cdot \text{in}^4$$

$$A_{\text{net}} := \frac{\pi \cdot (D_o^2 - D_i^2)}{4} \quad A = 17.08 \cdot \text{in}^2$$

$$r := \sqrt{\frac{I}{A}} \quad r = 7.69 \cdot \text{in}$$

$$SR_{\text{eff}} := \frac{K \cdot L}{r} \quad \text{Slenderness ratio (SR)} \quad SR = 61$$

Column slenderness ratio separating elastic and inelastic buckling (see AISC Specification, Eq. (E2-1)):

$$C_c := \sqrt{2 \cdot \frac{\pi^2 \cdot E}{F_y}} \quad C_c = 126.1$$

Allowable axial stress (AISC Specification, Eqs. (E2-1) and (E2-2) with SR (Slenderness Ratio) substituted for Kl/r : If SR greater than or equal to C_c use E2-2, if less than C_c use E2-1.

$$F_{2a} := \begin{cases} \frac{12 \cdot \pi^2 \cdot E}{23 \cdot SR^2} & \text{if } SR \geq C_c \\ \left(1 - \frac{SR^2}{2 \cdot C_c^2} \right) \cdot F_y & \text{otherwise} \\ \frac{5}{3} + \frac{3 \cdot SR}{8 \cdot C_c} - \frac{SR^3}{8 \cdot C_c^3} & \end{cases} \quad F_{2a} = 17.36 \cdot \text{ksi}$$

Allowable Compressive Stress (with a 1/3 increase)

$$F_{2\text{Allow}} := F_{2a} \cdot \left(1 + \frac{1}{3} \right) \quad F_{2\text{Allow}} = 23.15 \cdot \text{ksi}$$

COLUMN	WITHOUT ANTENNA	WITH ANTENNA
MOMENT FROM WIND LOAD AT TOP OF PANEL	$M_{top2} = 770.94 \cdot \text{kip} \cdot \text{ft}$	$M_{topA2} = 819.03 \cdot \text{kip} \cdot \text{ft}$
COLUMN WIND REACTION AT TOP OF PANEL	$R_{top2} = 8.33 \cdot \text{kip}$	$R_{topA2} = 8.85 \cdot \text{kip}$
MOMENT FROM WIND LOAD AT BOT. OF PANEL	$M_{bot2} = 1844.37 \cdot \text{kip} \cdot \text{ft}$	$M_{botA2} = 2073.46 \cdot \text{kip} \cdot \text{ft}$
COLUMN WIND REACTION AT BOT. OF PANEL	$R_{bot2} = 17.35 \cdot \text{kip}$	$R_{botA2} = 19.5 \cdot \text{kip}$
COLUMN STRESS FROM WIND	$S_{w2} = 1.02 \cdot \text{ksi}$	$S_{wA2} = 1.14 \cdot \text{ksi}$
COLUMN STRESS FROM AXIAL LOAD	$S_{a2} = 10.92 \cdot \text{ksi}$	$S_{aA2} = 11.06 \cdot \text{ksi}$
TOTAL COLUMN STRESS	$S_{c2} = 11.94 \cdot \text{ksi}$	$S_{cA2} = 12.2 \cdot \text{ksi}$

BRACING ROD	WITHOUT ANTENNA	WITH ANTENNA
BRACING ROD FORCE	$F_{r2} = 8.58 \cdot \text{kip}$	$F_{rA2} = 10.14 \cdot \text{kip}$
BRACING ROD STRESS	$S_{r2} = 6.99 \cdot \text{ksi}$	$S_{rA2} = 8.26 \cdot \text{ksi}$

STRUT	WITHOUT ANTENNA	WITH ANTENNA
STRUT FORCE	$F_{s2} = 5741.39 \text{ lb}$	$F_{sA2} = 6782.49 \text{ lb}$
STRUT STRESS	$S_{s2} = 90653.55 \frac{\text{lb}}{\text{ft}^2}$	$S_{sA2} = 107091.97 \frac{\text{lb}}{\text{ft}^2}$

PANEL 2 STRESS SUMMARY	% INCREASE		REMARK
COLUMN STRESS	C2 = 2.17. %	$S_{cA2} = 12.2 \cdot \text{ksi}$	LESS THAN 23.15ksi, OK
BRACING ROD STRESS	R2 = 18.13. %	$S_{rA2} = 8.26 \cdot \text{ksi}$	LESS THAN 24ksi, OK
STRUT STRESS	S2 = 18.13. %	$S_{sA2} = 0.74 \cdot \text{ksi}$	LESS THAN 3.98ksi, OK

PANEL 3 DATA

COLUMN OUTER DIA.	$D_{o3} := OD_{leg}$	$D_{o3} = 22 \cdot \text{in}$
COLUMN THICKNESS.		$T_{leg} := 0.25 \text{in}$
COLUMN INNER DIA.	$D_{i3} := D_{o3} - 2 \cdot T_{leg}$	$D_{i3} = 21.5 \cdot \text{in}$
ROD DIA.		$D_{rod3} := 1.375 \text{in}$
STRUT DESCRIPTION		W10x49
STRUT AREA		$A_{strut3} := 14.4 \text{in}^2$

PANEL 3 ANALYSIS

$$M_{top3} := M_{2_tank}$$

$$M_{top3} = 1844.37 \cdot \text{kip} \cdot \text{ft}$$

$$M_{topA3} := M_{2_total}$$

$$M_{topA3} = 2073.46 \cdot \text{kip} \cdot \text{ft}$$

$$M_{bot3} := M_{3_tank}$$

$$M_{bot3} = 3299.12 \cdot \text{kip} \cdot \text{ft}$$

$$M_{botA3} := M_{3_total}$$

$$M_{botA3} = 3767.51 \cdot \text{kip} \cdot \text{ft}$$

$$R_{top3} := \max\left(\frac{M_{top3}}{3X_3}, \frac{M_{top3}}{4 \cdot X_3 \cdot \cos(\phi)}\right)$$

$$R_{top3} = 17.35 \cdot \text{kip}$$

$$R_{topA3} := \max\left(\frac{M_{topA3}}{3X_3}, \frac{M_{topA3}}{4 \cdot X_3 \cdot \cos(\phi)}\right)$$

$$R_{topA3} = 19.5 \cdot \text{kip}$$

$$R_{bot3} := \max\left(\frac{M_{bot3}}{3X_4}, \frac{M_{bot3}}{4 \cdot X_4 \cdot \cos(\phi)}\right)$$

$$R_{bot3} = 27.47 \cdot \text{kip}$$

$$R_{botA3} := \max\left(\frac{M_{botA3}}{3X_4}, \frac{M_{botA3}}{4 \cdot X_4 \cdot \cos(\phi)}\right)$$

$$R_{botA3} = 31.37 \cdot \text{kip}$$

$$W_{add.3} := W_{app} + W_{CAB.ft} \cdot (z - AGL_{P3B}) + W_{appDish} \dots \\ + W_{CABDish.ft} \cdot (z_{Dish} - AGL_{P3B}) + W_{app.SN} \dots \\ + W_{CABS.N.ft} \cdot (z_{SN} - AGL_{P3B}) + W_{app3} \dots \\ + W_{CAB3.ft} \cdot (z_3 - AGL_{P3B}) + W_{app.4} \dots \\ + W_{CAB4.ft} \cdot (z_4 - AGL_{P3B}) \dots \\ + W_{app.SL} + W_{CABS.L.ft} \cdot (z_{SL} - AGL_{P3B})$$

$$W_{add.3} = 2.94 \cdot \text{kip}$$

$$S_{w3} := \frac{R_{bot3}}{0.25\pi \cdot (D_{o3}^2 - D_{i3}^2)}$$

$$S_{w3} = 1.61 \cdot \text{ksi}$$

$$S_{wA3} := \frac{R_{botA3}}{0.25\pi \cdot (D_{o3}^2 - D_{i3}^2)}$$

$$S_{wA3} = 1.84 \cdot \text{ksi}$$

$$S_{a3} := \frac{W_{leg.water} + W_{leg.structure}}{0.25\pi \cdot (D_{o3}^2 - D_{i3}^2)}$$

$$S_{a3} = 10.92 \cdot \text{ksi}$$

$$S_{aA3} := \frac{W_{leg.water} + W_{leg.structure} + W_{add.3}}{0.25\pi \cdot (D_{o3}^2 - D_{i3}^2)}$$

$$S_{aA3} = 11.1 \cdot \text{ksi}$$

$$S_{c3} := S_{w3} + S_{a3}$$

$$S_{c3} = 12.53 \cdot \text{ksi}$$

$$S_{cA3} := S_{wA3} + S_{aA3}$$

$$S_{cA3} = 12.93 \cdot \text{ksi}$$

$$F_{r3} := \frac{R_{bot3} - R_{top3}}{2 \cdot \sin(A3) \cdot \sin\left(\frac{180}{N_{leg}} \cdot \text{deg}\right)} \quad F_{r3} = 10.31 \cdot \text{kip}$$

$$F_{rA3} := \frac{R_{botA3} - R_{topA3}}{2 \sin(A3) \cdot \sin\left(\frac{180}{N_{leg}} \cdot \text{deg}\right)} \quad F_{rA3} = 12.08 \cdot \text{kip}$$

$$S_{r3} := \frac{F_{r3}}{0.25\pi \cdot D_{rod3}^2} \quad S_{r3} = 6.94 \cdot \text{ksi}$$

$$S_{rA3} := \frac{F_{rA3}}{0.25\pi \cdot D_{rod3}^2} \quad S_{rA3} = 8.14 \cdot \text{ksi}$$

$$F_{s3} := \text{if}(A_{strut3} = 0, 0, F_{r3} \cdot \cos(A3)) \quad F_{s3} = 7.41 \cdot \text{kip}$$

$$F_{sA3} := \text{if}(A_{strut3} = 0, 0, F_{rA3} \cdot \cos(A3)) \quad F_{sA3} = 8.69 \cdot \text{kip}$$

$$S_{s3} := \text{if}\left(A_{strut3} = 0, 0, \frac{F_{s3}}{A_{strut3}}\right) \quad S_{s3} = 0.51 \cdot \text{ksi}$$

$$S_{sA3} := \text{if}\left(A_{strut3} = 0, 0, \frac{F_{sA3}}{A_{strut3}}\right) \quad S_{sA3} = 0.6 \cdot \text{ksi}$$

$$C3 := \frac{S_{cA3} - S_{c3}}{S_{c3}} \quad C3 = 3.19 \cdot \%$$

$$R3 := \frac{S_{rA3} - S_{r3}}{S_{r3}} \quad R3 = 17.24 \cdot \%$$

$$S3 := \frac{S_{sA3} - S_{s3}}{S_{s3}} \quad S3 = 17.24 \cdot \%$$

Water Tower Strut W10x49 - Combination

Allowable Compressive Stress /ASD 9TH Edition/ : $K_{\text{eff}} := 1.0$ $F_{\text{allow}} := 36 \text{ ksi}$ $L_{\text{eff}} := X_4$

$I_{\text{xx}} := 93.4 \text{ in}^4$ $A_{\text{xx}} := 14.4 \text{ in}^2$ $r_{\text{xx}} := \sqrt{\frac{I}{A}}$

$SR_{\text{xx}} := \frac{K \cdot L}{r}$ Slenderness ratio (SR). $SR = 200$

Column slenderness ratio separating elastic and inelastic buckling (see AISC Specification, Eq. (E2-1)):

$C_c := \sqrt{2 \cdot \frac{\pi^2 \cdot E}{F_y}}$ $C_c = 126.1$

Allowable axial stress (AISC Specification, Eqs. (E2-1) and (E2-2) with SR (Slenderness Ratio) substituted for Kl/r : If SR greater than or equal to C_c use E2-2, if less than C_c use E2-1.

$$F_{3aRod} := \begin{cases} \frac{12 \cdot \pi^2 \cdot E}{23 \cdot SR^2} & \text{if } SR \geq C_c \\ \left(1 - \frac{SR^2}{2 \cdot C_c^2}\right) \cdot F_y & \text{otherwise} \\ \frac{5}{3} + \frac{3 \cdot SR}{8 \cdot C_c} - \frac{SR^3}{8 \cdot C_c^3} & \end{cases} \quad F_{3aRod} = 3.73 \cdot \text{ksi}$$

Allowable Compressive Stress (with a 1/3 increase)

$F_{3RodAllow} := F_{3aRod} \cdot \left(1 + \frac{1}{3}\right)$ $F_{3RodAllow} = 4.98 \cdot \text{ksi}$

REGIONS

Area: 9.12 sq in
 Perimeter: 48.in
 Bounding box: X: -4 -- 4 in
 Y: -4 -- 4 in
 Centroid: X: 0.00000 in
 Y: 0.00000 in
 Moments of inertia: X: 110 sq in sq in
 Y: 37.1 sq in sq in
 Product of inertia: XY: 0 sq in sq in
 Radii of gyration: X: 3.47 in
 Y: 2.02 in
 Principal moments (sq in sq in) and X-Y directions about centroid:
 I_x : 110 along [1 0]
 I_y : 37.1 along [0 1]

Water Tower Leg /Column/

COLUMN OUTER DIA. $D_{o3} = 22 \cdot \text{in}$ $D_{\text{max}} := D_{o3}$
 COLUMN THICKNESS. $T_{\text{leg}} = 0.25 \cdot \text{in}$
 COLUMN INNER DIA. $D_{i3} = 21.5 \cdot \text{in}$ $D_i := D_{i3}$

Allowable Compressive Stress /ASD 9TH Edition/ : $K := 1.0$ $F_{\text{max}} := 36 \text{ksi}$ $L := L_3$

$$I := \frac{\pi \cdot (D_o^4 - D_i^4)}{64} \quad I = 1010.26 \cdot \text{in}^4$$

$$A := \frac{\pi \cdot (D_o^2 - D_i^2)}{4} \quad A = 17.08 \cdot \text{in}^2$$

$$r := \sqrt{\frac{I}{A}} \quad r = 7.69 \cdot \text{in}$$

$$SR := \frac{K \cdot L}{r} \quad \text{Slenderness ratio (SR).} \quad SR = 61$$

Column slenderness ratio separating elastic and inelastic buckling (see AISC Specification, Eq. (E2-1)):

$$C_c := \sqrt{2 \cdot \frac{\pi^2 \cdot E}{F_y}} \quad C_c = 126.1$$

Allowable axial stress (AISC Specification, Eqs. (E2-1) and (E2-2) with SR (Slenderness Ratio) substituted for Kl/r : If SR greater than or equal to C_c use E2-2, if less than C_c use E2-1.

$$F_{3a} := \begin{cases} \frac{12 \cdot \pi^2 \cdot E}{23 \cdot SR^2} & \text{if } SR \geq C_c \\ \frac{\left(1 - \frac{SR^2}{2 \cdot C_c^2}\right) \cdot F_y}{\frac{5}{3} + \frac{3 \cdot SR}{8 \cdot C_c} - \frac{SR^3}{8 \cdot C_c^3}} & \text{otherwise} \end{cases} \quad F_{3a} = 17.36 \cdot \text{ksi}$$

Allowable Compressive Stress (with a 1/3 increase)

$$F_{3\text{Allow}} := F_{3a} \cdot \left(1 + \frac{1}{3}\right) \quad F_{3\text{Allow}} = 23.15 \cdot \text{ksi}$$

COLUMN	WITHOUT ANTENNA	WITH ANTENNA
MOMENT FROM WIND LOAD AT TOP OF PANEL	$M_{top3} = 1844.37 \cdot \text{kip} \cdot \text{ft}$	$M_{topA3} = 2073.46 \cdot \text{kip} \cdot \text{ft}$
COLUMN WIND REACTION AT TOP OF PANEL	$R_{top3} = 17.35 \cdot \text{kip}$	$R_{topA3} = 19.5 \cdot \text{kip}$
MOMENT FROM WIND LOAD AT BOT. OF PANEL	$M_{bot3} = 3299.12 \cdot \text{kip} \cdot \text{ft}$	$M_{botA3} = 3767.51 \cdot \text{kip} \cdot \text{ft}$
COLUMN WIND REACTION AT BOT. OF PANEL	$R_{bot3} = 27.47 \cdot \text{kip}$	$R_{botA3} = 31.37 \cdot \text{kip}$
COLUMN STRESS FROM WIND	$S_{w3} = 1.61 \cdot \text{ksi}$	$S_{wA3} = 1.84 \cdot \text{ksi}$
COLUMN STRESS FROM AXIAL LOAD	$S_{a3} = 10.92 \cdot \text{ksi}$	$S_{aA3} = 11.1 \cdot \text{ksi}$
TOTAL COLUMN STRESS	$S_{c3} = 12.53 \cdot \text{ksi}$	$S_{cA3} = 12.93 \cdot \text{ksi}$

BRACING ROD	WITHOUT ANTENNA	WITH ANTENNA
BRACING ROD FORCE	$F_{r3} = 10.31 \cdot \text{kip}$	$F_{rA3} = 12.08 \cdot \text{kip}$
BRACING ROD STRESS	$S_{r3} = 6.94 \cdot \text{ksi}$	$S_{rA3} = 8.14 \cdot \text{ksi}$

STRUT	WITHOUT ANTENNA	WITH ANTENNA
STRUT FORCE	$F_{s3} = 7.41 \cdot \text{kip}$	$F_{sA3} = 8.69 \cdot \text{kip}$
STRUT STRESS	$S_{s3} = 0.51 \cdot \text{ksi}$	$S_{sA3} = 0.6 \cdot \text{ksi}$

PANEL 3 STRESS SUMMARY	% INCREASE		REMARK
COLUMN STRESS	C3 = 3.19. %	$S_{cA3} = 12.93 \cdot \text{ksi}$	LESS THAN 23.15ksi (5%), OK
BRACING ROD STRESS	R3 = 17.24. %	$S_{rA3} = 8.14 \cdot \text{ksi}$	LESS THAN 24ksi, OK
STRUT STRESS	S3 = 17.24. %	$S_{sA3} = 0.6 \cdot \text{ksi}$	LESS THAN 4.98ksi, OK

PANEL 4 DATA

COLUMN OUTER DIA.	$D_{o4} := OD_{leg}$	$D_{o4} = 22 \cdot \text{in}$
COLUMN THICKNESS.		$T_{leg} := .28125 \text{in}$
COLUMN INNER DIA.	$D_{i4} := D_{o4} - 2 \cdot T_{leg}$	$D_{i4} = 21.44 \cdot \text{in}$
ROD DIA.		$D_{rod4} := 1.5 \text{in}$
STRUT DESCRIPTION		NO STRUT
STRUT AREA		$A_{strut4} := 0 \text{in}^2$

PANEL 4 ANALYSIS

$$M_{top4} := M_{3_tank}$$

$$M_{top4} = 3299.12 \cdot \text{kip} \cdot \text{ft}$$

$$M_{topA4} := M_{3_total}$$

$$M_{topA4} = 3767.51 \cdot \text{kip} \cdot \text{ft}$$

$$M_{bot4} := M_{4_tank}$$

$$M_{bot4} = 5124.79 \cdot \text{kip} \cdot \text{ft}$$

$$M_{botA4} := M_{4_total}$$

$$M_{botA4} = 5930.88 \cdot \text{kip} \cdot \text{ft}$$

$$R_{top4} := \max\left(\frac{M_{top4}}{3X_4}, \frac{M_{top4}}{4 \cdot X_4 \cdot \cos(\phi)}\right)$$

$$R_{top4} = 27.47 \cdot \text{kip}$$

$$R_{topA4} := \max\left(\frac{M_{topA4}}{3X_4}, \frac{M_{topA4}}{4 \cdot X_4 \cdot \cos(\phi)}\right)$$

$$R_{topA4} = 31.37 \cdot \text{kip}$$

$$R_{bot4} := \max\left(\frac{M_{bot4}}{3X_5}, \frac{M_{bot4}}{4 \cdot X_5 \cdot \cos(\phi)}\right)$$

$$R_{bot4} = 38.27 \cdot \text{kip}$$

$$R_{botA4} := \max\left(\frac{M_{botA4}}{3X_5}, \frac{M_{botA4}}{4 \cdot X_5 \cdot \cos(\phi)}\right)$$

$$R_{botA4} = 44.28 \cdot \text{kip}$$

$$W_{add.4} := W_{app} + W_{CAB.r} \cdot (z - AGL_{P4B}) + W_{appDish} \dots \\ + W_{CABDish.ft} \cdot (z_{Dish} - AGL_{P4B}) + W_{app.SN} \dots \\ + W_{CABS.N} \cdot (z_{SN} - AGL_{P4B}) + W_{app3} \dots \\ + W_{CAB3.ft} \cdot (z_3 - AGL_{P4B}) + W_{app.4} \dots \\ + W_{CAB4.ft} \cdot (z_4 - AGL_{P4B}) \dots \\ + W_{app.SL} + W_{CABSL.ft} \cdot (z_{SL} - AGL_{P4B})$$

$$W_{add.4} = 3.14 \cdot \text{kip}$$

$$S_{w4} := \frac{R_{bot4}}{0.25\pi \cdot (D_{o4}^2 - D_{i4}^2)}$$

$$S_{w4} = 1.99 \cdot \text{ksi}$$

$$S_{wA4} := \frac{R_{botA4}}{0.25\pi \cdot (D_{o4}^2 - D_{i4}^2)}$$

$$S_{wA4} = 2.31 \cdot \text{ksi}$$

$$S_{a4} := \frac{W_{leg.water} + W_{leg.structure}}{0.25\pi \cdot (D_{o4}^2 - D_{i4}^2)}$$

$$S_{a4} = 9.72 \cdot \text{ksi}$$

$$S_{aA4} := \frac{W_{leg.water} + W_{leg.structure} + W_{add.4}}{0.25\pi \cdot (D_{o4}^2 - D_{i4}^2)}$$

$$S_{aA4} = 9.89 \cdot \text{ksi}$$

$$S_{c4} := S_{w4} + S_{a4}$$

$$S_{c4} = 11.72 \cdot \text{ksi}$$

$$S_{cA4} := S_{wA4} + S_{aA4}$$

$$S_{cA4} = 12.2 \cdot \text{ksi}$$

$$F_{r4} := \frac{R_{bot4} - R_{top4}}{2 \cdot \sin(A4) \cdot \sin\left(\frac{180}{N_{leg}} \cdot \text{deg}\right)} \quad F_{r4} = 11.63 \cdot \text{kip}$$

$$F_{rA4} := \frac{R_{botA4} - R_{topA4}}{2 \sin(A4) \cdot \sin\left(\frac{180}{N_{leg}} \cdot \text{deg}\right)} \quad F_{rA4} = 13.92 \cdot \text{kip}$$

$$S_{r4} := \frac{F_{r4}}{0.25\pi \cdot D_{rod4}^2} \quad S_{r4} = 6.58 \cdot \text{ksi}$$

$$S_{rA4} := \frac{F_{rA4}}{0.25\pi \cdot D_{rod4}^2} \quad S_{rA4} = 7.87 \cdot \text{ksi}$$

$$F_{s4} := \text{if}(A_{strut4} = 0, 0, F_{r4} \cdot \cos(A4)) \quad F_{s4} = 0 \cdot \text{kip}$$

$$F_{sA4} := \text{if}(A_{strut4} = 0, 0, F_{rA4} \cdot \cos(A4)) \quad F_{sA4} = 0 \cdot \text{kip}$$

$$S_{s4} := \text{if}\left(A_{strut4} = 0, 0, \frac{F_{s4}}{A_{strut4}}\right) \quad S_{s4} = 0 \cdot \text{ksi}$$

$$S_{sA4} := \text{if}\left(A_{strut4} = 0, 0, \frac{F_{sA4}}{A_{strut4}}\right) \quad S_{sA4} = 0 \cdot \text{ksi}$$

$$C4 := \frac{S_{cA4} - S_{c4}}{S_{c4}} \quad C4 = 4.07 \cdot \%$$

$$R4 := \frac{S_{rA4} - S_{r4}}{S_{r4}} \quad R4 = 19.63 \cdot \%$$

$$S4 := \frac{S_{sA4} - S_{s4}}{S_{s4}} \quad S4 = 0 \cdot \%$$

Water Tower Leg /Column/

COLUMN OUTER DIA. $D_{o4} = 22 \cdot \text{in}$ $\overset{D_{o4}}{\overset{\text{mm}}{\text{mm}}} := D_{o4}$
 COLUMN THICKNESS. $T_{leg} = 0.2813 \cdot \text{in}$
 COLUMN INNER DIA. $D_{i4} = 21.44 \cdot \text{in}$ $\overset{D_{i4}}{\overset{\text{mm}}{\text{mm}}} := D_{i4}$

Allowable Compressive Stress /ASD 9TH Edition/ : $K_{\text{eff}} := 1.0$ $F_{\text{allow}} := 36 \text{ksi}$ $L_{\text{eff}} := L_4$

$$I_{\text{eff}} := \frac{\pi \cdot (D_o^4 - D_i^4)}{64} \quad I = 1131.7 \cdot \text{in}^4$$

$$A_{\text{eff}} := \frac{\pi \cdot (D_o^2 - D_i^2)}{4} \quad A = 19.19 \cdot \text{in}^2$$

$$r_{\text{eff}} := \sqrt{\frac{I}{A}} \quad r = 7.68 \cdot \text{in}$$

$$SR_{\text{eff}} := \frac{K \cdot L}{r} \quad \text{Slenderness ratio (SR).} \quad SR = 61$$

Column slenderness ratio separating elastic and inelastic buckling (see AISC Specification, Eq. (E2-1)):

$$C_c := \sqrt{2 \cdot \frac{\pi^2 \cdot E}{F_y}} \quad C_c = 126.1$$

Allowable axial stress (AISC Specification, Eqs. (E2-1) and (E2-2) with SR (Slenderness Ratio) substituted for Kl/r : If SR greater than or equal to C_c use E2-2, if less than C_c use E2-1.

$$F_{4a} := \begin{cases} \frac{12 \cdot \pi^2 \cdot E}{23 \cdot SR^2} & \text{if } SR \geq C_c \\ \left(1 - \frac{SR^2}{2 \cdot C_c^2} \right) \cdot F_y & \text{otherwise} \\ \frac{5}{3} + \frac{3 \cdot SR}{8 \cdot C_c} - \frac{SR^3}{8 \cdot C_c^3} & \end{cases} \quad F_{4a} = 17.31 \cdot \text{ksi}$$

Allowable Compressive Stress (with a 1/3 increase)

$$F_{4\text{Allow}} := F_{4a} \cdot \left(1 + \frac{1}{3} \right) \quad F_{4\text{Allow}} = 23.08 \cdot \text{ksi}$$

COLUMN		WITHOUT ANTENNA	WITH ANTENNA
MOMENT FROM WIND LOAD AT TOP OF PANEL		$M_{top4} = 3299.12 \cdot \text{kip} \cdot \text{ft}$	$M_{topA4} = 3767.51 \cdot \text{kip} \cdot \text{ft}$
COLUMN WIND REACTION AT TOP OF PANEL		$R_{top4} = 27.47 \cdot \text{kip}$	$R_{topA4} = 31.37 \cdot \text{kip}$
MOMENT FROM WIND LOAD AT BOT. OF PANEL		$M_{bot4} = 5124.79 \cdot \text{kip} \cdot \text{ft}$	$M_{botA4} = 5930.88 \cdot \text{kip} \cdot \text{ft}$
COLUMN WIND REACTION AT BOT. OF PANEL		$R_{bot4} = 38.27 \cdot \text{kip}$	$R_{botA4} = 44.28 \cdot \text{kip}$
COLUMN STRESS FROM WIND		$S_{w4} = 1.99 \cdot \text{ksi}$	$S_{wA4} = 2.31 \cdot \text{ksi}$
COLUMN STRESS FROM AXIAL LOAD		$S_{a4} = 9.72 \cdot \text{ksi}$	$S_{aA4} = 9.89 \cdot \text{ksi}$
TOTAL COLUMN STRESS		$S_{c4} = 11.72 \cdot \text{ksi}$	$S_{cA4} = 12.2 \cdot \text{ksi}$
BRACING ROD		WITHOUT ANTENNA	WITH ANTENNA
BRACING ROD FORCE		$F_{r4} = 11.63 \cdot \text{kip}$	$F_{rA4} = 13.92 \cdot \text{kip}$
BRACING ROD STRESS		$S_{r4} = 6.58 \cdot \text{ksi}$	$S_{rA4} = 7.87 \cdot \text{ksi}$
STRUT		WITHOUT ANTENNA	WITH ANTENNA
STRUT FORCE		$F_{s4} = 0 \cdot \text{kip}$	$F_{sA4} = 0 \cdot \text{kip}$
STRUT STRESS		$S_{s4} = 0 \cdot \text{ksi}$	$S_{sA4} = 0 \cdot \text{ksi}$
PANEL 4 STRESS SUMMARY	% INCREASE		REMARK
COLUMN STRESS	$C4 = 4.07 \cdot \%$	$S_{cA4} = 12.2 \cdot \text{ksi}$	LESS THAN 23.08ksi, OK
BRACING ROD STRESS	$R4 = 19.63 \cdot \%$	$S_{rA4} = 7.87 \cdot \text{ksi}$	LESS THAN 24ksi, OK
STRUT STRESS	$S4 = 0 \cdot \%$	$S_{sA4} = 0 \cdot \text{ksi}$	NO STRUT

WATER TANK STRUCTURE ANALYSIS SUMMARY

		AS BUILT	WITH ANTENNA	% INCREASE	COMMENT
PANEL 1	COLUMN STRESS	$S_{c1} = 11.41 \cdot \text{ksi}$	$S_{cA1} = 11.47 \cdot \text{ksi}$	$C1 = 0.47 \cdot \%$	LESS THAN 23.37ksi, OK
	ROD STRESS	$S_{r1} = 6.51 \cdot \text{ksi}$	$S_{rA1} = 6.96 \cdot \text{ksi}$	$R1 = 6.91 \cdot \%$	LESS THAN 24ksi, OK
	STRUT STRESS	$S_{s1} = 0.58 \cdot \text{ksi}$	$S_{sA1} = 0.62 \cdot \text{ksi}$	$S1 = 6.91 \cdot \%$	LESS THAN 3.34ksi, OK
PANEL 2	COLUMN STRESS	$S_{c2} = 11.94 \cdot \text{ksi}$	$S_{cA2} = 12.2 \cdot \text{ksi}$	$C2 = 2.17 \cdot \%$	LESS THAN 23.15ksi, OK
	ROD STRESS	$S_{r2} = 6.99 \cdot \text{ksi}$	$S_{rA2} = 8.26 \cdot \text{ksi}$	$R2 = 18.13 \cdot \%$	LESS THAN 24ksi, OK
	STRUT STRESS	$S_{s2} = 0.63 \cdot \text{ksi}$	$S_{sA2} = 0.74 \cdot \text{ksi}$	$S2 = 18.13 \cdot \%$	LESS THAN 3.98ksi, OK
PANEL 3	COLUMN STRESS	$S_{c3} = 12.53 \cdot \text{ksi}$	$S_{cA3} = 12.93 \cdot \text{ksi}$	$C3 = 3.19 \cdot \%$	LESS THAN 23.15ksi, OK
	ROD STRESS	$S_{r3} = 6.94 \cdot \text{ksi}$	$S_{rA3} = 8.14 \cdot \text{ksi}$	$R3 = 17.24 \cdot \%$	LESS THAN 24ksi, OK
	STRUT STRESS	$S_{s3} = 0.51 \cdot \text{ksi}$	$S_{sA3} = 0.6 \cdot \text{ksi}$	$S3 = 17.24 \cdot \%$	LESS THAN 4.98ksi, OK
PANEL 4	COLUMN STRESS	$S_{c4} = 11.72 \cdot \text{ksi}$	$S_{cA4} = 12.2 \cdot \text{ksi}$	$C4 = 4.07 \cdot \%$	LESS THAN 23.08ksi, OK
	ROD STRESS	$S_{r4} = 6.58 \cdot \text{ksi}$	$S_{rA4} = 7.87 \cdot \text{ksi}$	$R4 = 19.63 \cdot \%$	LESS THAN 24ksi, OK
	STRUT STRESS	$S_{s4} = 0 \cdot \text{ksi}$	$S_{sA4} = 0 \cdot \text{ksi}$	$S4 = 0 \cdot \%$	NO STRUT

Check Existing Anchor Bolts @ Base :

Reactions @ Base:

Outer Diameter of Foundation

$$D_{f_outer} := 47.3542\text{ft}$$

Maximum Moment

$$M_{4_total} = 5930.88 \cdot \text{kip} \cdot \text{ft}$$

Maximum Shear

$$V_{4_total} = 60 \cdot \text{kip}$$

Maximum Compression

$$P_{4_total} = 921 \cdot \text{kip}$$

Maximum Uplift / Per Leg (Orig.)

$$P_{up} := \frac{4 \cdot M_{4_tank}}{N_{leg} \cdot D_{f_outer}} - \frac{P_{structure}}{N_{leg}}$$

$$P_{up} = 87.42 \cdot \text{kip}$$

Maximum Uplift / Per leg

$$P_{4_up} := \frac{4 \cdot M_{4_total}}{N_{leg} \cdot D_{f_outer}} - \frac{P_{4_total} - P_{water}}{N_{leg}}$$

$$P_{4_up} = 103.66 \cdot \text{kip}$$

Increase in Uplift

$$\%_{up} := 100 \cdot \frac{P_{4_up} - P_{up}}{P_{up}}$$

$$\%_{up} = 18.57$$

Number of Anchor Bolts:

$$N_{anch_bolts} := 8$$

Diameter of Anchor Bolts:

$$D_{anch_bolts} := D_{bolt}$$

Type of Anchor Bolts:

'A36'

$$F_u := 36\text{ksi}$$

threads per inch

$$n_{tr} := 7$$

Overload Coefficient

$$k_{wind} := 1.33$$

Allowable Tensile Stress - ANSI/AWWA D100-96 (Tab. 4)

$$F_{t1} := 15\text{ksi}$$

Allowable Tensile Stress - ASD 9TH Edition (Tab. I-B)

$$F_{t2} := 19.1\text{ksi}$$

Moment @ Top of Foundation:

$$M_{base} := M_{botA4}$$

$$M_{base} = 5930.88 \cdot \text{kip} \cdot \text{ft}$$

(Empty Water Tank)

Axial Load @ Top of Foundation:

$$W_{base} := W_{structure}$$

$$W_{base} = 83.45 \cdot \text{kip}$$

(Empty Water Tank)

Maximum Tension per Bolt:

$$T_{max} := \frac{P_{4_up}}{2}$$

$$T_{max} = 51.83 \cdot \text{kip}$$

Tensile Stress Area:

$$A_{T,S} := 0.7854 \cdot \left(D_{anch_bolts} - \frac{0.9743 \cdot \text{in}}{n_{tr}} \right)^2$$

$$A_{T,S} = 2.7195 \cdot \text{in}^2$$

Maximum Actual Tensile Stress:

$$f_t := \frac{T_{max}}{A_{T,S}}$$

$$f_t = 19.06 \cdot \text{ksi}$$

Maximum Allowable Tensile Stress:

$$F_t := 1.33 \cdot \min(F_{t1}, F_{t2})$$

$$F_t = 19.95 \cdot \text{ksi}$$

CHECK := if $f_t \geq 0\text{psi}$

"Allowable Tensile Stress > Maximum Actual Tensile Stress OK" if $F_t \geq f_t$

"Allowable Tensile Stress < Maximum Actual Tensile Stress make changes" otherwise

"All Anchor Bolts are in Compression for Empty Water Tank Case - Adequate for Additional Loads" otherwise

CHECK = "Allowable Tensile Stress > Maximum Actual Tensile Stress OK"

Existing Water Tank Foundation - Section 1 (Bottom Foundation):

$\gamma_c := 150\text{pcf}$

Total Weight of Concrete Foundation $V_{\text{conc_found}} := 3663360\text{in}^3$

$W_{\text{conc_found}} := \gamma_c \cdot V_{\text{conc_found}}$

$W_{\text{conc_found}} = 318 \cdot \text{kip}$

Height of Concrete Foundation

$H_{\text{conc_found}} := 7.0\text{ft}$

Outer Diameter of Foundation

$D_{f_outer} = 47.35\text{ft}$

Area of Each Foundation

$A_{\text{found}} := 100\text{ft}^2$

Section Modulus of Foundation

$S_{\text{found}} := 2393\text{ft}^3$

Rections @ Bottom of Foundation :

Axial Force w/o Antennas

$W_{1_Tank} := P_{4_tank} + W_{\text{conc_found}}$

$W_{1_Tank} = 1235.76 \cdot \text{kip}$

Bending Moment w/o Antennas

$M_{1_Tank} := M_{4_tank} + V_{4_tank} \cdot H_{\text{conc_found}}$

$M_{1_Tank} = 5489.15 \text{ft} \cdot \text{kip}$

Bearing Pressure w/o Antennas

$P_{\text{Tank}} := \frac{4 \cdot M_{1_Tank}}{N_{\text{leg}} \cdot D_{f_outer}} + \frac{W_{1_Tank}}{N_{\text{leg}}}$

$P_{\text{Tank}} = 424856.17 \cdot \text{lb}$

$BP := \frac{P_{\text{Tank}}}{A_{\text{found}}}$

$BP = 4248.56 \cdot \text{psf}$

Axial Force with Antennas

$W_{1_Total} := P_{4_total} + W_{\text{conc_found}}$

$W_{1_Total} = 1238.9 \cdot \text{kip}$

Bending Moment with Antennas

$M_{1_Total} := M_{4_total} + V_{4_total} \cdot H_{\text{conc_found}}$

$M_{1_Total} = 6354.2 \cdot \text{kip} \cdot \text{ft}$

Bearing Pressure with Antennas

$P_{\text{Total}} := \frac{4 \cdot M_{1_Total}}{N_{\text{leg}} \cdot D_{f_outer}} + \frac{W_{1_Total}}{N_{\text{leg}}}$

$P_{\text{Total}} = 443908.53 \cdot \text{lb}$

$BP_A := \frac{P_{\text{Total}}}{A_{\text{found}}}$

$BP_A = 4439.09 \cdot \text{psf}$

Increase in Bearing Pressure

$\Delta_{\text{pressure}} := \frac{BP_A - BP}{BP}$

$\Delta_{\text{pressure}} = 4.48\%$

***** By Engineering Judgement Existing Foundation is Adequate For New Antenna Loads *****

Check for Overturning Moment:**(No Water in Tank)** $\gamma_{soil} := 100\text{pcf}$ FOS_{OT} := 1.5

Overturning Moment

$$M_{OT} := M_{1_Total}$$

M_{OT} = 6354.2 ft·kip

Volume of Soil

$$V_{found} := 139\text{ft}^3$$

Total Weight of Backfilled Soil

$$W_{soil} := V_{found} \cdot \gamma_{soil} \cdot N_{leg}$$

W_{soil} = 55.6·kip

Resisting Moment

$$M_{RS} := 1.33 \cdot \left[\left(W_{structure} \right) \cdot 0.5 \cdot D_{f_outer} \dots \right. \\ \left. + 2 \cdot \left(\frac{W_{conc_found} + W_{soil}}{N_{leg}} \right) \cdot D_{f_outer} \right]$$

M_{RS} = 14392.88 ft·kip

Safety Ratio

$$R_{safe} := \frac{M_{RS}}{M_{OT}}$$

R_{safe} = 2.27

CHECK := "Actual Safety Ratio > Factor Of Safety OK" if R_{safe} ≥ FOS_{OT}
"Actual Safety Ratio < Factor Of Safety make changes" otherwise

CHECK = "Actual Safety Ratio > Factor Of Safety OK"

Check for Uplift: (No Water in Tank)FOS_{UP} := 1Check for Uplift - ANSI/AWWA D100-96 (Sec. 12.4.2)

Maximum Tension per Bolt

$$T_{max} = 51.83 \cdot \text{kip}$$

Number of Bolts

$$N_{anch_bolts} = 8$$

Weight of Concrete Foundation

$$W_{conc_found} = 318 \cdot \text{kip}$$

Maximum Counterweight per Bolt

$$W_{counter} := 1.33 \cdot \left[\frac{(W_{conc_found} + W_{soil})}{N_{anch_bolts}} \right]$$

$$W_{counter} = 62.11 \cdot \text{kip}$$

Safety Ratio

$$R_{safe_uplift} := \frac{W_{counter}}{T_{max}}$$

$$R_{safe_uplift} = 1.2$$

CHECK := "Actual Safety Ratio > Factor Of Safety OK" if R_{safe_uplift} ≥ FOS_{UP}
"Actual Safety Ratio < Factor Of Safety make changes" otherwise

CHECK = "Actual Safety Ratio > Factor Of Safety OK"

CONCLUSION:

THE MAX. INCREASE IN COLUMN STRESS IS ABOUT 4.07%, WHICH IS ACCEPTABLE BY ENGINEERING JUDGEMENT. THE MAX. ROD STRESS IS 8.26 KSI, WHICH IS LESS THAN THE ALLOWABLE OF 18 KSI x 1.33 (WIND ALLOWABLE) = 24 KSI. THE MAX. STRUT STRESS IS 0.74 KSI, WHICH IS LESS THAN THE ALLOWABLE 3.98 KSI.

PER PERIMETER PIER, THE UPLIFT WILL BE INCREASED BY 18.6%. THE UPLIFT OF 103 KIPS PER PERIMETER PIER IS LESS THAN THE UPLIFT RESISTANCE OF 124 KIPS. THEREFORE THE FOUNDATION OF THE TANK IS ADEQUATE FOR THE ADDITIONAL UPLIFT.

THE EXISTING (2) 2" DIA. ANCHOR BOLTS PER PIER, WILL SEE AN INCREASE IN STRESS OF 18.6% AND ARE STILL ADEQUATE.

Appendix 'A'
Existing Water Tank Design Drawings & Pictures

**Fullerton
Engineering
Consultants, Inc.**

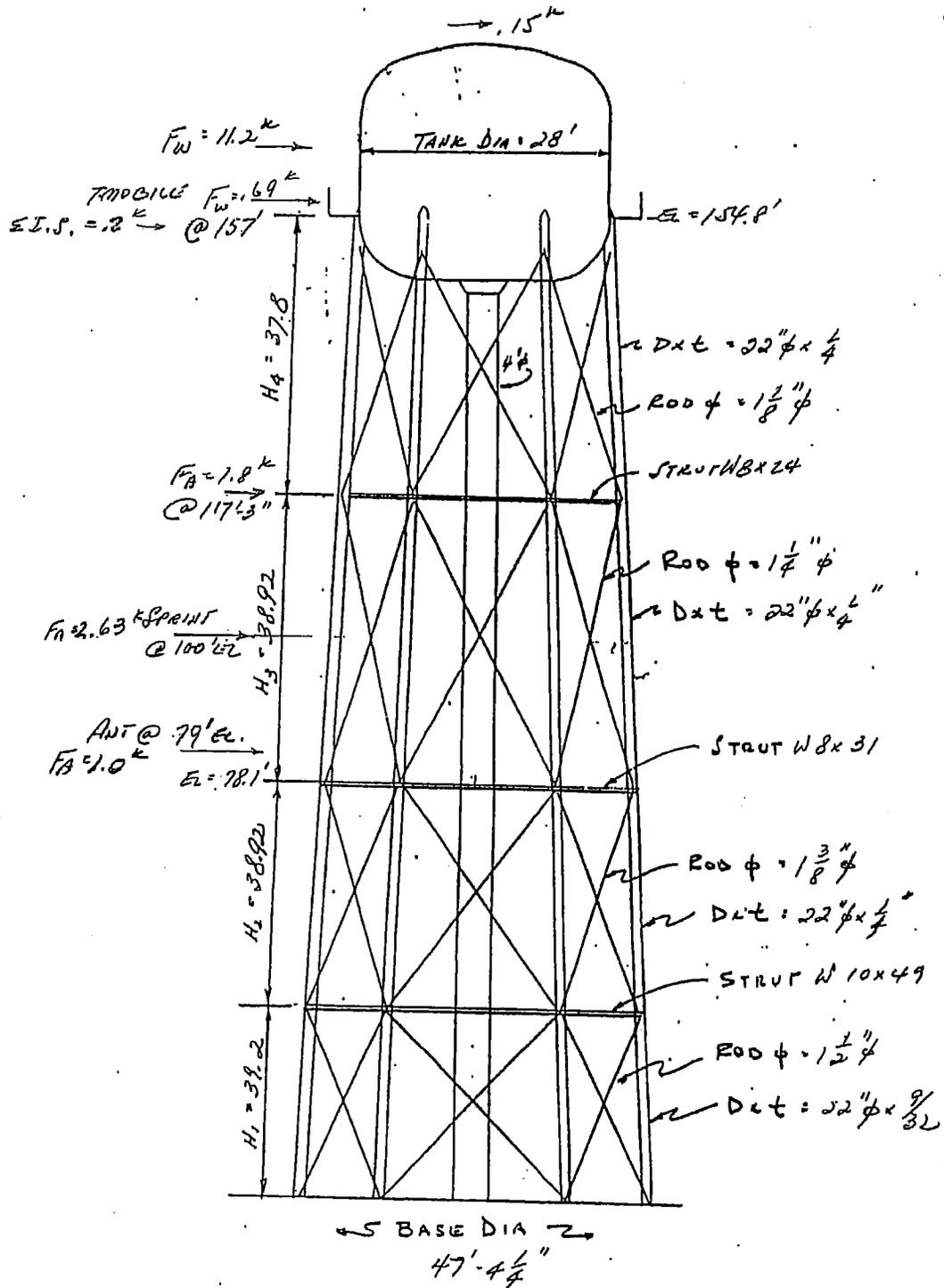
Site Name: Golf Center
Site No. IL-CHI5290
Computed by BMS
Checked by AJR

Date

06/01/2009

Page No.

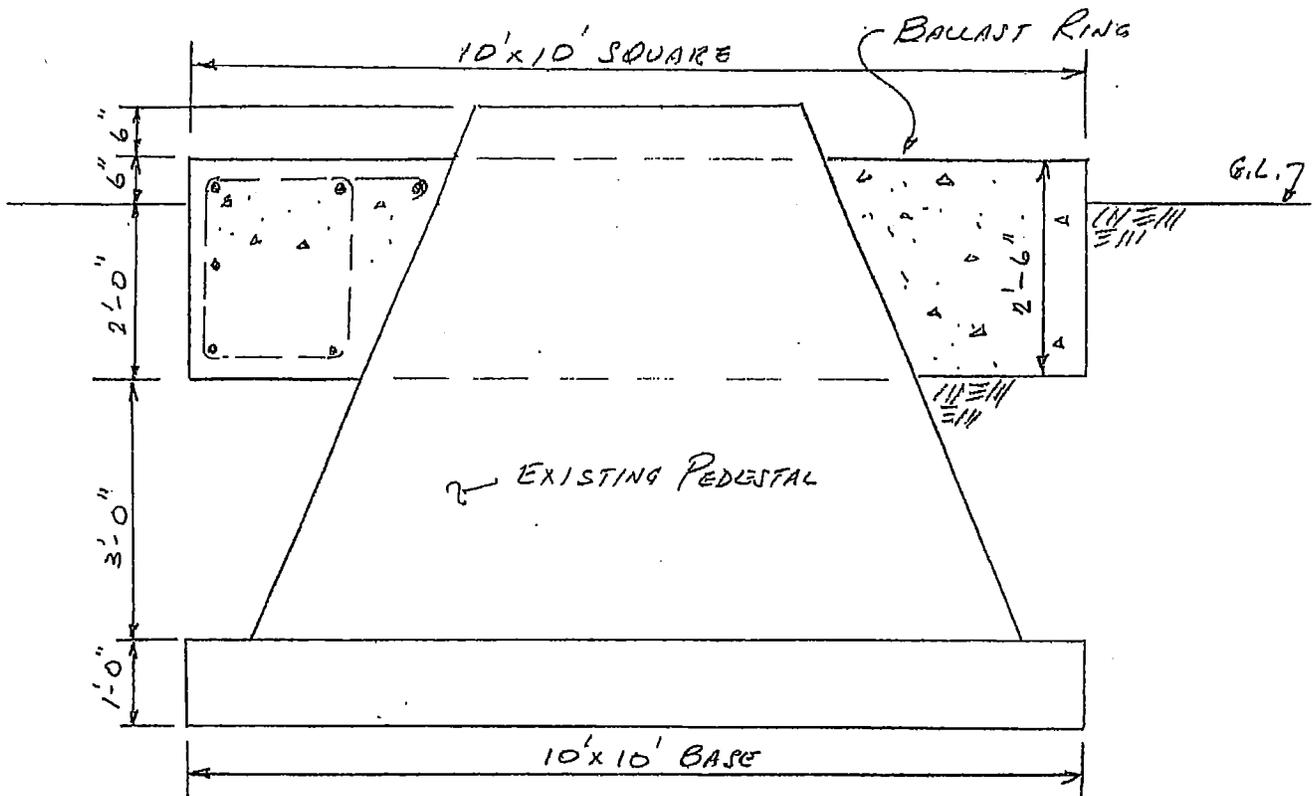
64 of 65



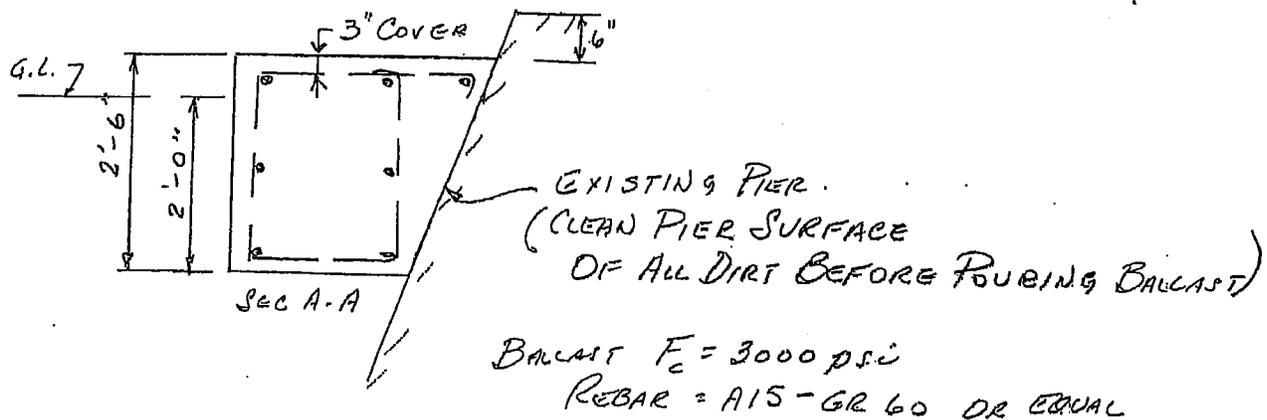
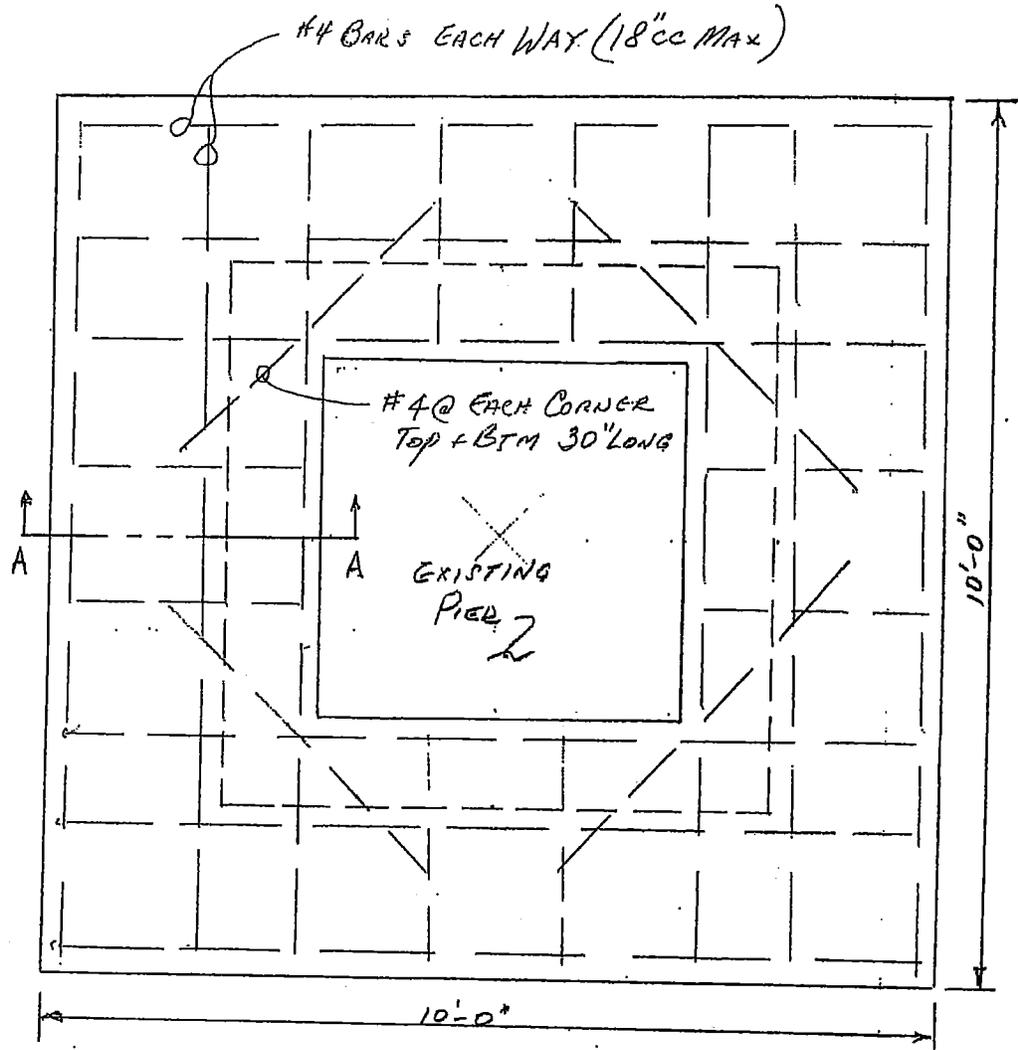
100MG ELLIPSOIDAL
COMPUTER MODEL

HOFFMAN EST

①



ELEVATION
 SEE SHEET (2) FOR PLAN VIEW



BALLAST PLAN VIEW

Appendix 'B'
Manufacturer Data Sheets

**Fullerton
Engineering
Consultants, Inc.**

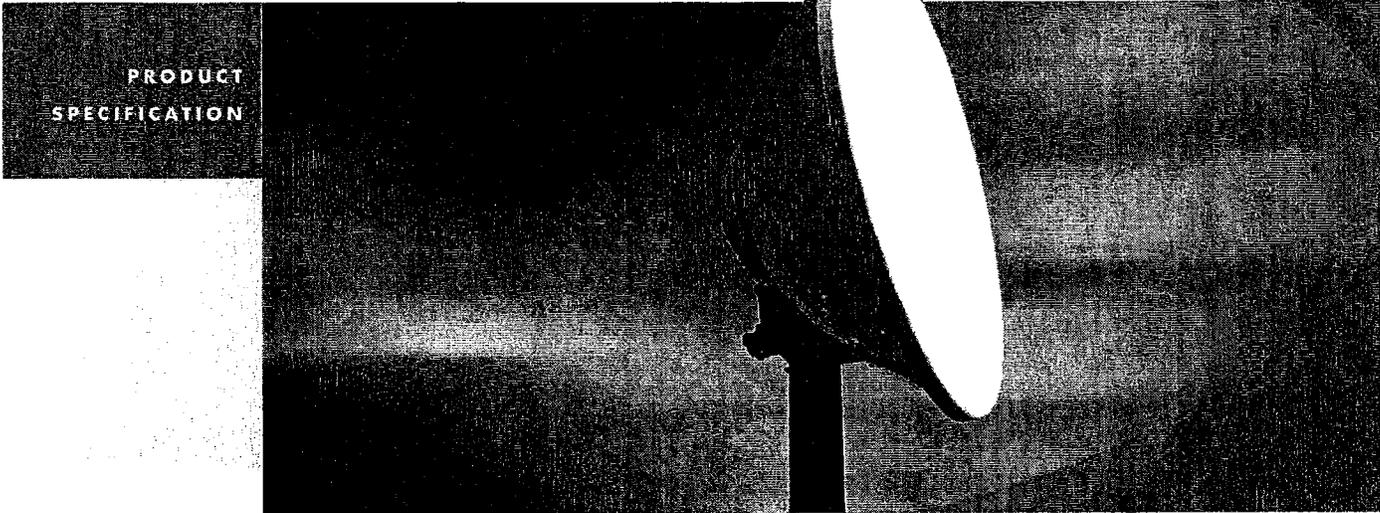
Site Name: Golf Center
Site No. IL-CHI5290
Computed by BMS
Checked by AJR

Date

06/01/2009

Page No.

65 of 65



PRODUCT
SPECIFICATION

ValuLine® III Next Generation Antennas VHLP2

SPECIFICATIONS

	VHLP2-7W	VHLP2-10W	VHLP2-11	VHLP2-13	VHLP2-15	VHLP2-18	VHLP2-23	VHLP2-26	VHLP2-28	VHLP2-32	VHLP2-38
Frequency Band, GHz	7.125–8.5	10.55–10.68	10.7–11.7	12.70–13.25	14.25–15.35	17.7–19.7	21.2–23.6	24.25–26.5	27.5–29.5	31.8–33.4	37.0–40.0
Bottom Band Gain, dBi	29.5	33.7	34.0	35.6	36.5	38.3	39.8	40.8	41.8	43.4	44.6
Mid Band Gain, dBi	30.7	33.8	34.4	35.8	36.8	38.7	40.4	41.2	42.2	43.7	45.2
Top Band Gain, dBi	31.9	34.3	35.0	36.0	37.2	39.1	41.0	41.8	42.7	44.0	45.8
Beamwidth, degrees	4.7	3.7	3.3	2.7	2.5	2.1	1.7	1.5	1.3	1.0	0.9
Front/Back, dB	57	56	60	62	65	67	66	68	68	61	66
XPD, dB	32	30	30	30	30	30	30	30	30	30	30
Return Loss, dB	17.7	17.7	17.7	17.7	17.7	17.7	17.7	17.7	17.7	17.7	17.7
Regulatory Compliance											
ETSI Class	R1 C3	R1 C2	R1 C3	R1 C3	R2 C3	R2 C3	R3 C3	R4 C3	R4 C3	R5 C3B	R5 C3B
FCC Part 101	N/A	CAT A*	CAT B	N/A	N/A	CAT A	CAT A	CAT A	N/A	N/A	CAT A
Brazil Anatel	N/A	C2	C2	C2	C2	C2	C2	C2	C2	C2	C2
Canada SRSP	N/A	310.5	N/A	312.7B	314.5A	Note 1	Note 2	N/A	N/A	N/A	338.6A
Andrew RPE Number	7075A	7085B, 7086B*	7083A	7004	7008	7012A	7016A	7020A	7024A	7028	7032A

Note 1: Meets Canada SRSP 317.7A, 318.5, 318.8

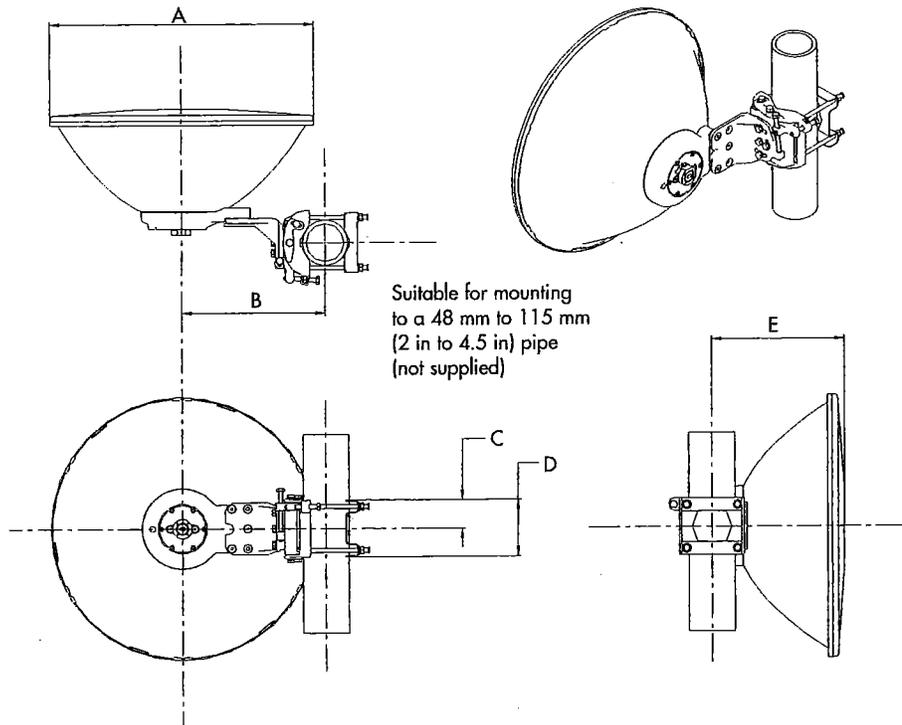
Note 2: Meets Canada SRSP 312.2A, 321.8B

* Use for FCC band (10.5–10.7 GHz)

One Company. A World of Solutions.

SPECIFICATIONS

ValuLine® III Next Generation Antennas—VHLP2



Antenna Dimensions, mm (in)

A	663 (26.1)
B	358 (14.1)
C	72 (2.8)
D	143 (5.6)
E	335 (13.2)

Antenna Fine Adjustment

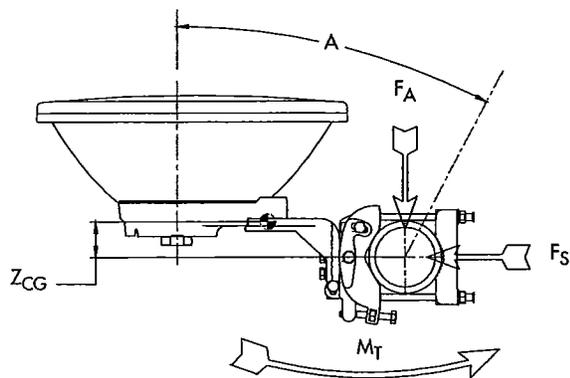
Fine Aximuth	±10°
Fine Elevation	±25°

SPECIFICATIONS

ValuLine® III Next Generation Antennas—VHLP2

Wind Loading

The axial, side, and twisting moment forces stated below are the maximum loads applied to the tower by the antenna at a survival windspeed of 250 km/h (155 mph). They are the result of wind from the most critical direction for each parameter. The individual maximums may not occur simultaneously. All forces are referenced to the antenna mounting pipe.



Axial force	F_A	1066 N (240 lb)
Side force	F_S	496 N (111 lb)
Moment	M_T	382 N-m (282 lb-ft)
Angle A for M_T maximum		0°
Z_{CG} * without ice, mm (in)		125 (4.9)
X_{CG} with 12 mm (1/2 in) radial ice, mm (in)		188 (7.4)

* Z_{CG} is the axial distance from the center of gravity to the mounting pipe.

Antenna Weights Including Mount

Antenna without ice, kg (lb)	12.28 (27)
Antenna with 12 mm (1/2 in) radial ice, kg (lb)	24.7 (54)

Antenna Packed Weights (Gross)

Weight, kg (lb)	19.42 (43)
-----------------	------------

Packed Antenna Dimensions (Single Unit Pack)

Dimensions, cm (in)	70 x 70 x 56 (27.5 x 27.5 x 22.0)
---------------------	-----------------------------------



One Company. A World of Solutions.

Andrew Corporation
3 Westbrook Corporate Center
Suite 900
Westchester, IL 60154 US

Customer Support Center
From North America
Telephone: 1-800-255-1479
Fax: 1-800-349-5444

International
Telephone: +1-708-873-2307
Fax: +1-779-435-8579

Internet: www.andrew.com

All designs, specifications, and availabilities of products and services presented in this bulletin are subject to change without notice.

Bulletin PA-101028.4-EN (9/07)

© 2007 Andrew Corporation, Westchester, IL 60154 US

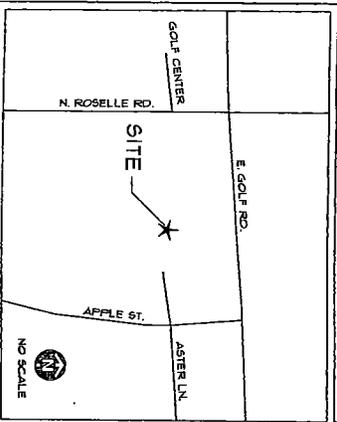
SHEET INDEX

NO.	DESCRIPTION
T-1	TITLE SHEET
C-1	SITE PLAN
C-2	ENLARGED SITE PLAN
C-3	SHELTER/ANTENNA PLAN
C-4	SITE ELEVATION & DETAILS
E-1	SINGLE LINE NOTES

DRIVING DIRECTIONS

DEPART FROM CLEAR LAUREL CENTER, 2620 N. RIVER RD., ROSEMONT, IL 60069. TRAVEL WEST ON N. RIVER RD. TURN LEFT AT N. RIVER RD. TURN RIGHT ONTO 1-50 W/ PARTIAL TOLL ROAD. TAKE THE ROSSELLE RD EXIT (TOLL ROAD) KEEP LEFT AT THE FORK. FOLLOW SIGNS FOR CLEAR LAUREL CENTER. TAKE PARTIAL TOLL ROAD TURN LEFT AT N. ROSSELLE RD. TURN RIGHT ON ASTER LN. AT APPLE ST. TURN LEFT AT ASTER LANE.

VICINITY MAP



clear wireless LLC

a Nevada limited liability company, a Sprint affiliate

SITE NAME
GOLF CENTER

SITE NUMBER
IL-CHI5290

SITE ADDRESS
95 ASTER LANE
HOFFMAN ESTATES, IL 60169

PROJECT TYPE
BACKHAUL PROJECT

PROJECT TEAM

clear wireless LLC
a Nevada limited liability company,
a Sprint affiliate
5600 N. RIVER RD.
SUITE 300
ROSEMONT, IL 60069
TEL: (312) 318-3000
APPLICANT

EVEREST GROUP
THE EVEREST GROUP, LLC
7003 W. 117TH ST.
MORTON, IL 60452
TEL: (708) 932-1863
REAL ESTATE

ENGINEER/STRUCTURAL
Fullerton Engineering Consultants
3100 W. HICKINS RD.
SUITE 600
ROSEMONT, ILLINOIS 60069
TEL: (312) 318-3000
FAX: (312) 318-3005

- HANDICAP ACCESS REQUIREMENTS ARE NOT REQUIRED
- FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION
- FACILITY HAS NO PUMPING OR REFRIGERANTS
- THIS FACILITY SHALL NOT EXCEED ALL FAA AND FCC REGULATORY REQUIREMENTS
- ALL WORK MUST CONFORM TO CLEARWIRE "CLEARWIRE CONSTRUCTION INSTALLATION GUIDE - UPTAX"

- NEW CLEARWIRE BACKHAUL DISH ANTENNAS INSTALLED ON EXISTING WATER TOWER
- NEW CLEARWIRE BACKHAUL EQUIPMENT INSTALLED IN EXISTING SHED

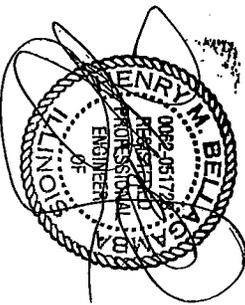
PROJECT SUMMARY

SITE NAME:	GOLF CENTER
SITE NO.:	IL-CHI5290
SPRINT HOST NO.:	IL349
SITE ADDRESS:	95 ASTER LANE HOFFMAN ESTATES, IL 60169
COUNTY:	COOK
LANDLORD ADDRESS:	VILLAGE OF HOFFMAN ESTATES 5000 ROSSELLE ROAD HOFFMAN ESTATES, IL 60169 (647) 851-5100
SITE COORDINATES (NAD 83):	Easting: 1742078.7 Northing: 1742078.7 Zone: 18NAD 93
ASSOCIATION:	INTERNATIONAL BUILDING CODE
BUILDING CODE:	2003 EDITION
ELECTRICAL CODE:	NATIONAL ELECTRIC CODE 2005 EDITION

ENGINEER'S LICENSE

I CERTIFY THAT THESE DRAWINGS WERE PREPARED BY THE ENGINEER UNDER MY DIRECT SUPERVISION AND CONTROL, AND TO THE BEST OF MY KNOWLEDGE AND BELIEF COMPLY WITH THE REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE 2003 EDITION.

LICENSED ENGINEER - STATE OF ILLINOIS



APPROVALS

APPROVED BY: *[Signature]* DATE: 11/30/09

SIGNED: *[Signature]* DATE: 11/30/09

EXPENSE	DATE	DATE
CLEARWIRE CONST.		
CLEARWIRE RF		
CLEARWIRE OPS		
LANDLORD		

DRAWING SCALED TO 11"X17"

clear wireless LLC
a Nevada limited liability company,
a Sprint affiliate
5600 N. RIVER RD.
SUITE 300
ROSEMONT, IL 60069
(312) 318-3000

ENGINEER/STRUCTURAL
Fullerton Engineering Consultants
3100 W. HICKINS RD.
SUITE 600
ROSEMONT, ILLINOIS 60069
TEL: (312) 318-3000
FAX: (312) 318-3005
DESIGN FILE NO. IL-4-002148

REVISION	DATE	BY	CHK
APPROVED BY:			
DATE:			
BY:			
DATE:			
BY:			
DATE:			

SHEET NAME	SHEET NO.
GOLF CENTER	IL-CHI5290
SITE ADDRESS	95 ASTER LANE HOFFMAN ESTATES, IL 60169

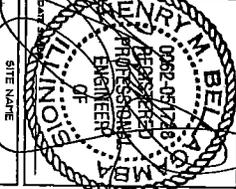
SHEET NAME	SHEET NO.
TITLE SHEET	T-1

THIS DRAWING IS THE PROPERTY OF FULLERTON ENGINEERING CONSULTANTS, INC. IT IS FOR THE EXCLUSIVE USE OF THIS PROJECT. ANY RE-USE OF THIS DRAWING WITHOUT THE EXPRESSED WRITTEN CONSENT OF FULLERTON ENGINEERING CONSULTANTS, INC. IS PROHIBITED.

clear wireless LLC
 a Nevada limited liability company,
 a Sprint affiliate
 5600 N. RIVER RD.
 SUITE 300
 ROSEMONT, IL 60018
 (630) 516-5000

Fullerton Engineering Consultants
 5100 W. HICKING RD.
 ROSEMONT, ILLINOIS 60018
 TEL: 631-731-0200
 FAX: 631-731-0205
 DESIGN FIRST NO. 194-0071439

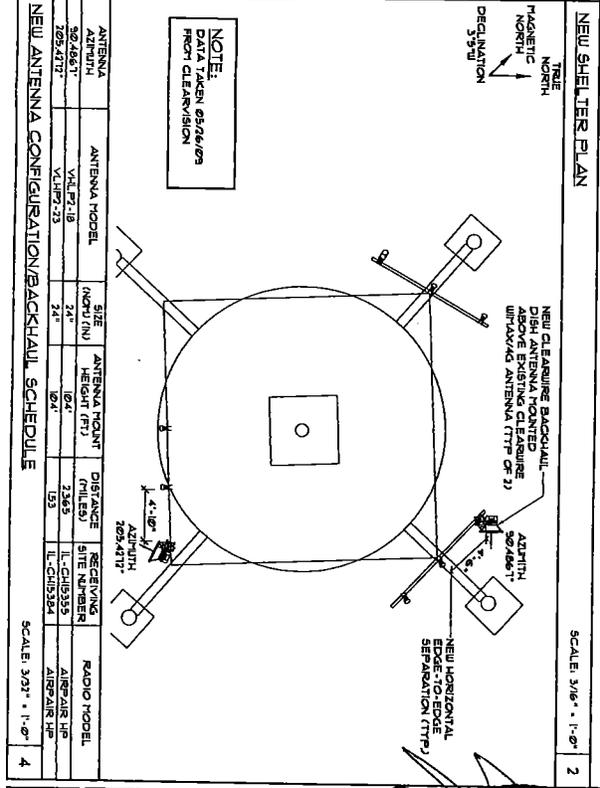
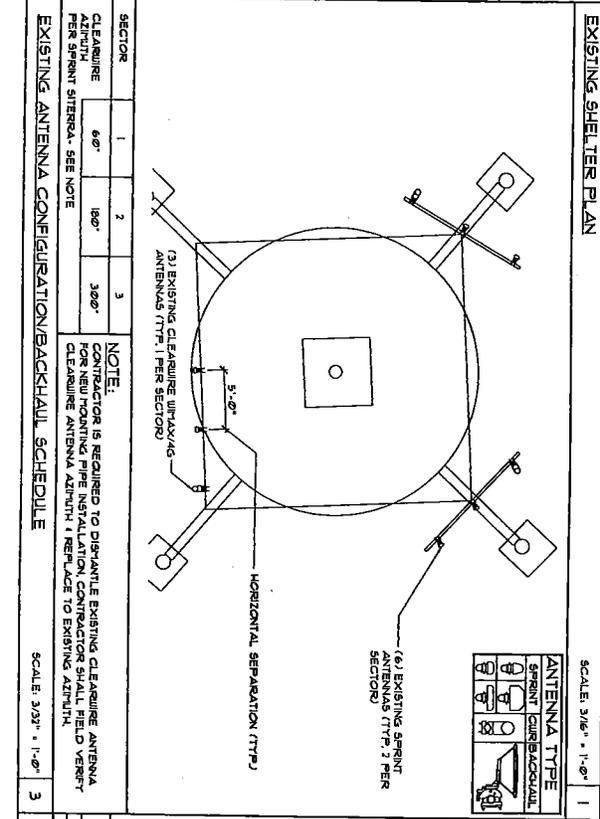
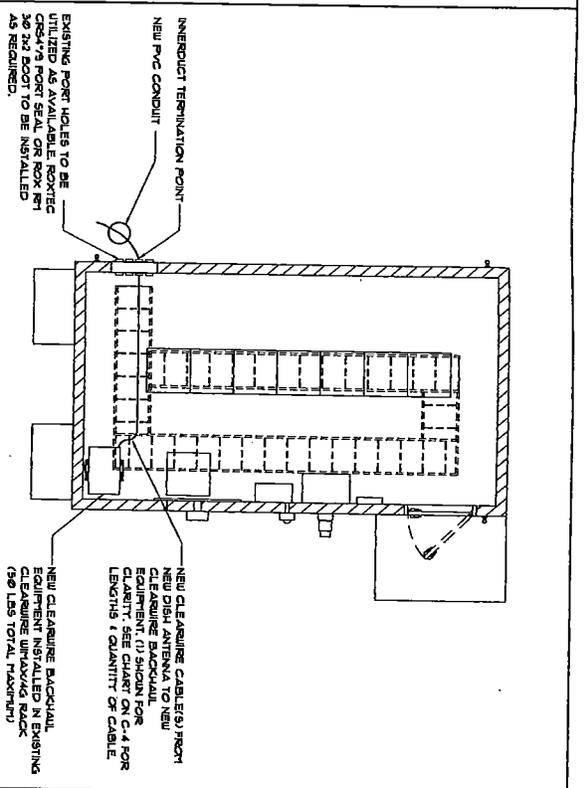
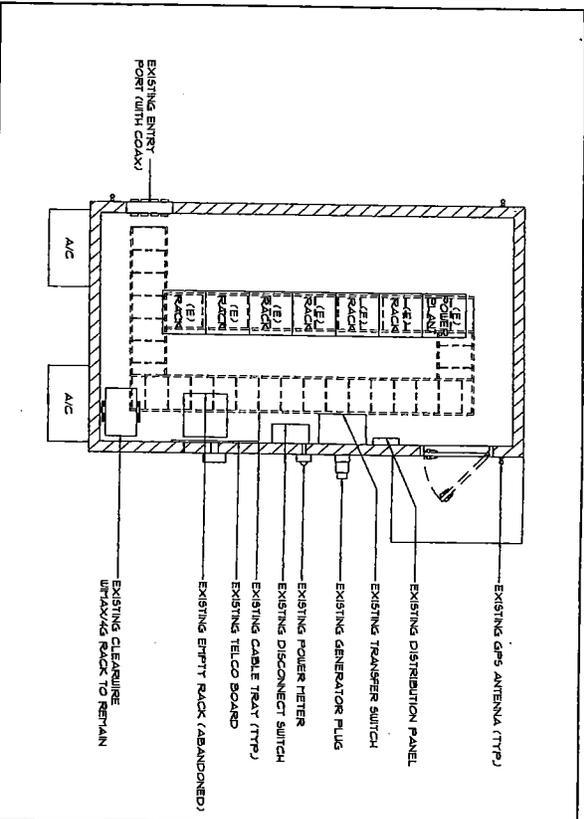
CHECKED BY:	JP
APPROVED BY:	TBS
DATE:	03/14/09
DESCRIPTION:	300 REVIEW
DATE:	03/14/09
DESCRIPTION:	FINAL
DATE:	03/14/09
DESCRIPTION:	501



GOLF CENTER
 SITE NO.
 IL-CH15290
 SITE ADDRESS
 25 ASTER LANE
 MORNAN ESTATES, IL 60169

SHEET NAME
**SHELTER/
 ANTENNA
 PLAN**

SHEET NUMBER
C-3



ANTENNA AZIMUTH	30/46/61	ANTENNA MODEL	VALENTI 23	SIZE (INCH)	24"	ANTENNA POINT (TYP)	0/21	DISTANCE (FT)	2385	SITE NUMBER	IL-CH15290	RADIO MODEL	ALPSTAR 100
ANTENNA AZIMUTH	30/46/61	ANTENNA MODEL	VALENTI 23	SIZE (INCH)	24"	ANTENNA POINT (TYP)	0/21	DISTANCE (FT)	2385	SITE NUMBER	IL-CH15290	RADIO MODEL	ALPSTAR 100

THIS DRAWING IS THE PROPERTY OF FULLERTON ENGINEERING CONSULTANTS, INC. IT IS FOR THE EXCLUSIVE USE OF THIS PROJECT. ANY RE-USE OF THIS DRAWING WITHOUT THE EXPRESSED WRITTEN CONSENT OF FULLERTON ENGINEERING CONSULTANTS, INC. IS PROHIBITED.

VILLAGE OF HOFFMAN ESTATES
ZONING BOARD OF APPEALS

FINDING OF FACT

DATE OF PUBLIC HEARINGS: November 17 and December 8, 2009

DATE OF PRESENTATION TO VILLAGE BOARD: December 14, 2009

PETITION: Hearing held at the request of the Village of Hoffman Estates (Lessor) and The Everest Group/Sprint, Clearwire (Lessee) to consider a special use under the Zoning Code to permit the installation of communication antennas and accompanying equipment on the property located at 3990 Huntington Boulevard.

DISTRICT IN WHICH PROPERTY IS LOCATED: R-7, One Family Residential District

ZONING CODE SECTION(S) FOR SPECIAL USE: 9-3-9-A, 9-5-7-C-4

FINDING-OF-FACT: The ZBA found that the Standards for a Special Use (Section 9-1-18) were met.

MOTION: Request to grant the Village of Hoffman Estates (Lessor) and The Everest Group/Sprint, Clearwire (Lessee), *a special use under Sections 9-3-9-A and 9-5-7-C-4 to permit the installation of two (2) microwave dishes and associated equipment to be no greater than one hundred and thirty four (134) feet high on a Village water tank at 3990 Huntington Boulevard.* The following conditions shall apply:

1. This special use shall be subject to approval of the final lease agreement with the Village of Hoffman Estates.
2. No advertising shall be allowed on the equipment or structures.
3. The petitioner shall pay all costs associated with the third party review and inspections, as required by the Village's Public Works Department policy.
4. Should the operation of these microwave dishes cease for a period of six (6) months, the dishes and associated equipment shall be removed per Zoning Code Section 9-1-18-L.

The petitioner was agreeable to the above listed conditions.

RECOMMENDATION: The Zoning Board of Appeals (ZBA) recommends approval of this request.

The petitioner, Mr. Faber representing Sprint-Clearwire, requested a special use to install two microwave dish antennas on the water tank. The dishes operate under a different radio band from the existing cellular antennas and are used to carry the increased volumes of data (video and internet) common in modern cell phones. The Zoning Board noted that the prior request at this location, made by Cricket Communications, was denied. The Chairman noted the dissimilarities of this request from the Cricket request in that no new shelter is being constructed

and the dishes are a relatively small installation. The Zoning Board confirmed that the structural report was completed and was accepted by the Village. The Zoning Board confirmed that each dish weighs approximately 45 pounds including the mounting equipment. The Zoning Board recommended approval of the request.

AUDIENCE COMMENTS

None.

VOTE:

7 Ayes

0 Nays

0 Absent

ZONING BOARD OF APPEALS

Chairman William Weaver

Vice-Chairman Ronald Jehlik

Denise Wilson

Michael Ciffone

Masoom Ali

Donna Boomgarden

Michael Gaeta

*** IMMEDIATE AUTHORIZATION TO APPLY FOR PERMITS IS REQUESTED ***

**THIS SPECIAL USE WILL EXPIRE UNLESS ACTED UPON WITHIN ONE (1) YEAR
OF VILLAGE BOARD APPROVAL**

FINDING OF FACT WRITTEN BY DEVELOPMENT SERVICES STAFF

VILLAGE OF HOFFMAN ESTATES

Memo

TO: William Weaver, Zoning Board of Appeals Chairman
FROM: Josh Edwards, Assistant Planner *JAE*
RE: **SPRINT-CLEARWIRE COMMUNICATIONS - 3990 HUNTINGTON BOULEVARD - SPECIAL USE - COMMUNICATION ANTENNAS AND ASSOCIATED EQUIPMENT**
DATE: December 3, 2009
HEARING DATE: December 8, 2009

1. REQUEST SUMMARY

Request by Village of Hoffman Estates (owner) and Sprint-Clearwire Communications (lessee) for a special use to permit the installation of two microwave dishes and associated equipment on a Village owned water tank at 3990 Huntington Boulevard.

2. BACKGROUND

The existing water tank at 3990 Huntington Boulevard currently contains communication antennas installed by Sprint, Nextel, and T-Mobile. One set of existing antennas is mounted on the neck of the structure at a height of 83 feet and the other sets are mounted on top of the tank at 133 feet and 143'6". The equipment for the existing antennas is located near the base of the tank in a shelter to the southwest (Nextel/Sprint), in a shelter to the north (T-Mobile), and on a concrete pad to the north (Sprint).

3. PETITIONER PROPOSAL

The petitioner is proposing to install two, 30 inch high microwave dishes at 134 feet above grade on top of the bulb of the water tank. The installation requires a special use approval. The two dishes would be attached to the existing array of Sprint cellular antennas located at 133 feet high, which includes 9 antennas.

The function of the new dishes is to support and upgrade the Sprint network with voice and data transmission as an alternative to installing miles of underground fiber optic wires throughout the coverage area.

No separate building or equipment is proposed on the ground. The new equipment for the dishes would be installed entirely within the existing brick equipment shelter located southwest of the water tank. Wiring would be installed underground between the shelter and water tank; and within the water tank to the antenna array.

4. SITE CONDITIONS

The subject property is zoned R-7, One Family Residential District. The surrounding properties are zoned R-7, One Family Residential District.

5. APPLICABLE REQUIREMENTS

- a) Section 9-5-7-C-4 (p. CD9:78) states that public utility and public service uses shall be permitted in the R-7 District subject to the issuance of a special use permit in accordance with the provisions of Section 9-1-18.
- b) Section 9-3-9-A (p. CD9:58) states that the total antenna height of any communications tower, antenna or combination thereof of any height over 45 feet upon Village owned or leased facilities shall be permitted only as a special use.

6. ADJACENT OWNER COMMENTS

Standard notification letters have been mailed and as of this writing no comments have been received.

7. RELEVANT SPECIAL USE HISTORY

Subject Property

- a) DENIED – September 2008 – The Village board denied a request by Cricket Communications to install 3 cellular antennas and associated equipment on the ground.
- b) Ordinance No. 3871-2006 was granted to allow T-Mobile to erect 12 antennas at a height of 143 feet 6 inches.
- c) Ordinance No. 2979-1998 was granted to allow Sprint PCS to erect 9 antennas at a height of 83 feet.

- d) Ordinance No. 2930-1997 was granted to allow OneComm Corporation (Nextel) to erect 9 antennas at a height of 133 feet. (Nextel is now a part of Sprint.)
- e) Ordinance No. 1361-1982 was granted to allow the Village of Hoffman Estates Police and Fire Departments to erect receiving antennas.

Similar Properties

Over the past decade various communication antennas have been installed on all 6 water tanks throughout the Village.

8. PUBLIC WORKS COMMENTS

A thorough structural analysis and engineering review is required prior to the zoning review for Village owned water tanks. A structural analysis prepared by a licensed engineer on behalf of the petitioner has been reviewed by Public Works and the Village's third party engineering consultants. The structural analysis concluded that the installation will have no adverse impact on the water tank. Public Works has accepted the report.

9. IMMEDIATE AUTHORIZATION TO APPLY FOR PERMIT(S)

The petitioner has requested an Immediate Authorization to Apply for Permit(s).

10. DEVELOPMENT SERVICES COMMENTS

The most recent proposal on this property made by Cricket Communications was denied. This proposal is different from the Cricket proposal in that no new shelter or other equipment is proposed on the ground within view of surrounding residences. The only visible additions will be located above the bulb of the water tank.

The proposal to install dishes onto a water tank rather than erecting a separate antenna tower will minimize visual clutter. The proposed dishes are designed to upgrade the existing antennas. The height and shape of the water tank and the relatively small size of the two dishes will reduce the visibility of the dishes from surrounding properties. The dish is smaller than standard cellular antenna arrays and is comparable in size to a residential television satellite dish and no more aesthetically obtrusive.

The existing special use approval and lease for the existing 9 antennas will stay in effect. A new lease will be required for this new installation.

The Federal Telecommunications Act of 1996 expressly preempts local governments from regulating the placement, construction, or modification of personal wireless services on the basis of environmental or health concerns. Such concerns cannot be discussed or used as a basis of the decision whether to grant a special use.

11. MOTION

Should the Zoning Board find that the Standards for a Special Use are met, the following motion is provided with conditions:

A special use under Sections 9-3-9-A and 9-5-7-C-4 to permit the installation of two (2) microwave dishes and associated equipment to be no greater than one hundred and thirty four (134) feet high on a Village water tank at 3990 Huntington Boulevard.

The following conditions shall apply:

- 1. This special use shall be subject to approval of the final lease agreement with the Village of Hoffman Estates.*
- 2. No advertising shall be allowed on the equipment or structures.*
- 3. The petitioner shall pay all costs associated with the third party review and inspections, as required by the Village's Public Works Department policy.*
- 4. Should the operation of these microwave dishes cease for a period of six (6) months, the dishes and associated equipment shall be removed per Zoning Code Section 9-1-18-L.*

cc: Corporation Counsel, D. O'Malley, D. Plass, R. Norton, Petitioner

VILLAGE OF HOFFMAN ESTATES
ZONING BOARD OF APPEALS

REQUEST FOR SPECIAL USE HEARING

FOR VILLAGE USE ONLY

Hearing Fee \$ 400.00 Date Paid 7/9/09 Received By J. EDWARDS

Hearing Date: 12/8/09 Time: 7:30 pm Legal Published

Receipt Number 293400 Check No. 104885 Zoning District R-7

PLEASE PRINT OR TYPE

Village of Hoffman Estates

1. Name of Property Owner(s)* _____

E-Mail Address joshua.edwards@hoffmanestates.org Fax 847-490-6868

1900 Hassell Road Phone 847-882-9100

Owner's Address _____ Phone _____

Hoffman Estates City _____ State IL Zip 60169

City _____ State _____ Zip _____

Subject Property's Address (if different than #1): 3990
3390 Huntington Blvd

2. Person applying if other than owner:*

Name Will Faber Company The Everest Group/ Sprint, Clearwire

E-Mail Address wfaber85@gmail.com Fax 773/409-5426

Address 7013 W. 111th St. Phone 773-987-5299

City Worth State IL Zip 60482

City _____ State _____ Zip _____
03-92-313-244-532

3. Property Index Number (PIN) _____

5. Please describe the proposed use, or attach a letter.

Addition of 2 dish antennas to existing telecommunications facility

* If "owner" is an entity other than an individual(s), then an Economic Disclosure Statement must also be filed.

6. **An Immediate Authorization to Apply for Permits** allows the Code Enforcement Division to begin the building permit review process prior to the adoption of the Ordinance by the Village Board. The Immediate Authorization makes it possible for you to expedite the plan review process by approximately two weeks. Contact the Code Enforcement Division at 847/781-2631 to discuss the building permit application and review process.

Please check one of the following

Yes, I request Immediate Authorization to Apply for Permits upon approval of my application by the Village Board, allowing me to begin the building permit review process prior to adoption of the Ordinance approving my special use.

Or

No, I do not request Immediate Authorization to Apply for Permits.

7. I, the undersigned, certify the information and submissions provided accurately represent the current conditions and proposed improvement(s) requiring a special use.

Owner's Signature ~~Michael Healey~~

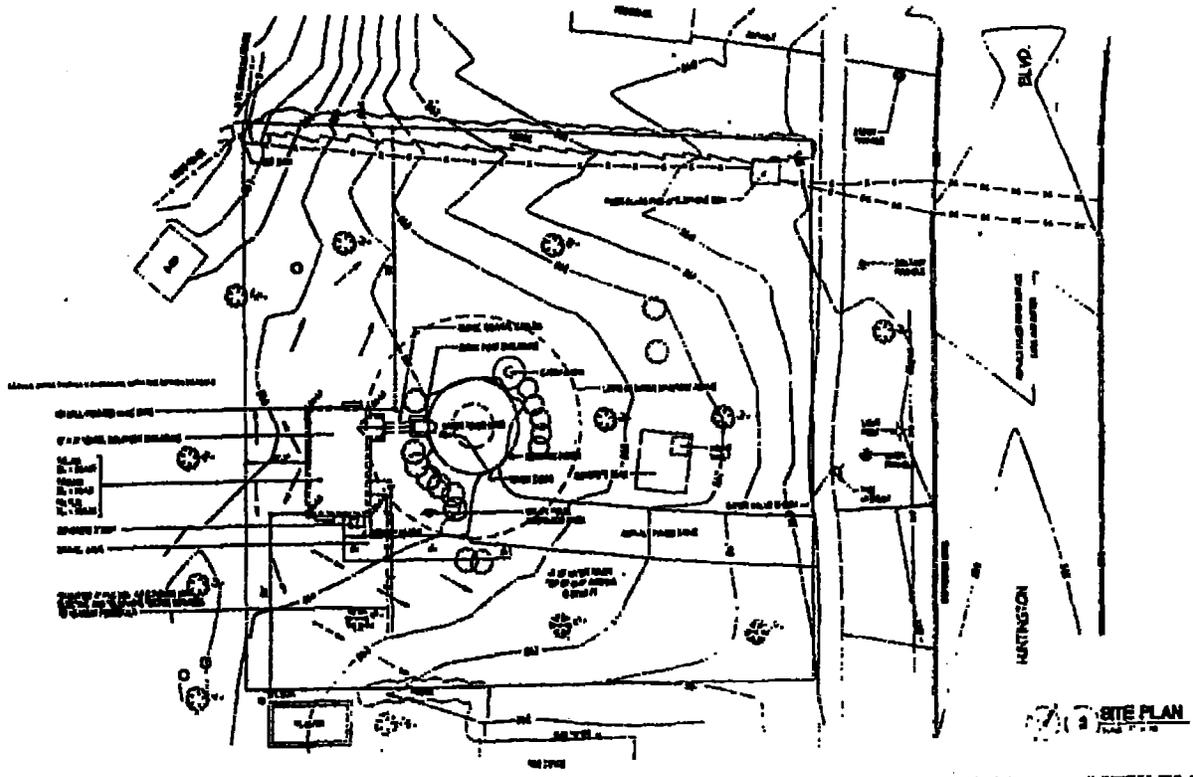
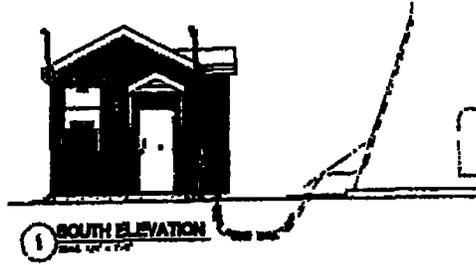
Name (Please Print) Michael Healey

Applicant's Signature William Fale

Name (Please Print) William Fale

All requests for a hearing must be accompanied by the items required according to the nature of the request. All fees must be paid before Zoning Board can hear any case. Any additional fees must be paid before any findings or reports are given to the Village Board.

The Premises are described and/or depicted as follows:



INITIALS
MSK
JES

Notes:



THE EVEREST GROUP
Representing Sprint/ Clearwire

7013 W 111th St
Worth, IL 60482

773-987-5299

wfaber85@gmail.com

July 8, 2009
Village of Hoffman Estates
1900 Hassell Road
Hoffman Estates, IL 60169

This application for a special use permit on behalf of Sprint/ Clearwire is for the purpose of installing two additional antennas and two lines of associated coaxial cable on the water tank at 3990 Huntington Blvd. The purpose of the communications system upgrade is to facilitate wireless backhaul coverage over the existing Sprint telecommunications network. Effectively, the current T1 system will be replaced with wireless connections between individual communication sites and the central switch. The new antenna will make this linkage possible, thereby lowering costs to consumers and increasing the reliability and robustness of the existing and future data network.

Sincerely yours,

Will Faber

The Everest Group

DEPARTMENT OF PUBLIC WORKS

Memo

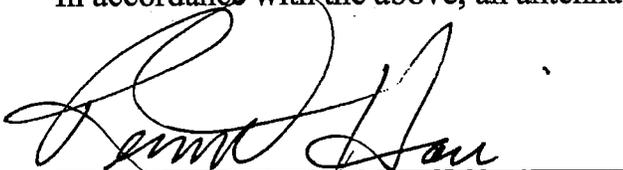
TO: Don Plass, Director of Code Enforcement
FROM: Kenneth Hari, Director of Public Works
RE: CLEARWIRE ANTENNA INSTALLATIONS
DATE: October 16, 2009

Urgent For Review Please Comment Please Reply Enclosure(s)

I have reviewed the proposed projects, including the upgrade load evaluations report issued by Robert Wozniak and the plan review completed by SEH, and concluded that the installations will have no adverse impact on the respective tower located at Huntington Blvd.. Accordingly, a construction permit may be issued for the Huntington Blvd. facility subject to the following:

- *Before construction may begin, the installation contractor must attend a pre-construction meeting with Public Works Staff.*
- *During construction, the tower site at ground level must be clean at all times with no equipment or construction materials stored on site at any time.*

In accordance with the above, an antenna installation permit(s) may be released.


Kenneth Hari, Director of Public Works

Dir/Corr09/ClearwireAntennaInstall 101309

cc: w/o Attachments
Joseph Nebel, Superintendent of Operations
Haileng Xiao, Superintendent of Water & Sewer
Josh Edwards, Planner



HOFFMAN ESTATES

GROWING TO GREATNESS

November 5, 2009

To All Interested Parties:

Please be advised the Zoning Board of Appeals of the Village of Hoffman Estates will conduct a public hearing at the request of the Village of Hoffman Estates (Lessor) and The Everest Group/Sprint, Clearwire (Lessee) to consider a special use under the Zoning Code to permit the installation of two 30 inch dish antennas (data) on the existing water tower at ± 134 feet in height and accompanying equipment installed within an existing brick shelter on the property located at 3990 Huntington Boulevard.

The hearing will be held in the Municipal Building, 1900 Hassell Road, Hoffman Estates, Illinois, **Tuesday, November 17, 2009 at 7:30 p.m.**

A map designating the subject site is included on the back of this letter. Appropriate time will be available for questions/comments from interested parties. Should you wish any additional information, please feel free to contact the Planning Division at 847/781-2660. Material is available at the office for review.

Should it become necessary to continue this hearing to an additional date, it will be announced at the conclusion of testimony at the hearings. No further notification of this review will be sent.

Sincerely,

Josh Edwards, Assistant Planner
Department of Development Services

JE/pm

1900 Hassell Road
Hoffman Estates, Illinois 60169
www.hoffmanestates.org

Phone: 847-882-9100
Fax: 847-843-4822

William D. McLeod
MAYOR

Raymond M. Kincaid
TRUSTEE

Gary J. Pilafas
TRUSTEE

Karen V. Mills
TRUSTEE

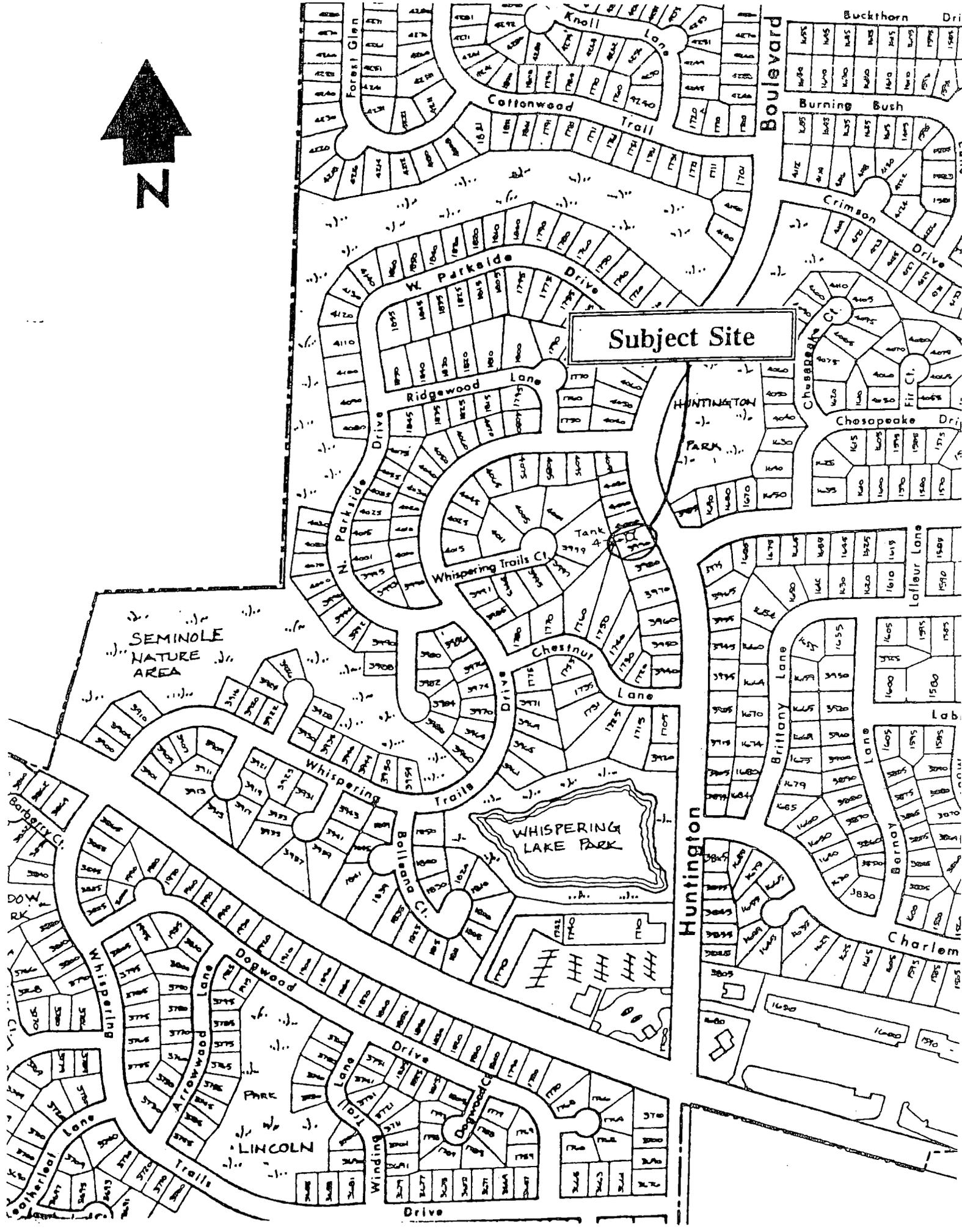
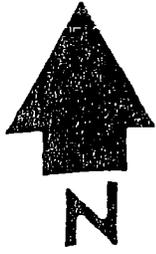
Jacquelyn Green
TRUSTEE

Bev Romanoff
VILLAGE CLERK

Cary J. Collins
TRUSTEE

Anna Newell
TRUSTEE

James H. Norris
VILLAGE MANAGER



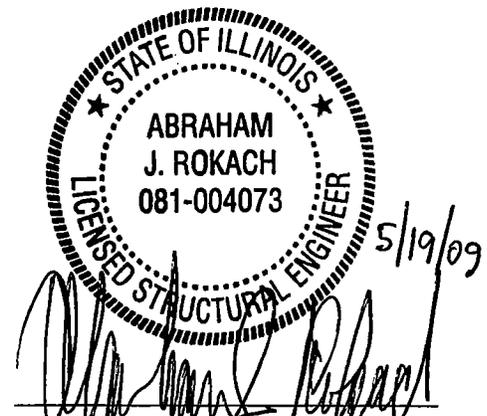
Structural Report

Prepared for: Clear Wireless LLC.

Existing 125 ft. Pedisphere

Clearwire Site No. IL-CHI5380A (IL3759)
Hoffman Estates
3990 Huntington Boulevard
Hoffman Estates, Cook Co., IL 30195

May 19, 2009



Abraham J. Rokach S.E.
Illinois SE License No. 081-04073
Expires 11/30/2010

I certify that this report was prepared by me, or under my direct supervision and control, and, to the best of my knowledge and belief, complies with the requirements of the applicable building code.

Project Summary

Scope:

Structural analysis of existing 125 ft. water tower under proposed antenna configuration.

Design Criteria:

1. International Building Code, 2006 Edition
2. AWWA-D100-05
3. TIA/EIA-222-F 1996

New Equipment Loads:

Antenna: (1) - 2 ft. Dish @ centerline 132' AGL
(1) - 2.5 ft. Dish @ centerline 128' AGL
(2) - 2" Diameter Innerduct w/ cables

Design Loads:

Basic Wind Speed 76 mph (fastest mile) / 90 mph (3-second gust), for appurtenance, per TIA/EIA-222-F 1996.

100 mph (fastest mile) / 120 mph (3-second gust), for water tower structure, per ANSI/AWWA D100-96.

Reference:

Water Tower Drawing, by CB&I, contract no. 61393, dated 1977.

Conclusion:

The structural analysis shows that new dish antenna installation will increase the stresses on the existing water tank. Based on the following calculations, the existing water tank structure, including anchor bolts, are still adequate for new loads.

Structural Analysis on Existing Water Tank

Wind Speed

Effective Wind Speed for Water Tank Supports (AWWA-D100-96)

$V_{eff} := 100\text{mph}$

Wind Speed For Antennas (TIA-EIA-222-F)

$V_{app} := 76\text{mph}$

Material Properties

Material

(36 ksi)

Modulus of Elasticity

$E := 29000\text{ksi}$

Shape Coefficient for Water Tank Structure

Effective Area Coefficient (Flat)

$C_{d_flat} := 1$

Effective Area Coefficient (Cylindrical)

$C_{d_cyl} := 0.6$

Effective Area Coefficient (Sphere)

$C_{d_sphere} := 0.5$

Water Tank Dimensions

Water Tank

Tank Capacity

$Vol := 250000\text{gal}$

Average Tank Diameter

$d_{tank} := 44\text{ft}$

Tank Height

$h_{tank} := 32\text{ft}$

Top of Roof

$H_{roof} := 124\text{ft}$

Top of Capacity Level

$H_{TCL} := 119\text{ft}$

Bottom of Capacity Level

$H_{BCL} := 88\text{ft}$

Mean Height of Water Tank

$$H_{mean} := \frac{H_{TCL} + H_{BCL}}{2}$$

$H_{mean} = 103.5\text{ft}$

Shaft

Diameter of Shaft

$d_{inner} := 9\text{ft}$

Shaft Wall Thickness

$t_{shaft} := 0.8\text{in}$

Inner Diameter of Shaft

$$d_{shaft} := d_{inner} + 2 \cdot t_{shaft}$$

$d_{shaft} = 9.13\text{ft}$

Length of Shaft

$L_{shaft} := 67\text{ft}$

Bell

Height of Bell

$h_{bell} := 16\text{ft}$

Diameter of Bell

$d_{bell} := 19\text{ft}$

Bolt Circle

Diameter of Bolt Circle

$d_{circle} := 19.5\text{ft}$

Number of Bolts

$N_{bolt} := 12$

Diameter of Bolts

$d_{bolt} := 1.875\text{in}$

Shaft Properties

Radius of Gyration	$r_{\text{shaft}} := \sqrt{\frac{d_{\text{shaft}}^2 + d_{\text{inner}}^2}{4}}$	$r_{\text{shaft}} = 76.94 \cdot \text{in}$
Slenderness Ratio	$KL_r := \frac{K \cdot L_{\text{shaft}}}{r_{\text{shaft}}}$	$KL_r = 20.9$
Elastic Section Modulus	$S_{\text{shaft}} := \frac{\pi \cdot (d_{\text{shaft}}^4 - d_{\text{inner}}^4)}{32 \cdot d_{\text{shaft}}}$	$S_{\text{shaft}} = 7383.8 \cdot \text{in}^3$
Gross Area	$A_{g_shaft} := \pi \cdot d_{\text{inner}} \cdot t_{\text{shaft}}$	$A_{g_shaft} = 271.43 \cdot \text{in}^2$
Radius to Thickness Ratio	$R_{\text{shaft}} := \frac{d_{\text{shaft}}}{2}$	$\frac{t_{\text{shaft}}}{R_{\text{shaft}}} = 0.0146$

Wind Load on Structures

Wind Pressure

Flat	$P_{\text{flat}} := 30 \cdot C_{d_flat} \cdot \left(\frac{V_{\text{eff}}}{100 \text{mph}} \right)^2 \cdot \text{psf}$	$P_{\text{flat}} = 30 \cdot \text{psf}$
Cylindrical	$P_{\text{cyl}} := 30 \cdot C_{d_cyl} \cdot \left(\frac{V_{\text{eff}}}{100 \text{mph}} \right)^2 \cdot \text{psf}$	$P_{\text{cyl}} = 18 \cdot \text{psf}$
Sphere	$P_{\text{sphere}} := 30 \cdot C_{d_sphere} \cdot \left(\frac{V_{\text{eff}}}{100 \text{mph}} \right)^2 \cdot \text{psf}$	$P_{\text{sphere}} = 15 \cdot \text{psf}$

Wind Projected Area

Tank	$A_{\text{tank}} := \frac{\pi \cdot d_{\text{tank}}^2}{4}$	$A_{\text{tank}} = 1520.53 \text{ft}^2$
Shaft	$A_{\text{shaft}} := L_{\text{shaft}} \cdot d_{\text{shaft}}$	$A_{\text{shaft}} = 611.93 \text{ft}^2$
Bell	$A_{\text{bell}} := \frac{d_{\text{shaft}} + d_{\text{bell}}}{2} \cdot h_{\text{bell}}$	$A_{\text{bell}} = 225.07 \text{ft}^2$

Wind Load

Tank	$F_{\text{tank}} := A_{\text{tank}} \cdot P_{\text{sphere}}$	$F_{\text{tank}} = 22.81 \cdot \text{kip}$
Shaft	$F_{\text{shaft}} := A_{\text{shaft}} \cdot P_{\text{cyl}}$	$F_{\text{shaft}} = 11.01 \cdot \text{kip}$
Bell	$F_{\text{bell}} := A_{\text{bell}} \cdot P_{\text{cyl}}$	$F_{\text{bell}} = 4.05 \cdot \text{kip}$

Antenna Load Summary

Carrier =	<ul style="list-style-type: none"> "Carrier 1" "(Double Pod)" "Sprint" "Sprint" "Clearwire 1" "Clearwire 1" "Carrier 2" 	Elev_Ant =	<ul style="list-style-type: none"> 140 130 128 128 132 132 80 	ft	Num_Ant =	<ul style="list-style-type: none"> 9 0 6 3 1 1 9 	Num_Cable =	<ul style="list-style-type: none"> 9 0 6 3 1 1 9 					
Antenna =	<ul style="list-style-type: none"> "6 ft. Panel Antennas" "None" "EMS RR65-12-XXXBL" "CSS CSS-XS4-65-R" "2 ft. Dish" "2.5 ft. Dish" "6 ft. Panel Antennas" "Mini Monopole" 	Height =	<ul style="list-style-type: none"> 72 0 48 48 24 30 72 	in	Width =	<ul style="list-style-type: none"> 8 0 12 6.7 24 30 8 	in	Depth =	<ul style="list-style-type: none"> 4 0 7 4.1 6 8 4 	in	Weight =	<ul style="list-style-type: none"> 32 0 25 26 15 32 32 	lb
Mount =	<ul style="list-style-type: none"> "Pod Mount" "Water Tower Rooftop Assembly" "None" "Single Water Tank Mount" "Single Water Tank Mount" "Single Water Tank Mount" 	Num_Mnt =	<ul style="list-style-type: none"> 1 2 1 1 1 6 	A_Mnt =	<ul style="list-style-type: none"> 16 20 20 0 3 3 3 	ft ²	Wt_Mnt =	<ul style="list-style-type: none"> 1532 3000 1000 0 80 80 80 	lb				

Force Coefficient on Appurtenance $Ca(As, F) :=$

$Ca \leftarrow 0$	$Ca \leftarrow 0$
if	$F = 1$
$Ca \leftarrow 1.4$	if $As < 7$
$Ca \leftarrow 2.0$	if $As > 25$
$Ca \leftarrow 1.4 + \frac{(As - 7)}{18} \cdot 0.6$	$otherwise$
$otherwise$	$Ca \leftarrow 0.8$
$Ca \leftarrow 1.2$	if $As > 25$
$Ca \leftarrow 0.8 + \frac{(As - 7)}{18} \cdot 0.4$	$otherwise$
Ca	Ca

Wind Pressure on Appurtenance

$$\begin{aligned}
 P_{Ant} := & \text{for } i \in 1..N \\
 & \left[\begin{aligned}
 Kz_i & \leftarrow \left(\frac{\text{Elev_Ant}_i}{33\text{ft}} \right)^{0.2857} \\
 qz_i & \leftarrow \left[0.00256 \cdot Kz_i \cdot \left(\frac{V_{app}}{\text{mph}} \right)^2 \right] \cdot \text{psf} \\
 Gh_i & \leftarrow 0.65 + \frac{0.6}{\left(\frac{\text{Elev_Ant}_i}{33\text{ft}} \right)^{0.1429}} \\
 P & \leftarrow qz_i \cdot Gh_i
 \end{aligned} \right. \\
 & P_{Ant} = \begin{pmatrix} 26.34 \\ 25.79 \\ 25.67 \\ 25.67 \\ 25.9 \\ 25.9 \\ 22.45 \end{pmatrix} \cdot \text{psf}
 \end{aligned}$$

New Appurtenance

Antenna 1

Carrier_C = "Carrier 1"

C_w := 1

n := 1 F_w := 1

Model / Quantity

Antenna_n = "6 ft. Panel Antennas"

Elev_Ant_n = 140 ft

Dimension

Height_n = 72-in Width_n = 8-in Depth_n = 4-in Weight_n = 32 lb

Number of Antennas & Cables

Num_Ant_n = 9 Num_Cable_n = 9

Aspect Ratio & Force Coefficient

As := $\frac{\text{Height}_n}{\text{Width}_n}$ As = 9 CA := Ca(As, F) CA = 1.47

Area / Antenna:

A_ant_n := CA Height_n · Width_n A_ant_n = 5.87 ft²

Platform

Model & Quantity

Mount_C = "Mini Monopole"

Num_Mnt_C = 1

Wt_Mnt_C = 1532 lb A_Mnt_C = 16 ft²

New Appurtenance Load

Total Area of Appurtenance

$$A_{appC} := \sum_{i=1}^1 (\text{Num_Ant}_i \cdot A_{ant_i} + \text{Num_Mnt}_i \cdot A_{Mnt_i}) \quad A_{appC} = 68.8 \text{ft}^2$$

Total Weight of Appurtenance

$$W_{appC} := \sum_{i=1}^1 (\text{Num_Ant}_i \cdot \text{Weight}_i + \text{Num_Mnt}_i \cdot \text{Wt_Mnt}_i) \quad W_{appC} = 1820 \text{lb}$$

Existing Appurtenance**Antenna 2**Carrier_C = "(Double Pod)"

$$\frac{C}{W} := 2$$

$$\frac{n}{w} := 2 \quad \frac{F}{w} := 1$$

Model

Antenna_n = "None"Elev_Ant_n = 130 ft

Dimension

Height_n = 0·inWidth_n = 0·inDepth_n = 0·inWeight_n = 0 lb

Number of Antennas & Cables

Num_Ant_n = 0Num_Cable_n = 0

Aspect Ratio:

$$\frac{As}{ww} := \frac{\text{Height}_n}{\text{Width}_n}$$

As = 0

Force Coefficient

$$\frac{CA}{wwww} := Ca(As, F)$$

CA = 1.4

Area / Antenna:

$$A_{\text{ant}_n} := CA \cdot \text{Height}_n \cdot \text{Width}_n$$

A_{ant_n} = 0 ft²**Platform**

Model & Quantity

Mount_C = "Pod Mount"Num_Mnt_C = 2Wt_Mnt_C = 3000 lbA_Mnt_C = 20 ft²**Existing Appurtenance Load**

Total Area of Appurtenance

$$A_{\text{app}_C} := \sum_{i=2}^2 (\text{Num_Ant}_i \cdot A_{\text{ant}_i} + \text{Num_Mnt}_i \cdot A_{\text{Mnt}_i}) \quad A_{\text{app}_C} = 40 \text{ ft}^2$$

Total Weight of Appurtenance

$$W_{\text{app}_C} := \sum_{i=2}^2 (\text{Num_Ant}_i \cdot \text{Weight}_i + \text{Num_Mnt}_i \cdot \text{Wt_Mnt}_i) \quad W_{\text{app}_C} = 6000 \text{ lb}$$

Existing Appurtenance**Antenna 3**Carrier_C = "Sprint"

$$\frac{C}{W} := 3$$

$$\frac{n}{w} := 3 \quad \frac{F}{w} := 1$$

Model / Quantity

Antenna_n = "EMS RR65-12-XXXBL"Elev_Ant_n = 128 ft

Dimension

Height_n = 48·inWidth_n = 12·inDepth_n = 7·inWeight_n = 25 lb

Number of Antennas & Cables

Num_Ant_n = 6Num_Cable_n = 6

Aspect Ratio & Force Coefficient

$$\frac{As}{ww} := \frac{\text{Height}_n}{\text{Width}_n}$$

As = 4

$$\frac{CA}{wwww} := Ca(As, F)$$

CA = 1.4

Area / Antenna:

$$A_{\text{ant}_n} := CA \cdot \text{Height}_n \cdot \text{Width}_n$$

A_{ant_n} = 5.6 ft²**Platform**

Model & Quantity

Mount_C = "Water Tower Rooftop Assembly"Num_Mnt_C = 1Wt_Mnt_C = 1000 lbA_Mnt_C = 20 ft²

Existing Appurtenance Load

Total Area of Appurtenance $A_{appC} := \sum_{i=3}^3 (\text{Num_Ant}_i \cdot A_{ant_i} + \text{Num_Mnt}_i \cdot A_{Mnt_i}) \quad A_{appC} = 53.6 \text{ft}^2$

Total Weight of Appurtenance $W_{appC} := \sum_{i=3}^3 (\text{Num_Ant}_i \cdot \text{Weight}_i + \text{Num_Mnt}_i \cdot \text{Wt_Mnt}_i) \quad W_{appC} = 1150 \text{lb}$

Existing Appurtenance

Antenna 4	Carrier _C = "Sprint"	$C_w := 4$
Model / Quantity	Antenna _n = "CSS CSS-XS4-65-R"	$n_w := 4 \quad F_w := 1$
Dimension	Height _n = 48 · in Width _n = 6.7 · in Depth _n = 4.1 · in Weight _n = 26 lb	
Number of Antennas & Cables		Num_Ant _n = 3 Num_Cable _n = 3
Aspect Ratio & Force Coefficient	$As := \frac{\text{Height}_n}{\text{Width}_n} \quad As = 7.16$	$CA := Ca(As, F) \quad CA = 1.41$
Area / Antenna:	$A_{ant_n} := CA \cdot \text{Height}_n \cdot \text{Width}_n$	$A_{ant_n} = 3.14 \text{ft}^2$

Platform

Model & Quantity	Mount _C = "None"	Num_Mnt _C = 1
		Wt_Mnt _C = 0 lb A_Mnt _C = 0 ft ²

Existing Appurtenance Load

Total Area of Appurtenance $A_{appC} := \sum_{i=4}^4 (\text{Num_Ant}_i \cdot A_{ant_i} + \text{Num_Mnt}_i \cdot A_{Mnt_i}) \quad A_{appC} = 9.42 \text{ft}^2$

Total Weight of Appurtenance $W_{appC} := \sum_{i=4}^4 (\text{Num_Ant}_i \cdot \text{Weight}_i + \text{Num_Mnt}_i \cdot \text{Wt_Mnt}_i) \quad W_{appC} = 78 \text{lb}$

Existing Appurtenance

Antenna 5	Carrier _C = "Clearwire 1"	$C_w := 5$
Model / Quantity	Antenna _n = "2 ft. Dish"	$n_w := 5 \quad F_w := 1$
Dimension	Height _n = 24 · in Width _n = 24 · in Depth _n = 6 · in Weight _n = 15 lb	
Number of Antennas & Cables		Num_Ant _n = 1 Num_Cable _n = 1
Aspect Ratio & Force Coefficient	$As := \frac{\text{Height}_n}{\text{Width}_n} \quad As = 1$	$CA := Ca(As, F) \quad CA = 1.4$

Area / Antenna: $A_{ant_n} := CA \cdot Height_n \cdot Width_n$ $A_{ant_n} = 5.6 \text{ ft}^2$

Platform

Model & Quantity $Mount_C = \text{"Single Water Tank Mount"}$ $Num_Mnt_C = 1$

$Wt_Mnt_C = 80 \text{ lb}$ $A_Mnt_C = 3 \text{ ft}^2$

Existing Appurtenance Load

Total Area of Appurtenance $A_{appC} := \sum_{i=5}^5 (Num_Ant_i \cdot A_{ant_i} + Num_Mnt_i \cdot A_{Mnt_i})$ $A_{appC} = 8.6 \text{ ft}^2$

Total Weight of Appurtenance $W_{appC} := \sum_{i=5}^5 (Num_Ant_i \cdot Weight_i + Num_Mnt_i \cdot Wt_Mnt_i)$ $W_{appC} = 95 \text{ lb}$

Existing Appurtenance

Antenna 6 $Carrier_C = \text{"Clearwire 1"}$ $C_w := 6$
 $n_w := 6$ $F_w := 1$

Model / Quantity $Antenna_n = \text{"2.5 ft. Dish"}$ $Elev_Ant_n = 132 \text{ ft}$

Dimension $Height_n = 30 \cdot \text{in}$ $Width_n = 30 \cdot \text{in}$ $Depth_n = 8 \cdot \text{in}$ $Weight_n = 32 \text{ lb}$

Number of Antennas & Cables $Num_Ant_n = 1$ $Num_Cable_n = 1$

Aspect Ratio & Force Coefficient $As := \frac{Height_n}{Width_n}$ $As = 1$ $CA_w := Ca(As, F)$ $CA = 1.4$

Area / Antenna: $A_{ant_n} := CA \cdot Height_n \cdot Width_n$ $A_{ant_n} = 8.75 \text{ ft}^2$

Platform

Model & Quantity $Mount_C = \text{"Single Water Tank Mount"}$ $Num_Mnt_C = 1$

$Wt_Mnt_C = 80 \text{ lb}$ $A_Mnt_C = 3 \text{ ft}^2$

Existing Appurtenance Load

Total Area of Appurtenance $A_{appC} := \sum_{i=6}^6 (Num_Ant_i \cdot A_{ant_i} + Num_Mnt_i \cdot A_{Mnt_i})$ $A_{appC} = 11.75 \text{ ft}^2$

Total Weight of Appurtenance $W_{appC} := \sum_{i=6}^6 (Num_Ant_i \cdot Weight_i + Num_Mnt_i \cdot Wt_Mnt_i)$ $W_{appC} = 112 \text{ lb}$

Existing Appurtenance

$C := 7$

Antenna 7

Carrier_C = "Carrier 2"

$n := 7$ $F_w := 1$

Model / Quantity

Antenna_n = "6 ft. Panel Antennas"

Elev_Ant_n = 80 ft

Dimension

Height_n = 72 ·in

Width_n = 8 ·in

Depth_n = 4 ·in

Weight_n = 32 lb

Number of Antennas & Cables

Num_Ant_n = 9

Num_Cable_n = 9

Aspect Ratio & Force Coefficient

$A_s := \frac{\text{Height}_n}{\text{Width}_n}$ $A_s = 9$

$C_A := C_a(A_s, F)$ $C_A = 1.47$

Area / Antenna:

$A_{\text{ant}_n} := C_A \text{Height}_n \cdot \text{Width}_n$

$A_{\text{ant}_n} = 5.87 \text{ft}^2$

Platform

Model & Quantity

Mount_C = "Single Water Tank Mount"

Num_Mnt_C = 6

Wt_Mnt_C = 80 lb

A_Mnt_C = 3 ft²

Existing Appurtenance Load

Total Area of Appurtenance

$A_{\text{app}_C} := \sum_{i=7}^7 (\text{Num_Ant}_i \cdot A_{\text{ant}_i} + \text{Num_Mnt}_i \cdot A_{\text{Mnt}_i})$ $A_{\text{app}_C} = 70.8 \text{ft}^2$

Total Weight of Appurtenance

$W_{\text{app}_C} := \sum_{i=7}^7 (\text{Num_Ant}_i \cdot \text{Weight}_i + \text{Num_Mnt}_i \cdot \text{Wt_Mnt}_i)$ $W_{\text{app}_C} = 768 \text{lb}$

Summary of Loads

Appurtenance Wind Load & Weight

Carrier =	$\left(\begin{array}{c} \text{"Carrier 1"} \\ \text{"(Double Pod)"} \\ \text{"Sprint"} \\ \text{"Sprint"} \\ \text{"Clearwire 1"} \\ \text{"Clearwire 1"} \\ \text{"Carrier 2"} \end{array} \right)$	$F_{Ant} := \left\{ \begin{array}{l} \text{for } i \in 1..C \\ F_i \leftarrow A_{appi} \cdot P_{Ant_i} \\ F \end{array} \right.$	$F_{Ant} = \begin{pmatrix} 1.81 \\ 1.03 \\ 1.38 \\ 0.24 \\ 0.22 \\ 0.3 \\ 1.59 \end{pmatrix} \cdot \text{kip}$	$W_{app} = \begin{pmatrix} 1820 \\ 6000 \\ 1150 \\ 78 \\ 95 \\ 112 \\ 768 \end{pmatrix} \text{ lb}$
-----------	---	--	--	---

Total Antenna Wind Load

$$F_{Ant_{total}} := \sum_{i=1}^C F_{Ant_i} \quad F_{Ant_{total}} = 6.58 \cdot \text{kip}$$

Total Weight of Appurtenance

$$W_{App_{total}} := \sum_{i=1}^C W_{app_i} \quad W_{App_{total}} = 10.02 \cdot \text{kip}$$

Structure

Wind Load

Weight

$$F_{tank} = 22.81 \cdot \text{kip}$$

$$W_{water} := 62.4 \text{pcf} \cdot \text{Vol}$$

$$W_{water} = 2085.42 \cdot \text{kip}$$

$$F_{shaft} = 11.01 \cdot \text{kip}$$

$$W_{structure} := 10\% \cdot W_{water}$$

$$W_{structure} = 208.54 \cdot \text{kip}$$

$$F_{bell} = 4.05 \cdot \text{kip}$$

Overturning Moment right above the Bell

$$M_{tank} := F_{tank} \cdot (H_{mean} - h_{bell}) \quad M_{tank} = 1995.7 \cdot \text{kip} \cdot \text{ft}$$

$$M_{shaft} := F_{shaft} \cdot \frac{L_{shaft}}{2} \quad M_{shaft} = 369 \cdot \text{kip} \cdot \text{ft}$$

$$M_{Ant_bell_1} := F_{Ant_1} \cdot (Elev_{Ant_1} - h_{bell}) \quad M_{Ant_bell_1} = 224.69 \cdot \text{kip} \cdot \text{ft}$$

$$M_{Ant_bell_2} := F_{Ant_2} \cdot (Elev_{Ant_2} - h_{bell}) \quad M_{Ant_bell_2} = 117.58 \cdot \text{kip} \cdot \text{ft}$$

$$M_{Ant_bell_3} := F_{Ant_3} \cdot (Elev_{Ant_3} - h_{bell}) \quad M_{Ant_bell_3} = 154.11 \cdot \text{kip} \cdot \text{ft}$$

$$M_{Ant_bell_4} := F_{Ant_4} \cdot (Elev_{Ant_4} - h_{bell}) \quad M_{Ant_bell_4} = 27.08 \cdot \text{kip} \cdot \text{ft}$$

$$M_{Ant_bell_5} := F_{Ant_5} \cdot (Elev_{Ant_5} - h_{bell}) \quad M_{Ant_bell_5} = 25.84 \cdot \text{kip} \cdot \text{ft}$$

$$M_{Ant_bell_6} := F_{Ant_6} \cdot (Elev_{Ant_6} - h_{bell}) \quad M_{Ant_bell_6} = 35.3 \cdot \text{kip} \cdot \text{ft}$$

$$M_{Ant_bell_7} := F_{Ant_7} \cdot (Elev_{Ant_7} - h_{bell}) \quad M_{Ant_bell_7} = 101.71 \cdot \text{kip} \cdot \text{ft}$$

Total Overturning Moment

$$M_{total_bell} := M_{tank} + M_{shaft} + \sum_{i=1}^C M_{Ant_bell_i} \quad M_{total_bell} = 3051 \cdot \text{kip} \cdot \text{ft}$$

Overturning Moment at Anchor Bolts Level

$M_{\text{tank}} := F_{\text{tank}} \cdot H_{\text{mean}}$	$M_{\text{tank}} = 2360.62 \cdot \text{kip} \cdot \text{ft}$
$M_{\text{shaft}} := F_{\text{shaft}} \cdot \left(\frac{L_{\text{shaft}}}{2} + h_{\text{bell}} \right)$	$M_{\text{shaft}} = 545.23 \cdot \text{kip} \cdot \text{ft}$
$M_{\text{bell}} := F_{\text{bell}} \cdot \frac{h_{\text{bell}}}{2}$	$M_{\text{bell}} = 32.41 \cdot \text{kip} \cdot \text{ft}$
$M_{\text{Ant_anch1}} := F_{\text{Ant1}} \cdot \text{Elev_Ant1}$	$M_{\text{Ant_anch1}} = 253.68 \cdot \text{kip} \cdot \text{ft}$
$M_{\text{Ant_anch2}} := F_{\text{Ant2}} \cdot \text{Elev_Ant2}$	$M_{\text{Ant_anch2}} = 134.09 \cdot \text{kip} \cdot \text{ft}$
$M_{\text{Ant_anch3}} := F_{\text{Ant3}} \cdot \text{Elev_Ant3}$	$M_{\text{Ant_anch3}} = 176.13 \cdot \text{kip} \cdot \text{ft}$
$M_{\text{Ant_anch4}} := F_{\text{Ant4}} \cdot \text{Elev_Ant4}$	$M_{\text{Ant_anch4}} = 30.94 \cdot \text{kip} \cdot \text{ft}$
$M_{\text{Ant_anch5}} := F_{\text{Ant5}} \cdot \text{Elev_Ant5}$	$M_{\text{Ant_anch5}} = 29.4 \cdot \text{kip} \cdot \text{ft}$
$M_{\text{Ant_anch6}} := F_{\text{Ant6}} \cdot \text{Elev_Ant6}$	$M_{\text{Ant_anch6}} = 40.17 \cdot \text{kip} \cdot \text{ft}$
$M_{\text{Ant_anch7}} := F_{\text{Ant7}} \cdot \text{Elev_Ant7}$	$M_{\text{Ant_anch7}} = 127.13 \cdot \text{kip} \cdot \text{ft}$

Total Overturning Moment $M_{\text{total_anchor}} := M_{\text{tank}} + M_{\text{shaft}} + M_{\text{bell}} + \sum_{i=1}^C M_{\text{Ant_anch}_i}$ $M_{\text{total_anchor}} = 3729.81 \cdot \text{kip} \cdot \text{ft}$

Allowable Stresses on Shaft

Allowable Local Buckling Stress
per AWWA-D100-96 Class II Material

$F_L := 18 \text{ksi}$

Slenderness Reduction Factor

$$K_{\psi} := \begin{cases} C_c \leftarrow \sqrt{\frac{\pi^2 \cdot E}{F_L}} \\ K \leftarrow 1 - \frac{1}{2} \cdot \left(\frac{KL_r}{C_c} \right)^2 & \text{if } 25 < KL_r \leq C_c \\ K \leftarrow \frac{1}{2} \cdot \left(\frac{KL_r}{C_c} \right)^2 & \text{if } KL_r > C_c \\ K \leftarrow 1 & \text{if } KL_r \leq 25 \\ K \end{cases}$$

$K_{\psi} = 1$

Allowable Axial Stress

$F_{a_shaft} := F_L \cdot K_{\psi}$

$F_{a_shaft} = 18 \cdot \text{ksi}$

Allowable Shear Stress

$F_{v_shaft} := F_L$

$F_{v_shaft} = 18 \cdot \text{ksi}$

Allowable Bending Stress

$F_{b_shaft} := F_L$

$F_{b_shaft} = 18 \cdot \text{ksi}$

Stress Analysis

Shear Stress

$$f_{v_shaft} := \frac{F_{tank} + F_{shaft} + F_{Ant_{total}}}{A_{g_shaft}} \quad f_{v_shaft} = 0.15 \cdot \text{ksi}$$

$$f_{v_shaft} < F_{v_shaft} = 1 \quad \text{CHECK} = \text{"Shaft is adequate for shear loads"}$$

Axial Stress

$$f_{a_shaft} := \frac{W_{water} + W_{structure} + W_{App_{total}}}{A_{g_shaft}} \quad f_{a_shaft} = 8.49 \cdot \text{ksi}$$

$$f_{a_shaft} < F_{a_shaft} = 1 \quad \text{CHECK} = \text{"Shaft is adequate for axial loads"}$$

Bending Stress

$$M_{total} := M_{total_bell} \quad M_{total} = 3051 \cdot \text{kip} \cdot \text{ft}$$

$$f_{b_shaft} := \frac{M_{total}}{S_{shaft}} \quad f_{b_shaft} = 4.96 \cdot \text{ksi}$$

$$f_{b_shaft} < F_{b_shaft} = 1 \quad \text{CHECK} = \text{"Shaft is adequate for bending"}$$

Combined Stresses (Increased by 1.33 for wind)

$$\text{Stress} := \frac{f_{a_shaft}}{1.33F_{a_shaft}} + \frac{f_{b_shaft}}{1.33F_{b_shaft}} \quad \text{Stress} = 56.17 \cdot \%$$

Load Increases Due to Antennas

Increase in Lateral Load

$$\%V := \frac{F_{Ant_{total}}}{F_{tank} + F_{shaft} + F_{bell}} \quad \%V = 17.37 \cdot \%$$

Increase in Axial Load

$$\%P := \frac{W_{App_{total}}}{W_{water} + W_{structure}} \quad \%P = 0.44 \cdot \%$$

Increase in Moment Load

$$\%M := \frac{\sum_{i=1}^C M_{Ant_anch_i}}{M_{tank} + M_{shaft}} \quad \%M = 27.24 \cdot \%$$

Check Anchor Bolts

Diameter of Bolts & Number of Bolts

$$d_{\text{bolt}} = 88 \cdot \text{in} \quad N_{\text{bolt}} = 12$$

Cross Section Area

$$A_{\text{gbolt}} := \frac{\pi \cdot d_{\text{bolt}}^2}{4} \quad A_{\text{gbolt}} = 76 \cdot \text{in}^2$$

Diameter of Bolt Circle

$$d_{\text{circle}} = 19.5 \cdot \text{ft}$$

Allowable Strength on Anchor Bolt (AWWA-D100-05)

$$F_{t,\text{bolt}} = 31.25 \cdot \text{ksi}$$

Moment of Intertia

$$I_{\text{bolt}} := \begin{cases} I_{\text{total}} \leftarrow 0 \cdot \text{in}^4 \\ \text{for } i \in 1..N_{\text{bolt}} \\ \quad d_i \leftarrow \frac{d_{\text{circle}}}{2} \cdot \sin \left[\frac{360 \cdot \text{deg}}{N_{\text{bolt}}} \cdot (i - 1) \right] \\ \quad I_i \leftarrow A_{\text{gbolt}} \cdot (d_i)^2 \\ \quad I_{\text{total}} \leftarrow I_{\text{total}} + I_i \\ I_{\text{total}} \end{cases} \quad I_{\text{bolt}} = 6785.56 \cdot \text{in}^4$$

Section Modulus of Bolt Configuration

$$S_{\text{bolt}} := \frac{I_{\text{bolt}}}{d_{\text{circle}}} \quad S_{\text{bolt}} = 9.17 \cdot \text{in}^3$$

Overturning Moment at Anchor Bolt Level

$$M_{\text{total}} := M_{\text{total_anchor}} \quad M_{\text{total}} = 3729.81 \cdot \text{kip} \cdot \text{ft}$$

Stresses on Anchor Bolts due to Bending

$$f_b := \frac{M_{\text{total}}}{S_{\text{bolt}}} \quad f_b = 46.18 \cdot \text{ksi}$$

Stresses on Anchor Bolts due to Axial Loads

$$f_t := \frac{W_{\text{structure}} + W_{\text{Apptotal}}}{N_{\text{bolt}} \cdot A_{\text{gbolt}}} \quad f_t = 6.6 \cdot \text{ksi}$$

Uplift are possible to occur

$$T_w := f_b - f_t \quad T = 39.59 \cdot \text{ksi}$$

Allowable Tension Stress

$$F_t := 1.33 \cdot F_{t,\text{bolt}} \quad F_t = 41.56 \cdot \text{ksi}$$

$$\text{CHECK} = \text{"Anchor Bolts are adequate for tension"} \quad \frac{T}{F_t} = 95.24 \cdot \%$$

Conclusions

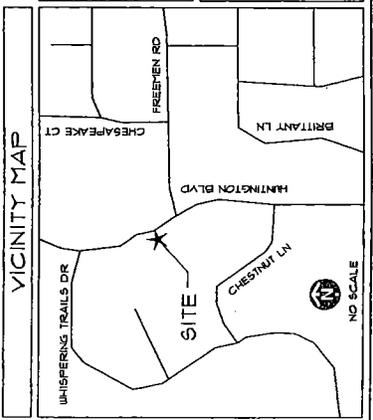
(2) New dish antennas will be installed on the existing antenna mount, by extending the support pipe of existing installation.

Further structural analysis shows that existing water tower structure is still **adequate** for new loads. Stresses above the bell and possibility of uplift are checked. Anchor bolts are also **adequate** for new loads.

SHEET INDEX	
NO.	DESCRIPTION
T-1	TITLE SHEET
C-1	SITE PLAN
C-2	ENLARGED SITE PLAN
C-3	SHELTER/ANTENNA PLAN
C-4	SITE ELEVATION & DETAILS
C-5	DETAILS
E-1	SINGLE LINE NOTES

DRIVING DIRECTIONS

DEPART FROM CLEARWIRE OFFICE,
 3950 HUNTINGTON BOULEVARD, IL 60195
 DEPART ON N. RIVER RD (N. RIVER RD) TURN LEFT (WEST) ONTO DR-72
 (W. JIGGINS RD) TURN RIGHT ONTO RAMP TOLL ROAD MERGE
 RIGHT TO 9 (NORTHWEST TOLLWAY) TURN RIGHT ONTO RAMP KEEP
 LEFT TO 5 (ROSELLE RD) TURN LEFT (WEST) ONTO DR-62 (W. ALCONQUIN RD)
 TURN RIGHT (NORTH) ONTO HUNTINGTON BLVD (ARRIVE 3950
 HUNTINGTON BLVD, SCHAMBERG, IL 60195)



clear w'reless LLC[®]

a Nevada limited liability company, a Sprint affiliate

SITE NAME
HOFFMAN ESTATES

SITE NUMBER
IL-CH15380

PROJECT TYPE
BACKHAUL PROJECT

APPLICANT
 clear w'reless LLC[®]
 a Nevada limited liability company,
 a Sprint affiliate
 5600 N. RIVER RD.
 ROSEMONT, ILLINOIS 60018
 (647) 318-3000

REAL ESTATE
 EVEREST GROUP
 THE EVEREST GROUP, LLC
 1015 W. WIRTH ST.
 WORTH, IL 60187
 TEL: (708) 503-1563

PROJECT TEAM
 W'LESS ENGINEERING CONSULTANTS, INC.
 3100 W. HICKENS RD.
 SUITE 800
 ROSEMONT, ILLINOIS 60018
 TEL: 647-332-0200
 FAX: 647-332-0205

SCOPE OF WORK:

- HANDICAP ACCESS REQUIREMENTS ARE NOT REQUIRED
- FACILITY IS UNHANNED AND NOT FOR HUMAN HABITATION
- FACILITY HAS NO PLUMBING OR REFRIGERANTS
- THIS FACILITY SHALL MEET OR EXCEED ALL FAA AND FCC REGULATORY REQUIREMENTS
- ALL WORK MUST CONFORM TO CLEARWIRE 'CLEARWIRE CONSTRUCTION INSTALLATION GUIDE - WH'AX'

SCOPE OF WORK:

- NEW CLEARWIRE BACKHAUL DISH ANTENNAS INSTALLED ON EXISTING WATER TANK.
- NEW CLEARWIRE BACKHAUL EQUIPMENT INSTALLED INSIDE EXISTING SHELTER.

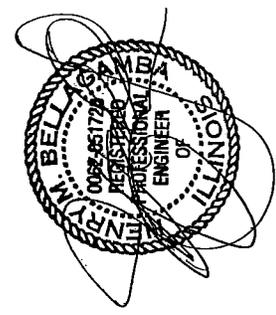
PROJECT SUMMARY

SITE NAME:	HOFFMAN ESTATES
SITE NO.:	IL-CH15380
SERVANT HOBI NO.:	IL37194
SITE ADDRESS:	3950 HUNTINGTON BOULEVARD HOFFMAN ESTATES, IL 60195
COUNTY:	COOK
LANDLORED ADDRESS:	VILLAGE OF HOFFMAN ESTATES 1900 HASSSELL ROAD HOFFMAN ESTATES, IL 60195 PHONE: 847-897-5000
SITE COORDINATES (FROTH CLEARVISION):	N 42.9541° (NAD 83) W 88.1225° (NAD 83)
LANDLORED:	
ZONING:	
JURISDICTION:	
BUILDING CODE:	INTERNATIONAL BUILDING CODE 2003 EDITION
ELECTRICAL CODE:	NATIONAL ELECTRICAL CODE 2003 EDITION

ENGINEER'S LICENSE

I CERTIFY THAT THESE DRAWINGS WERE PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND CONTROL AND TO THE BEST OF MY KNOWLEDGE AND BELIEF COMPLY WITH THE REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE, 2003 EDITION

LICENSED ENGINEER - STATE OF ILLINOIS



APPROVALS

CLEARWIRE CONST.:	DATE
CLEARWIRE RF:	DATE
CLEARWIRE OPS:	DATE
LANDLORD:	DATE

EXPIRES: 11/30/07 **SIGNED:** 6/18/07

DRAWING SCALED TO 11"x17"

clear w'reless LLC[®]
 a Nevada limited liability company,
 a Sprint affiliate
 5600 N. RIVER RD.
 ROSEMONT, ILLINOIS 60018
 (647) 318-3000

W'LESS ENGINEERING CONSULTANTS, INC.
 3100 W. HICKENS RD.
 SUITE 800
 ROSEMONT, ILLINOIS 60018
 TEL: 647-332-0200
 FAX: 647-332-0205
 DESIGN FIRM NO. 184-0224493

DATE	DESCRIPTION	BY
05/07/07	50% REVIEW	JP
05/16/07	FINAL	JP

SITE NAME: HOFFMAN ESTATES
SITE NO.: IL-CH15380
SITE ADDRESS: 3950 HUNTINGTON BOULEVARD
HOFFMAN ESTATES, IL 60195
SHEET NAME: TITLE SHEET
SHEET NUMBER: T-1

THIS DRAWING IS THE PROPERTY OF W'LESS ENGINEERING CONSULTANTS, INC. IT IS FOR THE EXCLUSIVE USE OF THIS PROJECT. ANY REUSE OF THIS DRAWING WITHOUT THE EXPRESS WRITTEN CONSENT OF W'LESS ENGINEERING CONSULTANTS, INC. IS PROHIBITED.

clear wireless LLC
 a Nevada limited liability company,
 a Sprint affiliate
 5602 N. RIVER RD.
 SUITE 100
 ROSEMONT, IL 60018
 TEL: 847-331-0700
 FAX: 847-331-0700
 DESIGN FIRM NO. 84-002498



APPROVED BY:	J.P.	M.S.
DATE	DESCRIPTION	INT.
02/07/05	50% REVIEW	NO
02/09/05	FINAL	NO

3100 Engineering Consultant
 5100 W. WASHINGTON RD.
 SUITE 6020
 ROSEMONT, ILLINOIS 60018
 TEL: 847-331-0700
 FAX: 847-331-0700
 DESIGN FIRM NO. 84-002498

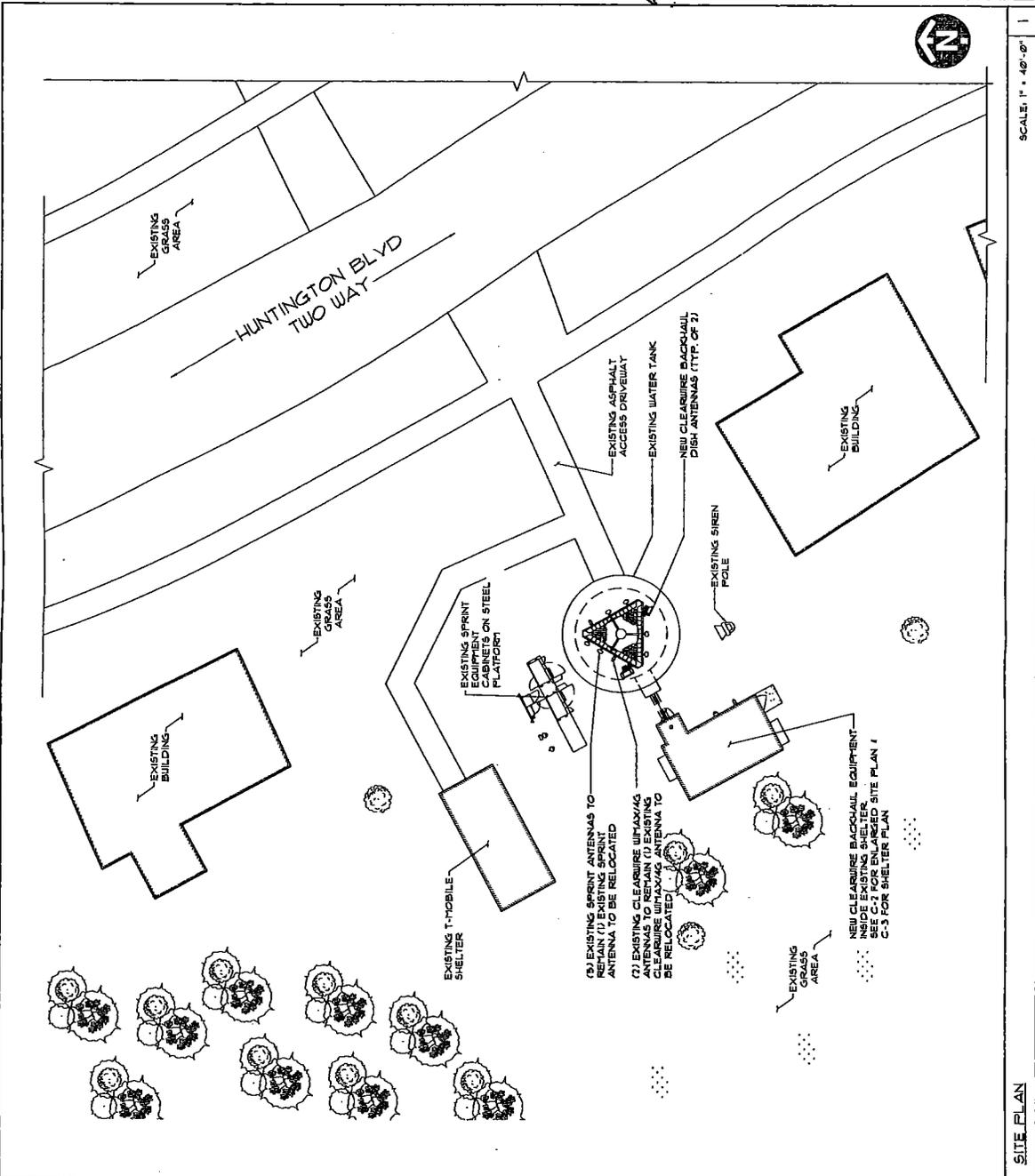
SITE NAME
HOFFMAN ESTATES

SITE NO.
IL-CH15380

SITE ADDRESS
 3950 HUNTINGTON BOULEVARD
 HOFFMAN ESTATES, IL 60143

SHEET NAME
SITE PLAN

SHEET NUMBER
C-1



<p>ABBREVIATIONS</p> <p>A/C AIR CONDITIONER AFF ABOVE FINISHED FLOOR AGL ABOVE GRADE LEVEL ALN ALTERNATE AREA LEVEL APPX APPROX AUS AMERICAN WIRE GAUGE BLDG BUILDING CAB CABINET COL COLUMN CONC CONCRETE CONDIT CONDIT CNG CNG F1 FOOT (FEET) EGBS EQUIPMENT GROUND BAR ELEC ELECTRICAL ENT ENT EQUIP ELECTRICAL METALLIC TUBING EQUIPMENT (E) EXISTING GALV GALVANIZED GND GROUND GPS GLOBAL POSITIONING SYSTEM (LB (') POINT(S) MAX MAX MFR MANUFACTURER MNT MAINT HNTY HUNTER NEW NEW NEC NATIONAL ELECTRICAL CODE NTS NOT TO SCALE OE/OT OVERHEAD ELECTRIC/TELEPHONE POS POSITION POS POSITION SQR SQUARE STL STEEL TOP & BOTTOM T, I, B TO BE DETERMINED TBD TO BE DETERMINED TYP TYPICAL U&AT UNDERGROUND ELECTRIC/TELEPHONE UNRES UNRES VIF VERT IN FIELD W/ WITH XFMR TRANSFORMER</p>	<p>SYMBOLS</p> <p>CENTERLINE PLATE REVISION WORK POINT UTILITY POLE BRICK COMPRESSED STONE CONCRETE EARTH GRAVEL MASONRY STEEL CENTERLINE PROPERTY LINE LEASE LINE EASEMENT LINE CHAIN LINK FENCE WOOD FENCE BELOW GRADE ELECTRIC TELEPHONE OVERHEAD ELECTRIC/TELEPHONE SECTION REFERENCE</p>
--	---

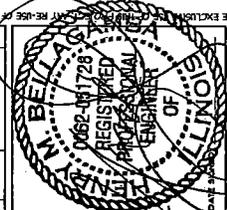
SCALE: 1" = 40'-0"

SITE PLAN

clear wireless LLC
 a Nevada limited liability company,
 a Sprint affiliate
 5400 N. RIVER RD.
 ROSEMONT, ILLINOIS 60018
 (847) 318-3000

ROSEMONT, ILLINOIS 60018
 TEL: 847-337-0200
 FAX: 847-337-0205
 DESIGN FIRM NO. 04-000198

CHECKED BY:	J.P.
APPROVED BY:	M.B.
DATE:	05/10/09
DESCRIPTION:	50A REVIEW
DATE:	05/10/09
BY:	FINAL



SITE NAME
HOFFMAN ESTATES

SITE NO.
IL-CHI5300

SITE ADDRESS
**3950 WASHINGTON BOULEVARD
 HOFFMAN ESTATES, IL 60135**

SHEET NAME
**ELEVATION &
 DETAILS**

SHEET NUMBER
C-4

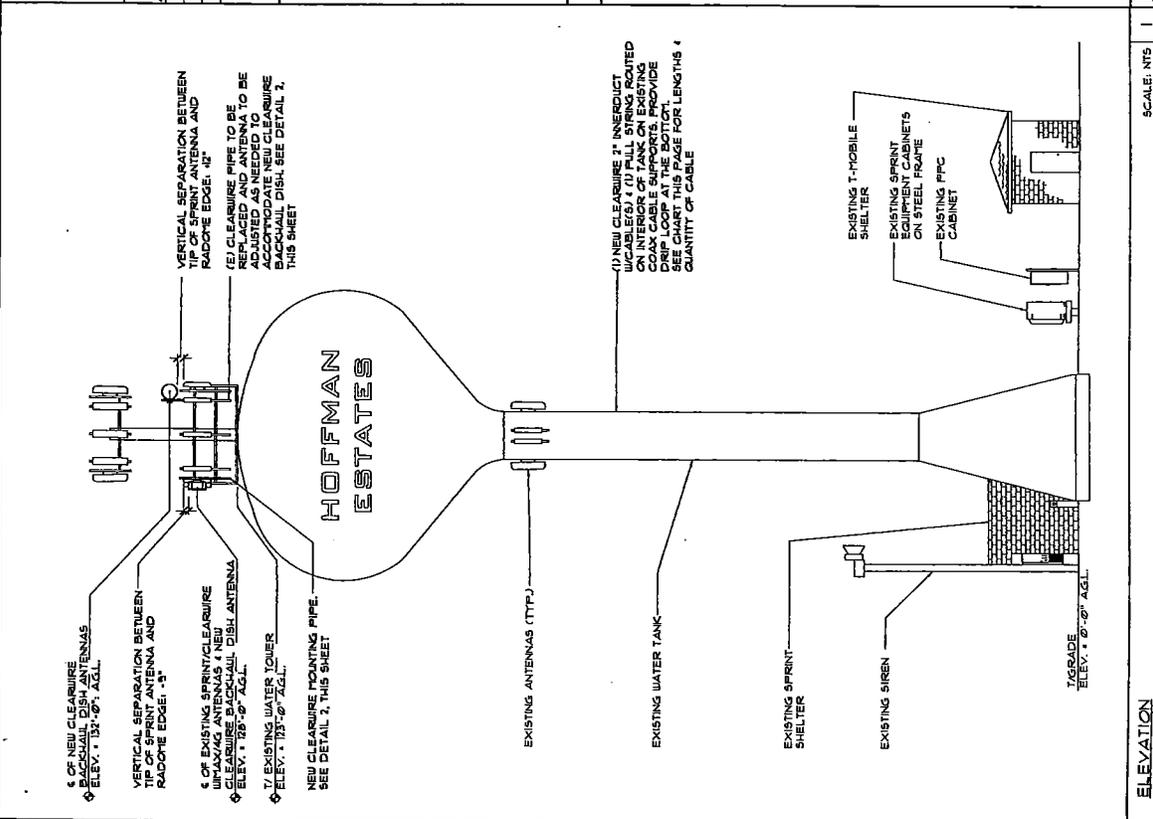
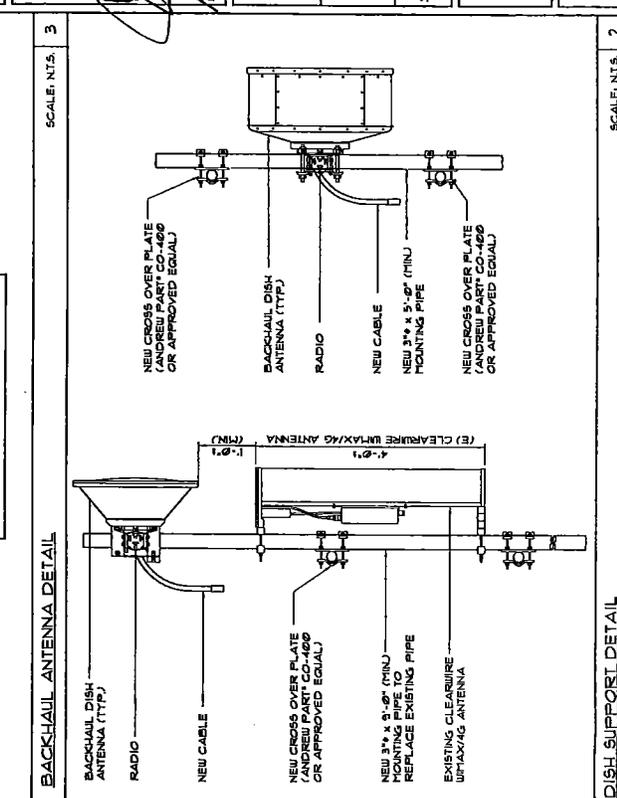
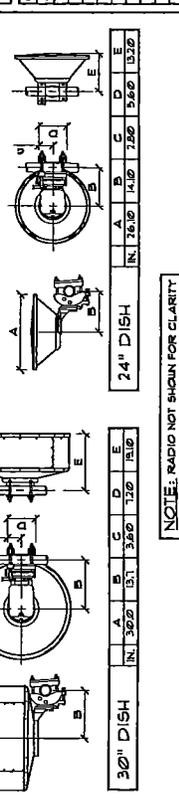
ANTENNA NOTES:
 1. THE SIZE, HEIGHT, AND DIRECTION OF THE ANTENNA SHALL BE ADJUSTED TO MEET SYSTEM REQUIREMENTS.
 2. CONTRACTOR SHALL VERIFY HEIGHT OF ANTENNA WITH CLEARANCE REPRESENTATIVE.
 3. ALL ANTENNA AZIMUTH TO BE FROM TRUE NORTH.

STRUCTURAL NOTES:
 1. STRUCTURAL CALCULATION PREPARED BY FULLERTON ENGINEERING CONSULTANTS, CONTRACTOR TO COORDINATE WITH CLEARANCE REPRESENTATIVE TO OBTAIN A COPY.
 2. CONTRACTOR TO REFER TO TOWER STRUCTURAL CALCULATIONS FOR ADDITIONAL LOADS, NO ERECTION OR MODIFICATION OF TOWER SHALL BE MADE WITHOUT APPROVAL OF STRUCTURAL ENGINEER.

BACKHAUL CABLE SCHEDULE

ANTENNA TYPE	AZIMUTH	QTY.	ESTIMATED LENGTH	RADIO MODEL	TYPE	SIZE
DISH	291°/156°	1 x 180'	180 FT.	5F0500	AIRPAIR HP	17" x
DISH	740°/291°	1 x 180'	180 FT.	5F0500	AIRPAIR HP	17" x
		TOTAL	360 FT.		OF	5F0500

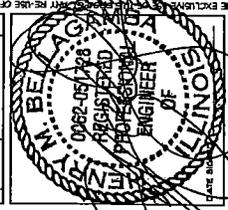
NOTES:
 1. ACTUAL LENGTHS SHALL BE DETERMINED PER SITE CONDITION BY THE CONTRACTOR.
 2. THE CONTRACTOR SHALL VERIFY THE ACTUAL LENGTHS BEFORE INSTALLATION.



clear wireless LLC.
 a Nevada limited liability company,
 a Sprint affiliate
 5600 N. RIVER RD.
 SUITE 300
 ROSEMONT, IL 60018
 (847) 316-3600

RE
 Fullerton Engineering Consultants
 SUITE 800
 5100 W. HIGGINS RD.
 ROSEMONT, ILLINOIS 60018
 TEL: 847-331-0225
 FAX: 847-331-0225
 DESIGN FIRM NO. 184-027436

NO.	DATE	DESCRIPTION	BY
01	08/01/09	50% REVIEW	RA
02	07/01/09	FINAL	RA



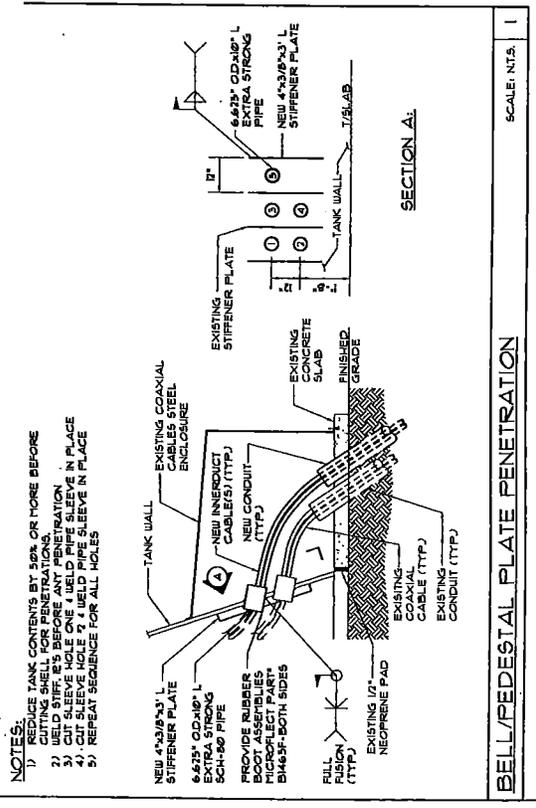
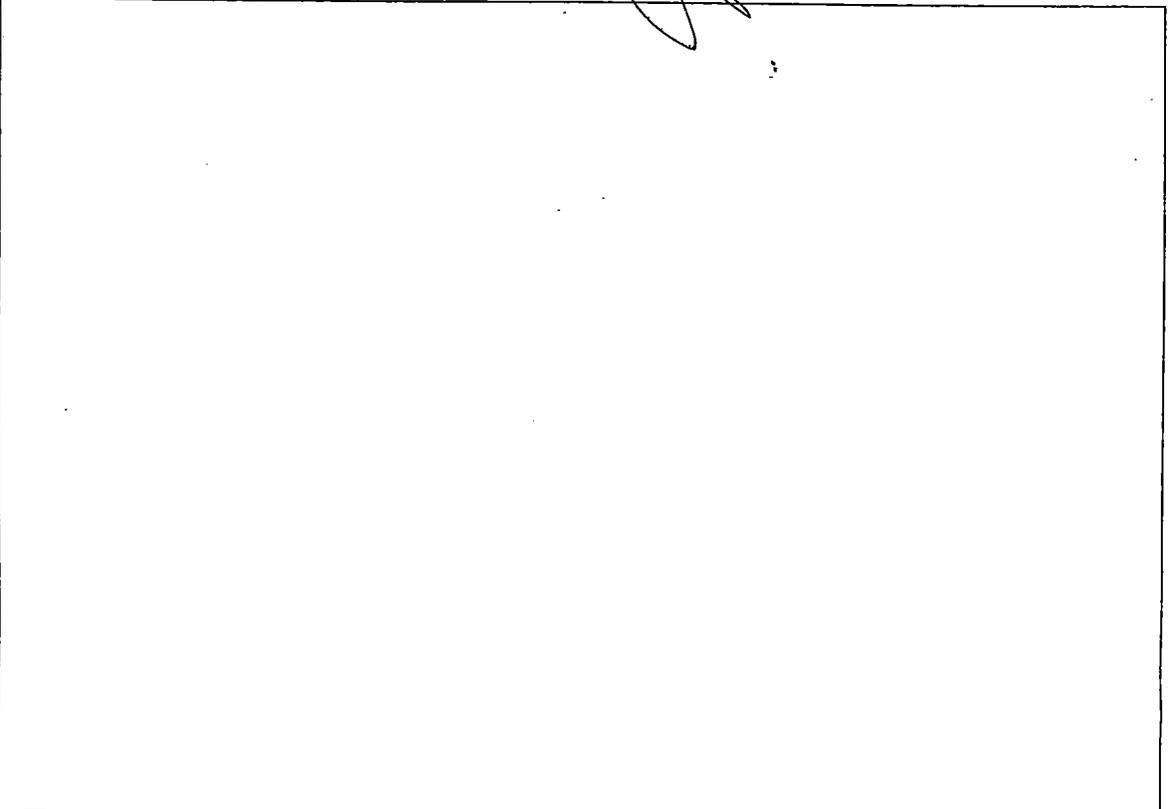
SITE NAME
HOFFMAN ESTATES

SITE NO.
IL-CHI5300

SITE ADDRESS
 3900 WASHINGTON BOULEVARD
 HOFFMAN ESTATES, IL 60135

SHEET NAME
DETAILS

SHEET NUMBER
C-5



THIS DRAWING IS THE PROPERTY OF FULLERTON ENGINEERING CONSULTANTS, INC. IT IS FOR THE EXCLUSIVE USE OF THE CLIENT. ANY REUSE OR REPRODUCTION OF THIS DRAWING WITHOUT THE EXPRESS WRITTEN CONSENT OF FULLERTON ENGINEERING CONSULTANTS, INC. IS PROHIBITED.

clear wireless LLC
a Nevada limited liability company, a Sprint affiliate

3600 N. RIVER RD.
SUITE 300
ROSEMONT, ILL. 60016
(708) 300-3000

ROSEMONT, ILLINOIS 60016
Fullerton Engineering Consultants
51909 W. HIGGINS RD.
TEL: 847-231-0200
FAX: 847-231-0205
DESIGN FIRM NO. 04-002498

CHECKED BY: JP
APPROVED BY: MJB
DATE: 08/20/09
DESCRIPTION: 50% REVIEW
DATE: 08/20/09
DATE: 08/20/09
DATE: 08/20/09

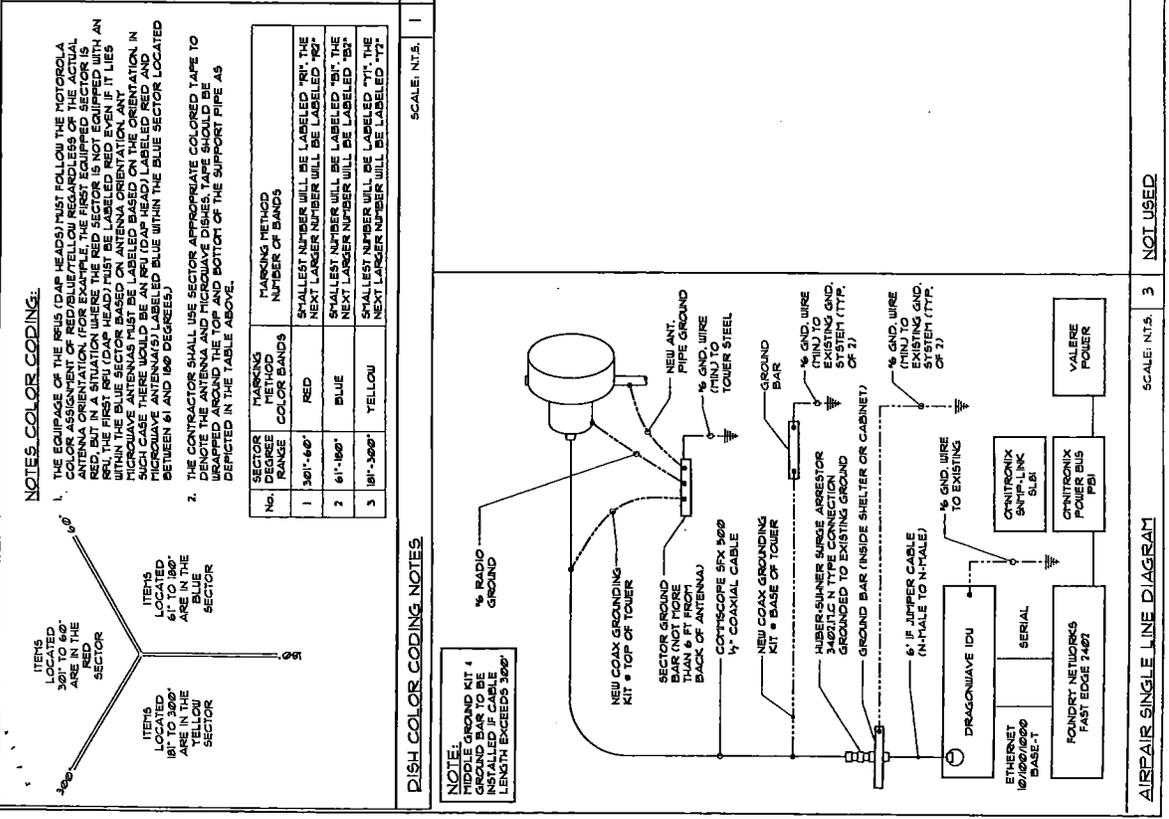
SITE NAME: HOFFMAN ESTATES
SITE NO: IL-CH15380
SITE ADDRESS: 3990 HANFINGTON BOULEVARD, HOFFMAN ESTATES, IL 60135
SHEET NAME: SINGLE LINE & NOTES
SHEET NUMBER: E-1

ANTENNA INSTALLATION NOTES SCALE: NTS, 2

- ANTENNA INSTALLATION SHALL BE CONDUCTED BY FIELD PERSONNEL WITH THE FIELD AND ERECTION CREW. RADIO ANTENNAS, TRANSMISSION LINES, AND SUPPORT STRUCTURES.
- CONTRACT COMPANIES AND THEIR EMPLOYEES SHALL FOLLOW ALL PRACTICES AND PROCEDURES AS SET FORTH IN THE CONTRACT AND PRACTICE MANUALS AND GUIDELINES WHILE PERFORMING SERVICE FOR CLEARWIRE.
- CONTRACT COMPANIES AND THEIR EMPLOYEES SHALL FOLLOW ALL PRACTICES AND PROCEDURES AS SET FORTH IN THE CONTRACT AND PRACTICE MANUALS AND GUIDELINES WHILE PERFORMING SERVICE FOR CLEARWIRE.
- CONTRACTOR SHALL PERFORM A SAFETY INSPECTION OF THE SITE PRIOR TO THE START OF WORK. IF CLEARWIRE LEASED OR OWNED SITE, CLEARWIRE SHOULD BE IMMEDIATELY NOTIFIED OF SAFETY HAZARDS FOUND DURING THE INSPECTION THAT COULD CAUSE DAMAGE TO CLEARWIRE EQUIPMENT OR PERSONNEL. IF THE SAFETY HAZARD IS CORRECTED, THE CONTRACTOR HAS NOTIFIED CLEARWIRE TO PERFORM THE CONNECTION.

GROUNDING NOTES SCALE: NTS, 4

- ALL BASE TRANSFER SITE EQUIPMENT SHALL BE GROUNDED IN ACCORDANCE WITH THE LATEST EDITIONS OF THE NATIONAL ELECTRICAL CODE (NEC) AND LIGHTNING PROTECTION CODE (NFPA 780) AND CLEARWIRE STANDARDS.
- THE ELECTRICAL SERVICE TO THE SITE SHALL BE GROUNDED AT THE SERVICE POINT IN ACCORDANCE WITH THE NATIONAL ELECTRICAL CODE AND IN ACCORDANCE WITH ANY LOCAL CODE.
- ALL UNDERGROUND (BELOW GRADE) GROUNDING CONNECTIONS SHALL BE MADE BY THE EXOTHERMIC PROCESS (MECHANICAL LUG ATTACHMENTS BELOW GRADE ARE NOT ACCEPTABLE). CONNECTIONS SHALL INCLUDE ALL CABLE TO CABLE SPLICES (TEES, X'S, ETC.). ALL UNDERGROUND GROUNDING CONNECTIONS SHALL BE MADE USING LIGHTNING PROTECTION SYSTEMS AS INDICATED. ALL MATERIALS USED (TOLDS, WELDING METAL, TOOLS, ETC.) SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATION AND PROCEDURES.
- ALL GROUNDING AND BONDING CONDUCTORS THAT ARE CONNECTED ABOVE GRADE OR INTERIOR TO A BUILDING SHALL BE CONNECTED USING TWO HOLE CRIMP TYPE (COMPRESSION) CONNECTIONS FOR 1/2" AND 3/4" INSULATED COPPER CONDUCTOR.
- THROUGHOUT THIS DOCUMENT SHALL BE MADE USING AN ANTI-OXIDATION PREVENTATIVE COMPOUND SUCH AS NOXONOL OR AN ANTI-OXIDANT SUCH AS NOXONOL (OR AN APPROVED EQUAL). COAT ALL WIRES BEFORE CONNECTING.
- ALL CONNECTIONS SHALL BE MADE TO BARE METAL. ALL PAINTED SURFACES SHALL BE FIELD INSPECTED AND MODIFIED TO ENSURE PROPER CONTACT. PRIOR TO EXOTHERMICALLY CONNECTING, GALVANIZING SHALL BE REMOVED BY GRINDING SURFACE TO BARE METAL. ALL CONNECTIONS SHALL BE FIELD INSPECTED AND MODIFIED TO BE SURE THEY ARE COMPLETELY SECURED AFTER COMPLETION.
- FERROUS METAL CLIPS WHICH COMPLETELY SURROUND THE GROUNDING AND TYPES THAT BE USED TO SUPPORT GROUNDING CONDUCTORS, STAINLESS STEEL CLIPS WHICH DO NOT COMPLETELY SURROUND THE FERROUS METAL CLIPS WHICH DO NOT COMPLETELY SURROUND THE GROUNDING CONDUCTOR.
- ALL HARDWARE BOLTS, NUTS, WASHERS AND LOCK WASHERS SHALL BE 1/2" STAINLESS STEEL. EVERY CONNECTION SHALL BE BOLT-FLAT. WASHER-BUSS-LUG-FLAT WASHER-LOCK WASHER-NUT IN THAT EXACT ORDER WITH NUT FACING OUTWARD. BACK-TO-BACK LUGGING. BOLT-FLAT WASHER-NUT IN THAT EXACT ORDER IS ACCEPTED WHERE NECESSARY TO CONNECT MANY LUGS TO A BUSS BAR. STACKING OF LUGS, BUSS-LUG-LUG, IS NOT ACCEPTABLE.



**ADDITIONAL
BUSINESS**

AGREEMENT

THIS AGREEMENT, for conditions of employment APPROVED BY THE VILLAGE BOARD ON DECEMBER __, 2009 by and between the Village of Hoffman Estates, State of Illinois, a municipal corporation, hereinafter sometimes called "Village", and James H. Norris, hereinafter sometimes called "Employee", both of whom understand as follows:

WITNESSETH

WHEREAS, the Village desires to continue to employ the services of said James H. Norris as Village Manager of the Village of Hoffman Estates, pursuant to the laws of the State of Illinois and the ordinances of said Village; and

WHEREAS, the parties desire to make provisions for the Employee's duties, compensation, benefits and other conditions of employment; and

WHEREAS, it is the desire of the President and Village Board to continue the services of Employee and provide inducement from him to remain in such employment, to make possible full work productivity by assuring Employee's morale and peace of mind with respect to future security, to provide for an equitable means for terminating Employee's services at such time as he may be unable to discharge his duties due to age or disability or when Village may desire to otherwise terminate his employ; and

WHEREAS, Employee desires to continue employment as Village Manager of the Village of Hoffman Estates and the Village agrees to a severance package as set forth herein.

NOW, THEREFORE, in consideration of the mutual covenants contained herein; the parties hereto agree as follows:

1. Duties - James H. Norris, as Village Manager of the Village of Hoffman Estates, shall continue to perform all duties required by law including, but not limited to, Section 4, Article 7, of the Hoffman Estates Municipal Code and the existing and future ordinances of the Village of Hoffman Estates, Illinois. These duties shall specifically include, but not be limited to, the following:

- A. The Village Manager shall be the administrative head of government subject to the collective direction and supervision of the President and Board of Trustees;

- B. He shall make all appointments to offices and positions as provided by law;
- C. He shall see that the laws and ordinances are enforced;
- D. He shall exercise administrative direction of all departments and divisions now existing or that may hereafter be created by the President and Board of Trustees;
- E. Attend all meetings of the President and Board of Trustees with the responsibility to take part in discussion, but having no vote;
- F. Recommend to the President and Board of Trustees for adoption such measures as may be deemed necessary for the efficient and effective operation of the Village;
- G. Prepare and submit the annual budget and keep the President and Board of Trustees fully advised as to the financial conditions and needs of the Village and to perform such other duties as may be prescribed by law or be required of him by any ordinance or resolution of the President and Board of Trustees.

2. Compensation - The Employee shall be compensated at a rate to be paid in the same manner as other employees of the Village. The President and Board of Trustees shall collectively schedule a review of the Employee's performance in March, annually. Salary review shall occur in conjunction with the March performance review and the President and Board of Trustees may adjust the Employee's annual compensation as the President and Board of Trustees shall deem appropriate.

3. The Employee shall be appointed for an indefinite term and shall be considered an at will employee.

4. Termination Conditions and Pay -

- A. Nothing in this Agreement shall prevent, limit or otherwise interfere with the right of the President and Board of Trustees to terminate the services of Employee at any time subject only to the provisions set forth in this Section, paragraphs C and D, of this Agreement.

B. Nothing in this Agreement shall prevent, limit or otherwise interfere with the right of the Employee to resign at any time from his position with the Village, subject only to the provision set forth in this Section, paragraph G, of this Agreement.

C. In the event that the Employee is terminated during the term of this Agreement by the Village President and Board of Trustees, by receipt and acknowledgment of written notice and concurrently while Employee is willing and able to perform the duties of Village Manager, in such event, the Village agrees to pay Employee under the following schedule:

1. Termination during the term of this Agreement an amount equal to six (6) months aggregate salary to be paid in the same manner as other employees of the Village or until full time professional employment is gained by the Employee, whichever occurs first.

D. In the event that the Employee and Village, are unable to reach agreement upon a new employment agreement between November 30 and December 31, 2010, the Employee shall be able to elect to be terminated as provided in this Section, paragraph E, and will receive six (6) months aggregate salary as severance and liquidated damages to be paid in the same manner as other employees of the Village or until full time professional employment is gained by the Employee, whichever arises first.

Aggregate salary for the terms of this Agreement shall mean salary and benefits including insurance benefits and retirement contributions or an amount sufficient to obtain equal coverage.

E. In the event the Village at any time during the term of this Agreement reduces the salary or other financial benefits of Employee in a greater percentage than an applicable across-the-board reduction for all employees of the Village, or in the event Village refuses, following written

notice to comply with any other provisions benefiting Employee herein, or the Employee receives a formal suggestion by the President and Board of Trustees that he resign, then in that event, Employee may, at his option, within fourteen (14) days of the suggestion of resignation or after delivery of the written demand, elect to be "terminated" as a result of such reduction, suggestions or resignation or such refusal to comply within the meaning and context of the herein termination/severance pay provisions.

F. If Employee shall die during any period in which payments are being made in accordance with Section C or D, the balance of the payments shall be made to Employee's heirs or estate. If Employee should die during his regular employment, the payments under Section C or D will not be made and the insurance proceeds shall be in lieu thereof.

G. In the event Employee voluntarily resigns his position with the Village, the Employee shall give Village thirty (30) days written notice, unless parties otherwise agree. If Employee voluntarily resigns his position, the termination/severance salary and benefit provisions of this Agreement shall not apply.

H. Upon termination, Village shall compensate Employee for all accrued sick, holiday and vacation leave in accordance with the Village of Hoffman Estates' Staff Policy Manual. Said compensation shall be based upon Employee's salary as of the date of employment termination.

I. In the event that the Employee is terminated because of his conviction of any felony or any illegal act involving malfeasance or dishonesty for personal gain then, in that event, Village shall have no obligation to pay termination compensation stated in this Agreement.

5. Automobile Use - Village shall provide Employee with the use of a municipal vehicle for all work related purposes and personal use within a 75-mile radius of Hoffman Estates. Employee shall be responsible for all costs related to personal use. Upon Employee's request, the provision of an automobile will change to an automobile allowance in the amount of \$550.00 per month.

6. Insurance Coverage -

A. Village shall provide full major medical insurance for Employee and dependents as provided for other Village employees.

B. Village shall provide Employee with life insurance in the amount of \$200,000.

7. Vacation and Sick Leave -

A. Employee shall accrue twenty-five (25) days of vacation per year.

B. Employee shall accrue twelve (12) days of sick leave per year and the right to use sick leave in advance of accrual up to 12 days upon presentation of good cause.

8. Hours of Work - The Village recognizes that the Employee must devote a great deal of his time outside normal office hours to business of the Village and, to that end, the Employee will be allowed to take personal compensatory time off during regular business hours. The Village shall have no cash obligation for any compensatory time not used. It is understood that the Village shall not be liable for hour-for-hour compensatory time. In addition, the Employee may engage in occasional teaching, writing, consulting, or military reserve service performed on Employee's time off. This shall not exceed more than 25 hours per year without the prior approval of the President and Board of Trustees.

9. Memberships, Conferences, Training and Education - Village shall budget and pay for the cost of memberships to the International City Management Association (ICMA), Illinois City Management Association (ILCMA), and local metropolitan association as well as the Hoffman Estates Chamber of Commerce. Village shall also budget for the cost of registration, travel and expenses for the ICMA and ILCMA annual conferences as well as other training programs and seminars which serve to continue the professional education and development of the Employee.

10. Business Expenses - The Village shall reimburse Employee for all employment related expenses, including business meals, as may be annually budgeted in accordance with Village policies.

11. General Conditions of Employment - In addition to the benefits cited herein, Village shall provide Employee with any and all benefits that apply to other full-time general Village employees.

12. Binding Effect - This Agreement shall be binding of the Village and Employee and the successors, assigns and heirs of each respectively.

13. Severability - If any clause or provisions herein shall be adjudged invalid or unenforceable by a Court of competent jurisdiction or by operation of any applicable law, it shall not affect the validity of any other clause or provision which shall remain in full force and effect.

14. Waiver of Breach - The waiver by the Village of a breach of any provisions of this Agreement by the Employee shall not operate or be construed as a waiver of any subsequent breach by the Employee.

15. Term - This Agreement shall terminate on December 31, 2010.

IN WITNESS WHEREOF, the Village of Hoffman Estates and Employee have caused this Agreement to be signed and executed.

William D. McLeod, Village President

ATTEST:

Village Clerk

Employee